

# Johns Homestead Park and Dam Improvements

3071 Lawrenceville Highway, Tucker, GA 30084

## Construction Plans

Issued date: 7/16/24



2300 Henderson Mill Road  
Suite 412  
Atlanta, Georgia 30345  
(404)895-2253  
www.RootDStudio.com

### OWNER / DEVELOPER:

The City of Tucker  
Parks and Recreation Department  
4898 Lavista Road  
Tucker, GA 30084

### LANDSCAPE ARCHITECT:

Root Design Studio  
3469 Lawrenceville Highway, #204  
Tucker, GA 30084

### TECHNICAL ADVISOR & QUALITY CONTROL:

Freedman Engineering Goup  
1000 Whitlock Avenue, Suite 320, #218  
Marietta, GA 30064

### DAM ENGINEERING & CIVIL ENGINEERING:

Walden Ashworth & Associates, Inc.  
1827 Powers Ferry Road  
Bldg. 23, Suite 300  
Atlanta, GA 30339

### ENVIRONMENTAL PERMITTING:

Campbell Environmental, Inc.  
2328 Sanford Road  
Decatur, GA 30033

### GEOTECHNICAL SERVICES:

ATLAS, Inc.  
3000 Northfield Place, Suite 1100  
Roswell, GA 30076

### STRUCTURAL ENGINEERING SERVICES:

Stability Engineering  
1376 Church St, Suite 200  
Decatur, GA 30030

### PROJECT INFORMATION:

Project Name:  
Johns Homestead Park and Dam Improvements

Project Address:  
3071 Lawrenceville Highway  
Tucker, GA 30084

Project Info:  
Land Lots: 143, 144, 165 & 166 of the 16th District  
Parcel ID Number: 18 165 02 005  
Zoning: R75

Project Area:  
Total Area: 49.40 Acres (2,151,864 Sq. Ft.)  
Disturbed Area: 4.98 Acres (217,329.41 Sq. Ft.)

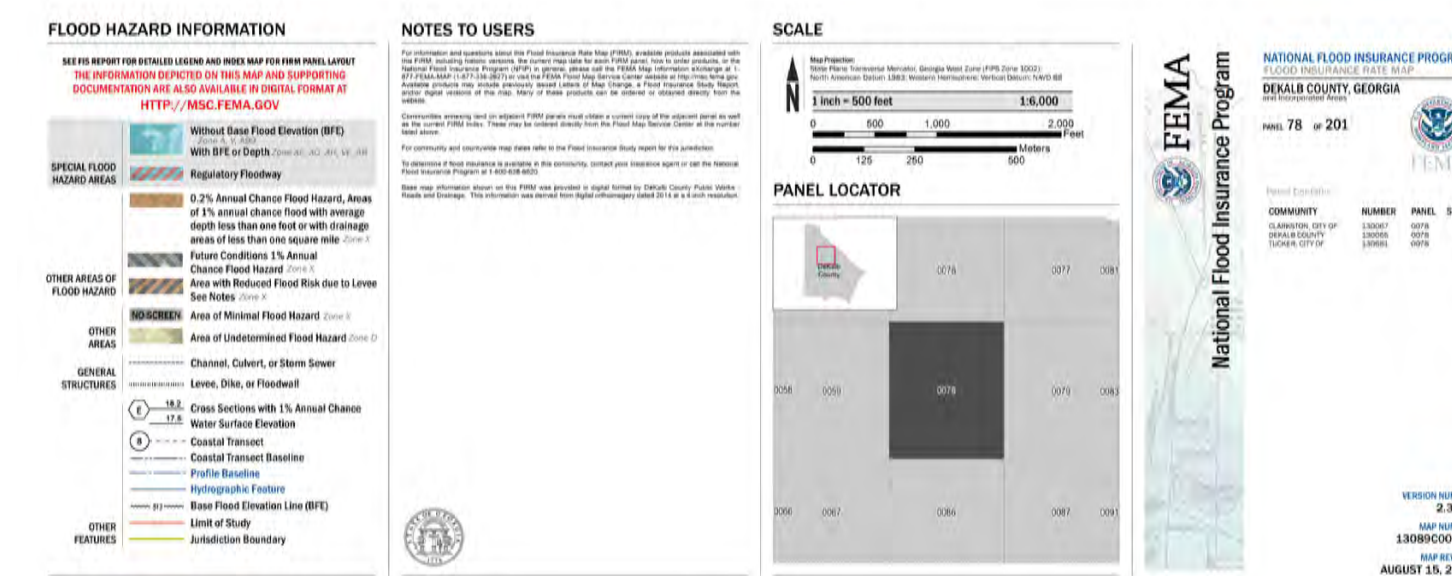
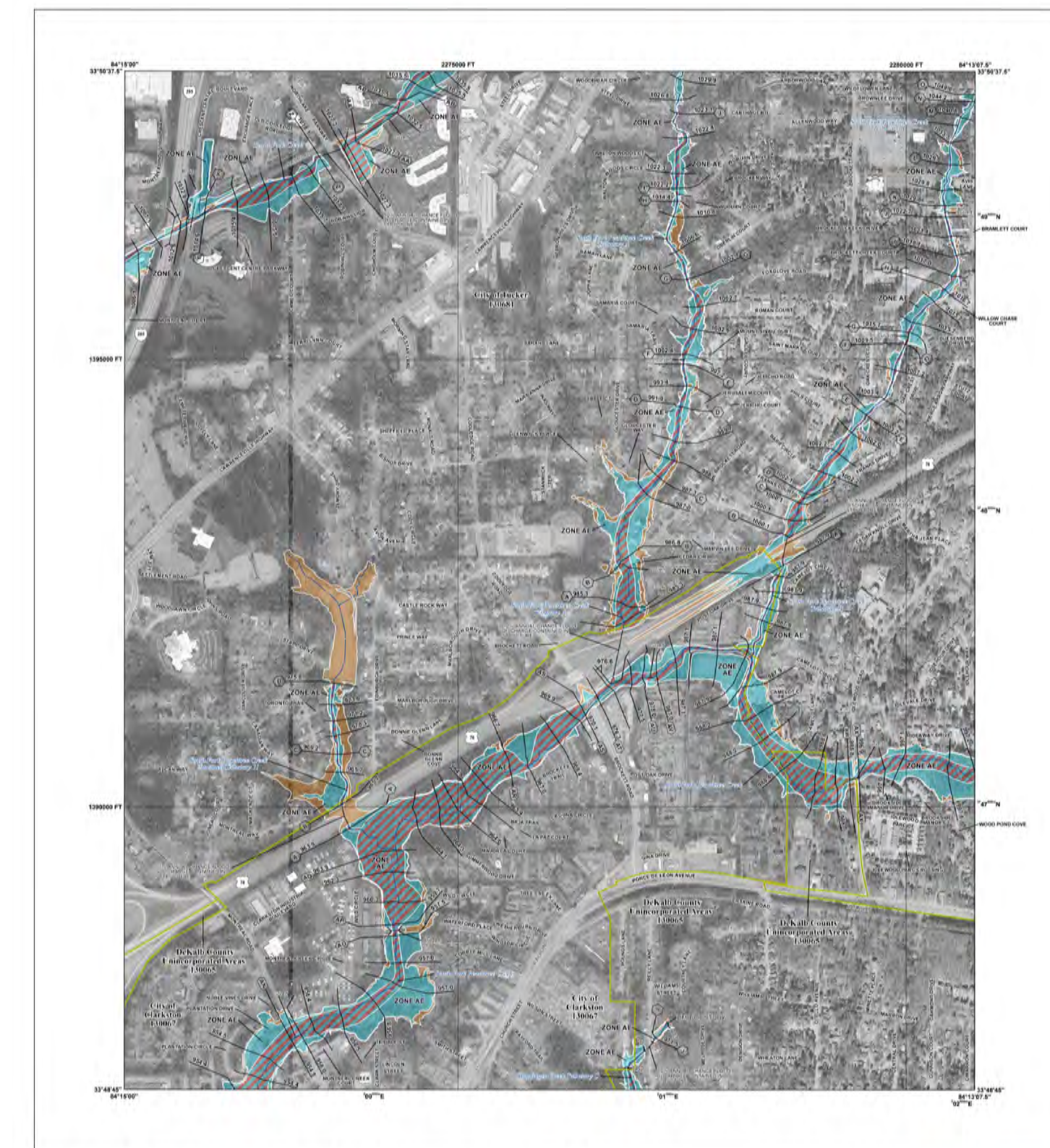
Impervious Coverage:  
Before Construction: 1,081 Sq. Ft. or 0.05 % of Project Site  
After Construction: 11,138 Sq. Ft. or 0.52% of Project Site

### Project Narrative:

This project consists of improvements to an existing park. Site Improvements will include the reconstruction of upper and lower pond dams, site hydrology improvements, trail improvements, parking lot construction, boardwalk/bridges, pedestrian bridges, signage, an accessible observation deck, site furnishings, mitigation landscaping and landscape for screening.

### GENERAL NOTES:

Zoning Conditions:  
Johns Homestead Park and Dam Improvements  
*None*



FEMA DEKALB COUNTY FIRM MAP 13089C0078K,  
REVISED AUGUST 15, 2019



# APPLICABLE CODE:

**Zoning Code:**

Code of Ordinances Tucker, GA

**Accessibility Code:**

ADA Standards for Accessible Design, 2010

**Building Code:**

International Building Code, 2021

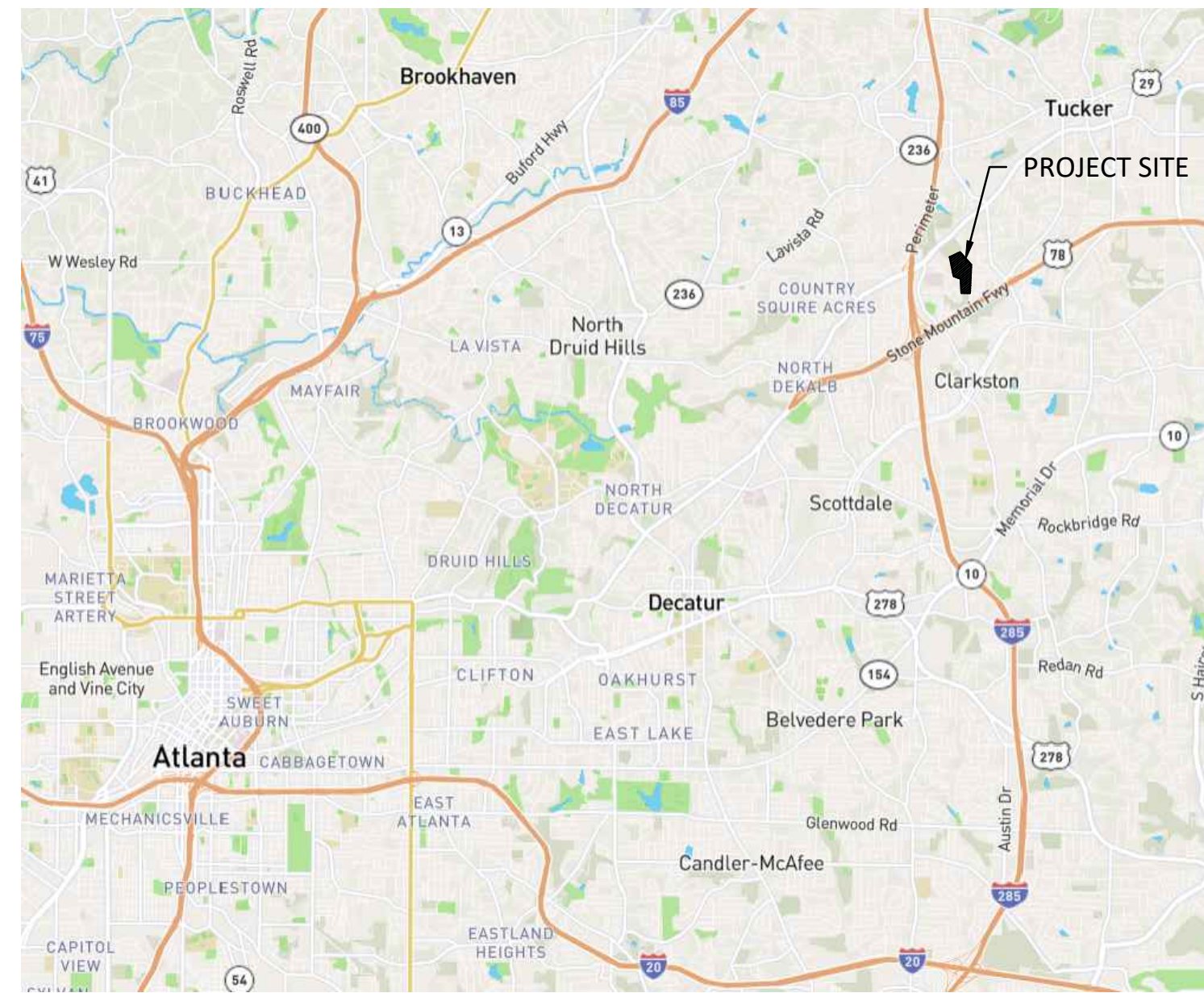
# DRAWING INDEX:

ROOT DESIGN STUDIO SHEETS  
(PARK/SITE IMPROVEMENTS, TREE REMOVAL, BUFFER MITIGATION AND LANDSCAPE REPLACEMENTS)

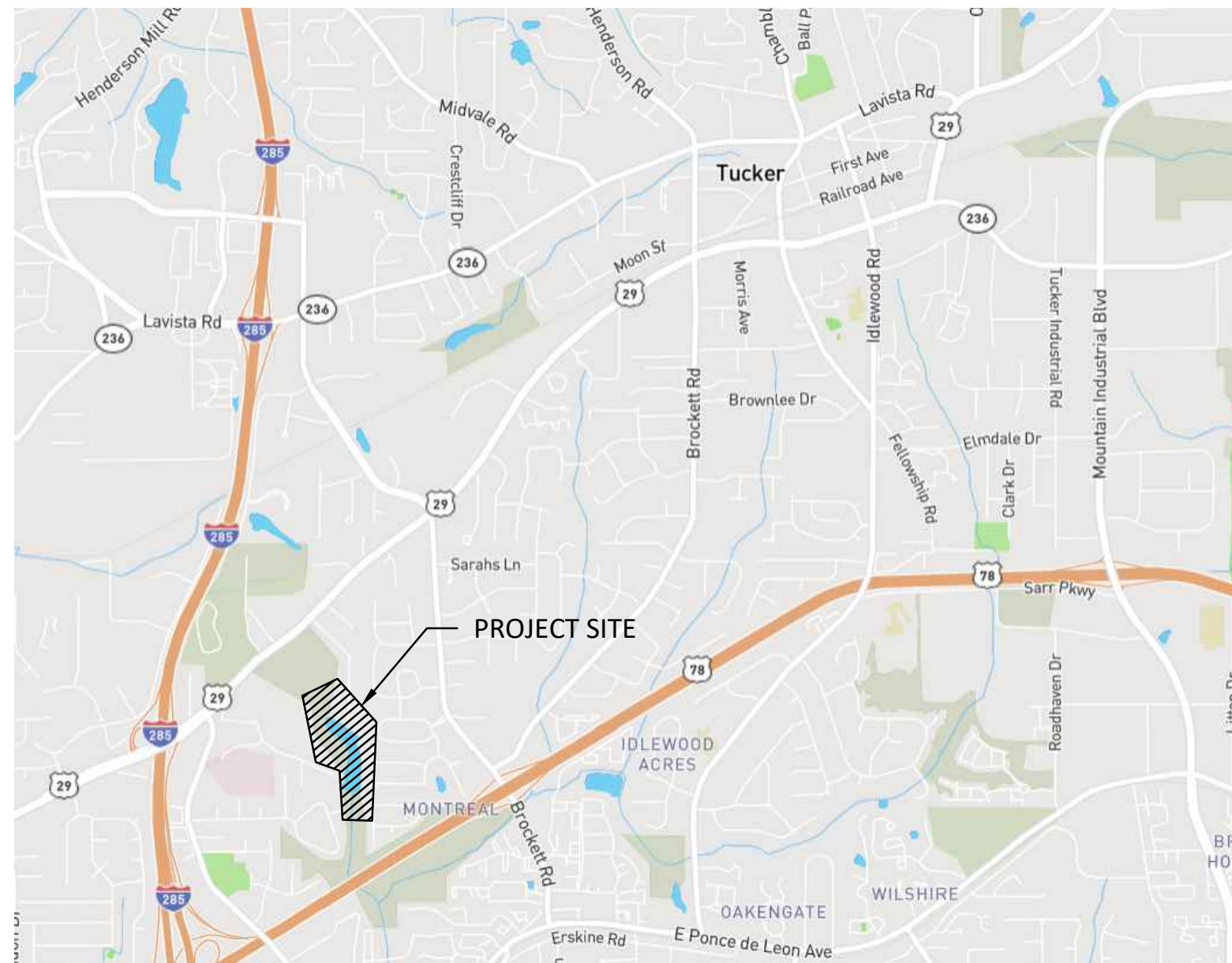
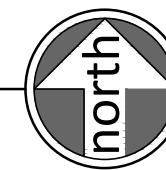
WALDEN ASHWORTH  
(DAM RENOVATIONS, EROSION CONTROL AND MASS SITE WORK)

SHEET #	SHEET NAME
G-000	COVER SHEET PROJECT INFORMATION AND DRAWING INDEX
V-100	EXISTING CONDITIONS
V-101	EXISTING CONDITIONS
V-102	EXISTING CONDITIONS
V-103	EXISTING CONDITIONS
C-000	GENERAL NOTES
C-001	KEY PLAN
C-100	SITE PLAN: AREA A
C-101	SITE PLAN: AREA B
C-300	TRAIL SECTIONS
C-400	STAPP DRIVE PARKING ENLARGEMENT PLAN
C-401	PAVILION ENLARGEMENT PLAN
C-402	BOARDWALK / BRIDGE ENLARGEMENT PLAN
C-403	PEDESTRIAN BRIDGE SPECIFICATIONS
C-500	CONSTRUCTION DETAILS
C-501	CONSTRUCTION DETAILS
C-502	CONSTRUCTION DETAILS
C-503	CONSTRUCTION DETAILS
C-504	CONSTRUCTION DETAILS
C-505	CONSTRUCTION DETAILS
C-506	CONSTRUCTION DETAILS
C-507	CONSTRUCTION DETAILS
C-508	CONSTRUCTION DETAILS
C-509A	CONSTRUCTION DETAILS
C-509B	CONSTRUCTION DETAILS
C-510	CONSTRUCTION DETAILS
C-511	CONSTRUCTION DETAILS
C-512	CONSTRUCTION DETAILS
C-513	CONSTRUCTION DETAILS
C-514	CONSTRUCTION DETAILS
C-515	CONSTRUCTION DETAILS
CG100	GRADING & DRAINAGE PLAN: AREA A
CG101	FINE GRADING PLAN: AREA B
CG102	GRADING AND DRAINAGE PLAN ENLARGEMENTS
L-000	BUFFER MITIGATION PLANTING NOTES
L-100	BUFFER MITIGATION PLANTING KEY PLAN
L-101	BUFFER MITIGATION PLANTING PLAN
L-500	BUFFER MITIGATION PLANTING DETAILS
LT000	TREE PLAN NOTES
LT001	TREE PLAN CALCULATIONS
LT002	TREE AND LANDSCAPE KEY PLAN
LT100	TREE PROTECTION AND REMOVAL PLAN: AREA A
LT101	TREE PROTECTION AND REMOVAL PLAN: AREA B
LT102	TREE AND LANDSCAPE PLAN: AREA A
LT103	TREE AND LANDSCAPE PLAN: AREA B
LT400	LANDSCAPE ENLARGEMENT PLANS
LT500	TREE PLAN DETAILS

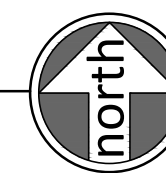
SHEET #	SHEET NAME
COV	JOHN HOMESTEAD PARK DAM RENOVATION COVER SHEET
42016-C-100	GENERAL AND STRUCTURAL NOTES
42016-C-101A	GEOTECHNICAL NOTES 1 OF 3
42016-C-101B	GEOTECHNICAL NOTES 2 OF 3
42016-C-101C	GEOTECHNICAL NOTES 3 OF 3
42016-C-102	RESERVOIR PLAN
42016-C-103A	EXISTING CONDITIONS PLAN - LOWER POND
42016-C-103B	EXISTING CONDITIONS PLAN - UPPER POND
42016-C-104	BORING PLAN
42016-C-105	DEMOLITION PLAN
42016-C-106	SITE LAYOUT PLAN
42016-C-107A	GRADING PLAN - LOWER POND
42016-C-107B	GRADING PLAN - UPPER POND
42016-C-108	UPPER AND LOWER POND - CENTERLINE SECTIONS
42016-C-109	UPPER POND - SECTIONS
42016-C-110	UPPER POND - SECTIONS
42016-C-111	LOWER POND - SECTIONS
42016-C-112	LOWER POND - SECTIONS AND DETAILS
42016-C-113	LOWER POND - SECTIONS AND DETAILS
42016-C-200	EROSION CONTROL - NPDES NOTES, NARRATIVE, BASIN MAP
42016-C-201	EROSION CONTROL - MONITORING NOTES
42016-C-202	EROSION CONTROL - INITIAL PLAN
42016-C-203	EROSION CONTROL - INTERMEDIATE PLAN
42016-C-203A	EROSION CONTROL - INTERMEDIATE PLAN 2
42016-C-204	EROSION CONTROL - FINAL PLAN
42016-C-204A	EROSION CONTROL - FINAL PLAN
42016-C-205	EROSION CONTROL - GRASSING DETAIL
42016-C-206	EROSION CONTROL - DETAILS 1
42016-C-207	EROSION CONTROL - DETAILS 2
42016-C-208	EROSION CONTROL - DETAILS 3
42016-C-209	EROSION CONTROL - CHECKLIST & TMDL
42016-S-300	STRUCTURAL - UPPER DAM SPILLWAY
42016-S-301	STRUCTURAL - UPPER DAM SPILLWAY PLAN, SECTIONS AND DETAILS
42016-S-302	STRUCTURAL - UPPER DAM SPILLWAY PLAN, SECTIONS AND DETAILS
42016-S-303	STRUCTURAL - LOWER SPILLWAY
42016-S-304	STRUCTURAL - LOWER SPILLWAY
42016-S-305	STRUCTURAL - LOWER SPILLWAY



VICINITY MAP  
(NOT TO SCALE)



LOCATION MAP  
(NOT TO SCALE)



Date: 07/16/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MK

**Johns Homestead Park & Recreation Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:		
NO.	DATE	DESCRIPTION

Sheet Title:  
**Project Information & Drawing Index**

Sheet No:  
**G-000**



**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

PREPARED IN THE OFFICE OF:



**GASKINS + LECRAW**

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1266 POWDER SPRINGS RD SW  
MARIETTA, GA 30064  
PHONE - 770.424.7188  
FAX - 770.424.7593  
www.gaskinsleccraw.com  
LSF001371

REVISIONS:

NO.	DATE	DESCRIPTION	BY
1			

TOPOGRAPHIC SURVEY FOR:  
**CITY OF TUCKER**


**JOHNS HOMESTEAD PARK**  
LAND LOTS 143, 144, 165 & 166,  
18TH DISTRICT  
CITY OF TUCKER, DEKALB COUNTY, GEORGIA

CALL BEFORE YOU DIG

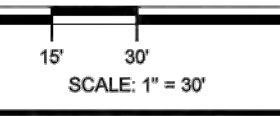


Know what's below.  
Call before you dig.

SCALE & NORTH ARROW:



GRID NORTH - GEORGIA WEST ZONE



SCALE: 1" = 30'

SURVEY INFO:

DRAWN BY:	TBS
REVIEWED BY:	DCO
FIELD DATE:	09/19/2022
OFFICE DATE:	10/03/2022
JOB #:	2207051

LEGEND

□	P.P. - POWER POLE
⊗	L.P. - LIGHT POLE
⊕	F.H. - FIRE HYDRANT
⊙	M.H. - SANITARY SEWER MANHOLE
⊖	W.M. - WATER METER
⊕	G.M. - GAS METER
⊖	R.B.S. - REINFORCING BAR SET
⊕	R.B.F. - REINFORCING BAR FOUND
⊖	C.T.P. - CRIMP TOP PIPE FOUND
⊕	O.T.P. - OPEN TOP PIPE FOUND
⊖	R/W MON. - RIGHT-OF-WAY MONUMENT
---	TYPE OF FENCE
⊕	J.B. - JUNCTION BOX
⊖	D.I. - DROP INLET / YARD INLET
⊕	C.B. - CATCH BASIN
---	R.C.P. - REINFORCED CONCRETE PIPE
---	C.M.P. - CORRUGATED METAL PIPE
---	F.F.E. - FINISHED FLOOR ELEVATION
⊕	WATER VALVE
⊕	S.E.W. - SEWER CLEAN OUT
⊕	TELEPHONE MANHOLE
---	OVERHEAD POWER LINES
---	H.W. - HEADWALL
⊕	POWERBOX
---	STREET ADDRESS
---	W-W - WATER LINE
---	U - UNDERGROUND TELEPHONE LINE
---	G - GAS LINE
---	E - UNDERGROUND ELECTRICAL LINE

GPS NOTES:

1.) HORIZONTAL DATUM IS NAD 83. VERTICAL DATUM IS NAVD 88.

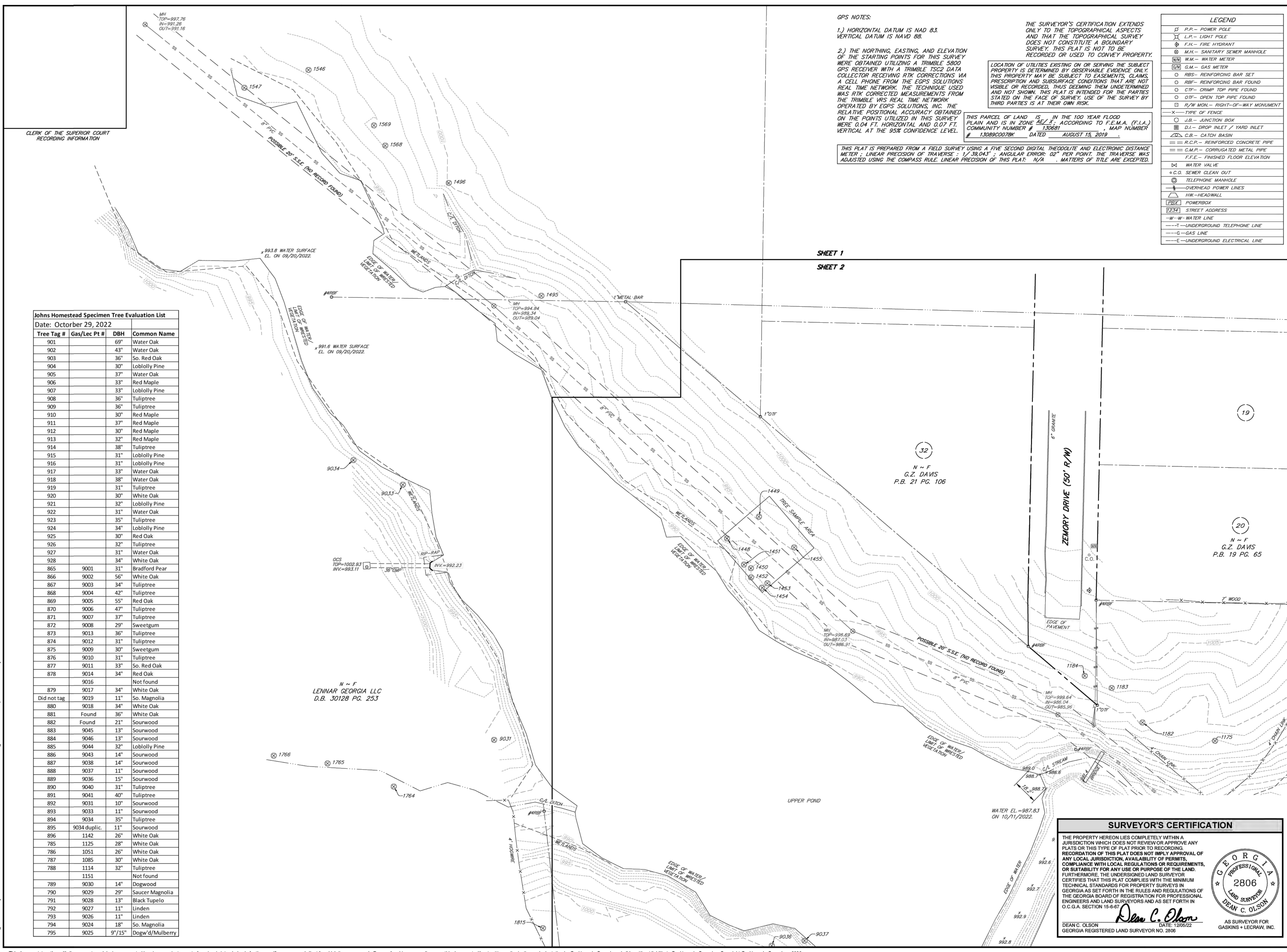
2.) THE NORTHING, EASTING, AND ELEVATION OF THE STARTING POINTS FOR THIS SURVEY WERE OBTAINED UTILIZING A TRIMBLE 5800 GPS RECEIVER WITH A TRIMBLE TSC2 DATA COLLECTOR RECEIVING RTK CORRECTIONS VIA A CELL PHONE FROM THE EGPS SOLUTIONS REAL TIME NETWORK. THE TECHNIQUE USED WAS RTK CORRECTED MEASUREMENTS FROM THE TRIMBLE VRS REAL TIME NETWORK OPERATED BY EGPS SOLUTIONS, INC. THE RELATIVE POSITIONAL ACCURACY OBTAINED ON THE POINTS UTILIZED IN THIS SURVEY WERE 0.04 FT. HORIZONTAL AND 0.07 FT. VERTICAL AT THE 95% CONFIDENCE LEVEL.

THIS PARCEL OF LAND IS IN THE 100 YEAR FLOOD PLAIN AND IS IN ZONE AE/ X. ACCORDING TO F.E.M.A. (F.L.A.) COMMUNITY NUMBER # 130681 MAP NUMBER # 1306900278X DATED AUGUST 15, 2019.

THIS PLAT IS PREPARED FROM A FIELD SURVEY USING A FIVE SECOND DIGITAL THEODOLITE AND ELECTRONIC DISTANCE METER. LINEAR PRECISION OF TRAVERSE = 1/30,043". ANGULAR ERROR: 02" PER POINT. THE TRAVERSE WAS ADJUSTED USING THE COMPASS RULE. LINEAR PRECISION OF THIS PLAT: N/A. MATTERS OF TITLE ARE EXCEPTED.

THE SURVEYOR'S CERTIFICATION EXTENDS ONLY TO THE TOPOGRAPHICAL ASPECTS AND THAT THE TOPOGRAPHICAL SURVEY DOES NOT CONSTITUTE A BOUNDARY SURVEY. THIS PLAT IS NOT TO BE RECORDED OR USED TO CONVEY PROPERTY.

LOCATION OF UTILITIES EXISTING ON OR SERVING THE SUBJECT PROPERTY IS DETERMINED BY OBSERVABLE EVIDENCE ONLY. THIS PROPERTY MAY BE SUBJECT TO EASEMENTS, CLAIMS, PRESCRIPTION AND SUBSURFACE CONDITIONS THAT ARE NOT VISIBLE OR RECORDED, THIS DEEMING THEM UNDETERMINED AND NOT SHOWN. THIS PLAT IS INTENDED FOR THE PARTIES STATED ON THE FACE OF SURVEY. USE OF THE SURVEY BY THIRD PARTIES IS AT THEIR OWN RISK.



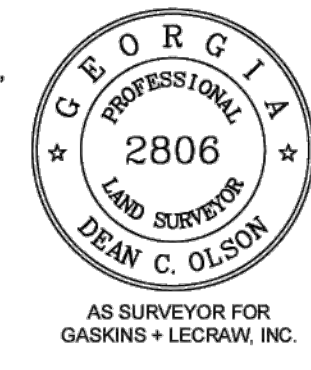
**Johns Homestead Specimen Tree Evaluation List**  
Date: October 29, 2022

Tree Tag #	Gas/Lec Pt #	DBH	Common Name
901		69"	Water Oak
902		43"	Water Oak
903		36"	So. Red Oak
904		30"	Loblolly Pine
905		37"	Water Oak
906		33"	Red Maple
907		33"	Loblolly Pine
908		36"	Tuliptree
909		36"	Tuliptree
910		30"	Red Maple
911		37"	Red Maple
912		30"	Red Maple
913		32"	Red Maple
914		38"	Tuliptree
915		31"	Loblolly Pine
916		31"	Loblolly Pine
917		33"	Water Oak
918		38"	Water Oak
919		31"	Tuliptree
920		30"	White Oak
921		32"	Loblolly Pine
922		31"	Water Oak
923		35"	Tuliptree
924		34"	Loblolly Pine
925		30"	Red Oak
926		32"	Tuliptree
927		31"	Water Oak
928		34"	White Oak
965	9001	31"	Bradford Pear
866	9002	56"	White Oak
867	9003	34"	Tuliptree
868	9004	42"	Tuliptree
869	9005	55"	Red Oak
870	9006	47"	Tuliptree
871	9007	37"	Tuliptree
872	9008	29"	Sweetgum
873	9013	36"	Tuliptree
874	9012	31"	Tuliptree
875	9009	30"	Sweetgum
876	9010	31"	Tuliptree
877	9011	33"	So. Red Oak
878	9014	34"	Red Oak
9015			Not Found
879	9017	34"	White Oak
Did not tag	9019	11"	So. Magnolia
880	9018	34"	White Oak
881	Found	36"	White Oak
882	Found	21"	Sourwood
883	9045	13"	Sourwood
884	9046	13"	Sourwood
885	9044	32"	Loblolly Pine
886	9043	14"	Sourwood
887	9038	14"	Sourwood
888	9037	11"	Sourwood
889	9036	15"	Sourwood
890	9040	31"	Tuliptree
891	9041	40"	Tuliptree
892	9031	10"	Sourwood
893	9033	11"	Sourwood
894	9034	35"	Tuliptree
895	9034 dupl.	11"	Sourwood
896	1142	26"	White Oak
785	1125	28"	White Oak
786	1051	26"	White Oak
787	1085	30"	White Oak
788	1114	32"	Tuliptree
	1151		Not found
789	9030	14"	Dogwood
790	9029	29"	Saucer Magnolia
791	9028	13"	Black Tupelo
792	9027	11"	Linden
793	9026	11"	Linden
794	9024	18"	So. Magnolia
795	9025	9'15"	Dogw b/Mulberry

**SURVEYOR'S CERTIFICATION**

THE PROPERTY HEREON LIES COMPLETELY WITHIN A JURISDICTION WHICH DOES NOT REVIEW OR APPROVE ANY PLATS OR THIS TYPE OF PLAT PRIOR TO RECORDING. RECORDATION OF THIS PLAT DOES NOT IMPLY APPROVAL OF ANY LOCAL JURISDICTION, AVAILABILITY OF PERMITS, COMPLIANCE WITH LOCAL REGULATIONS OR REQUIREMENTS, OR SUITABILITY FOR ANY USE OR PURPOSE OF THE LAND. FURTHERMORE, THE UNDERSIGNED LAND SURVEYOR CERTIFIES THAT THIS PLAT COMPLIES WITH THE MINIMUM TECHNICAL STANDARDS FOR PROPERTY SURVEYS IN GEORGIA AS SET FORTH IN THE RULES AND REGULATIONS OF THE GEORGIA BOARD OF REGISTRATION FOR PROFESSIONAL ENGINEERS AND LAND SURVEYORS AND AS SET FORTH IN O.C.G.A. SECTION 15-6-61.

*Dean C. Olson*  
DATE: 12/05/22  
AS SURVEYOR FOR GASKINS + LECRAW, INC.



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PHONE - 770.424.7168  
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www.gaskinslecrow.com  
LSF001371

REVISIONS:

NO.	DATE	DESCRIPTION	BY	CHECKED BY

Seal:  
Date: 07/16/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MK

# Johns Homestead Park & Dam Improvements

City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

TOPOGRAPHIC SURVEY FOR:  
**CITY OF TUCKER**  
**JOHNS HOMESTEAD PARK**  
LAND LOTS 143, 144, 165 & 166,  
18TH DISTRICT  
CITY OF TUCKER, DEKALB COUNTY, GEORGIA

CALL BEFORE YOU DIG

**811**  
Know what's below.  
Call before you dig.

SCALE & NORTH ARROW.

GRID NORTH-GEORGIA WEST ZONE  
SCALE: 1" = 30'

SURVEY INFO:  
DRAWN BY: TBS  
REVIEWED BY: DCO  
FIELD DATE: 09/19/2022  
OFFICE DATE: 10/03/2022  
JOB #: 2207051

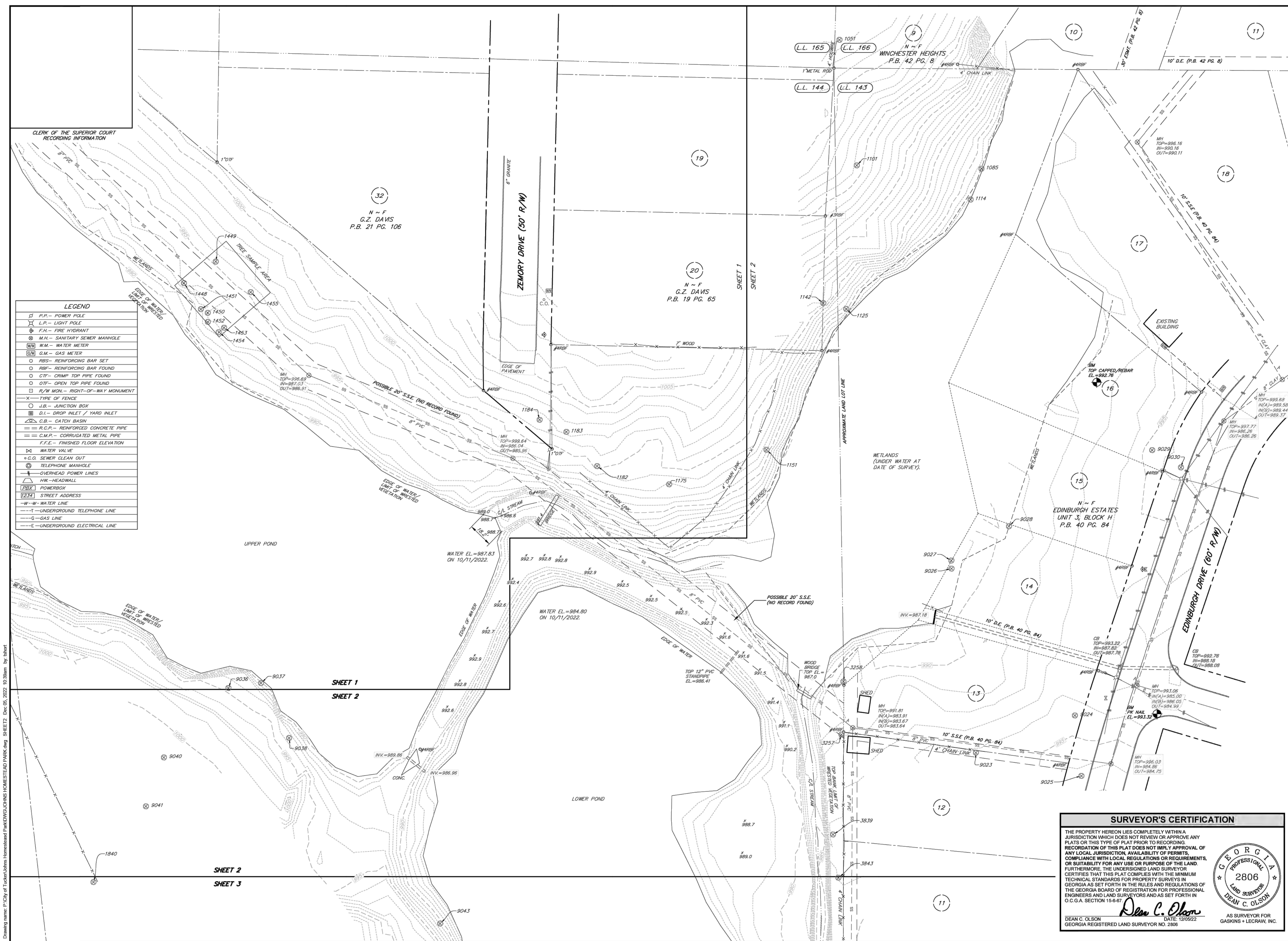
**2 OF 4**

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Existing Conditions**

Sheet No:  
**V-101**



**SURVEYOR'S CERTIFICATION**  
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*Dean C. Olson*  
DEAN C. OLSON  
GEORGIA REGISTERED LAND SURVEYOR NO. 2806  
DATE: 12/09/22  
AS SURVEYOR FOR  
GASKINS + LECRAW, INC.

Drawing name: P:\City of Tucker\Johns Homestead Park\JOHNS HOMESTEAD PARK.dwg SHEET 1 Date: 05/2022 10:30am by: shart

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Seal:

Date: 07/16/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MK

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Existing Conditions**

Sheet No:  
**V-102**

PREPARED IN THE OFFICE OF:

**GASKINS + LECRAW**

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1266 POWDER SPRINGS RD SW  
MARIETTA, GA 30064  
PHONE - 770-424-7168  
FAX - 770-424-7593  
www.gaskinslecraw.com  
LSF001371

REVISIONS:

NO.	DATE	DESCRIPTION	BY
1	07/16/2024	ADD UNRESERVED ELEC. NOTE	PS

TOPOGRAPHIC SURVEY FOR:  
**CITY OF TUCKER**

**JOHNS HOMESTEAD PARK**  
LAND LOTS 143, 144, 165 & 166,  
16TH DISTRICT  
CITY OF TUCKER, DEKALB COUNTY, GEORGIA

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Know what's below.  
Call before you dig.

SCALE & NORTH ARROW:

GRID NORTH-GEORGIA WEST ZONE

SCALE: 1" = 30'

SURVEY INFO:

DRAWN BY: TBS  
REVIEWED BY: DCO  
FIELD DATE: 09/19/2022  
OFFICE DATE: 10/03/2022  
JOB #: 2207051

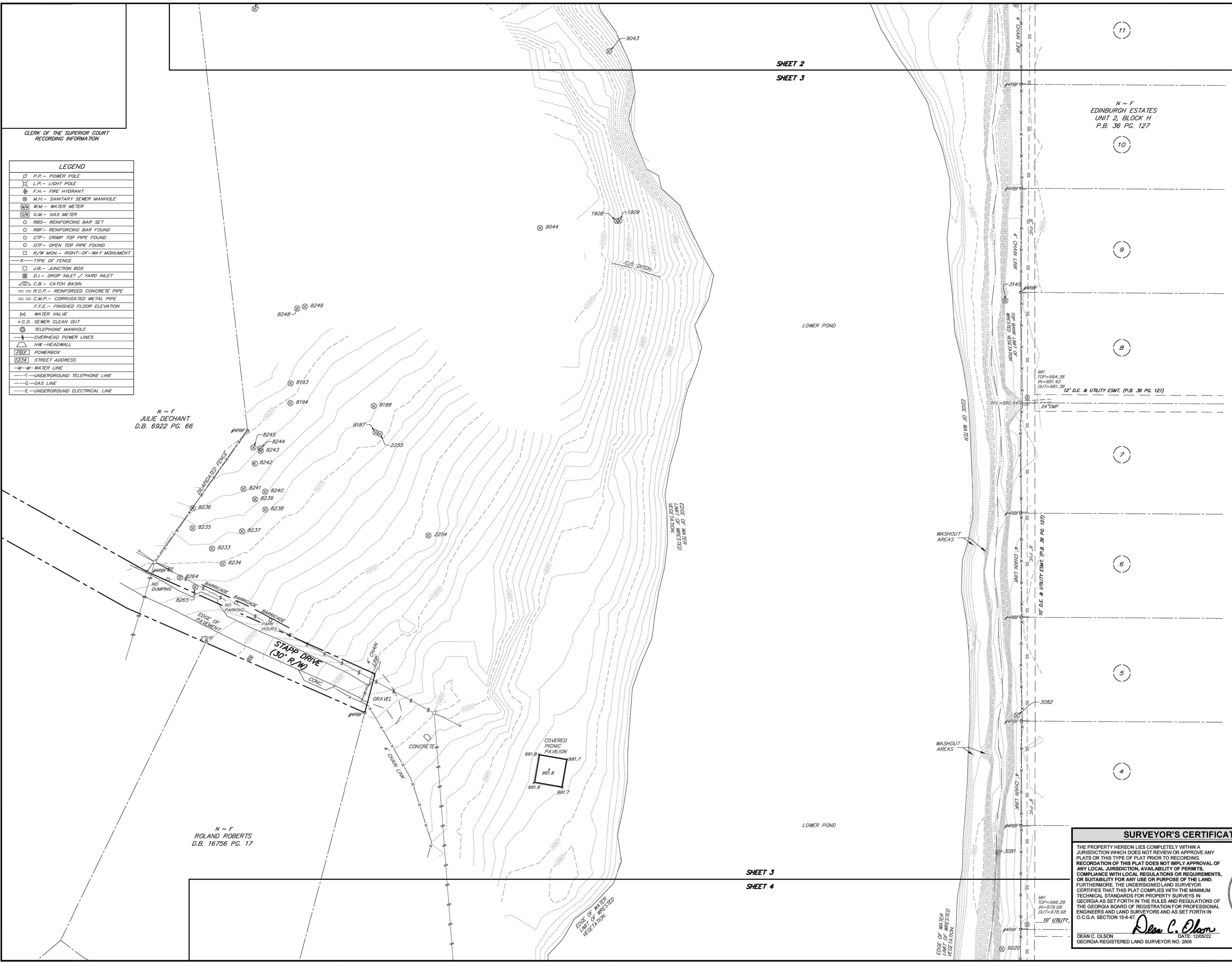
**3 OF 4**

**SURVEYOR'S CERTIFICATION**

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DATE: 12/05/22  
DEAN C. OLSON  
GEORGIA REGISTERED LAND SURVEYOR NO. 2806

AS SURVEYOR FOR  
GASKINS + LECRAW, INC.



LEGEND

○	P.P. - POWER POLE
○	L.P. - LIGHT POLE
○	F.H. - FIRE HYDRANT
○	M.H. - SANITARY SEWER MANHOLE
○	W.M. - WATER METER
○	G.M. - GAS METER
○	RBS - REINFORCING BAR SET
○	RBF - REINFORCING BAR FOUND
○	CTF - CRIMP TOP PIPE FOUND
○	OTF - OPEN TOP PIPE FOUND
□	R/W MON. - RIGHT-OF-WAY MONUMENT
-X-	TYPE OF FENCE
○	J.B. - JUNCTION BOX
□	D.I. - DROP INLET / YARD INLET
△	C.B. - CATCH BASIN
—	R.C.P. - REINFORCED CONCRETE PIPE
—	C.M.P. - CORRUGATED METAL PIPE
—	F.F.E. - FINISHED FLOOR ELEVATION
⊕	WATER VALVE
○	C.O. SEWER CLEAN OUT
○	TELEPHONE MANHOLE
—	OVERHEAD POWER LINES
—	H/W - HEADWALL
—	POWERPOLE
—	STREET ADDRESS
—	W-W - WATER LINE
—	—T— UNDERGROUND TELEPHONE LINE
—	—G— GAS LINE
—	—E— UNDERGROUND ELECTRICAL LINE

CLERK OF THE SUPERIOR COURT  
RECORDING INFORMATION



# Johns Homestead Park & Dam Improvements

City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Existing Conditions**

Sheet No:  
**V-103**

PREPARED IN THE OFFICE OF:



**GASKINS LECRAW**

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1286 POWDER SPRINGS RD SW  
MARIETTA, GA 30064  
PHONE - 770.424.7188  
FAX - 770.424.7553  
www.gaskinslecraw.com  
LSF001371

REVISIONS:

NO.	DATE	BY	DESCRIPTION

TOPOGRAPHIC SURVEY FOR:  
**CITY OF TUCKER**

**JOHNS HOMESTEAD PARK**  
LAND LOTS 143, 144, 185 & 186,  
CITY OF TUCKER, DEKALB COUNTY, GEORGIA

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Know what's below.  
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SCALE & NORTH ARROW:



GRID NORTH-GEORGIA WEST ZONE



SCALE: 1" = 30'

**4 OF 4**

SURVEY INFO:

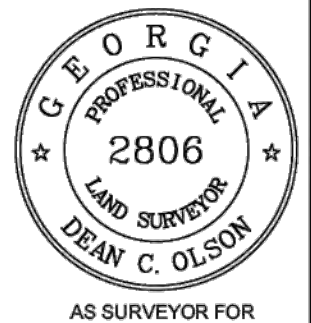
DRAWN BY: TBS  
REVIEWED BY: DCO  
FIELD DATE: 09/19/2022  
OFFICE DATE: 10/03/2022  
JOB #: 2207051

**LEGEND**

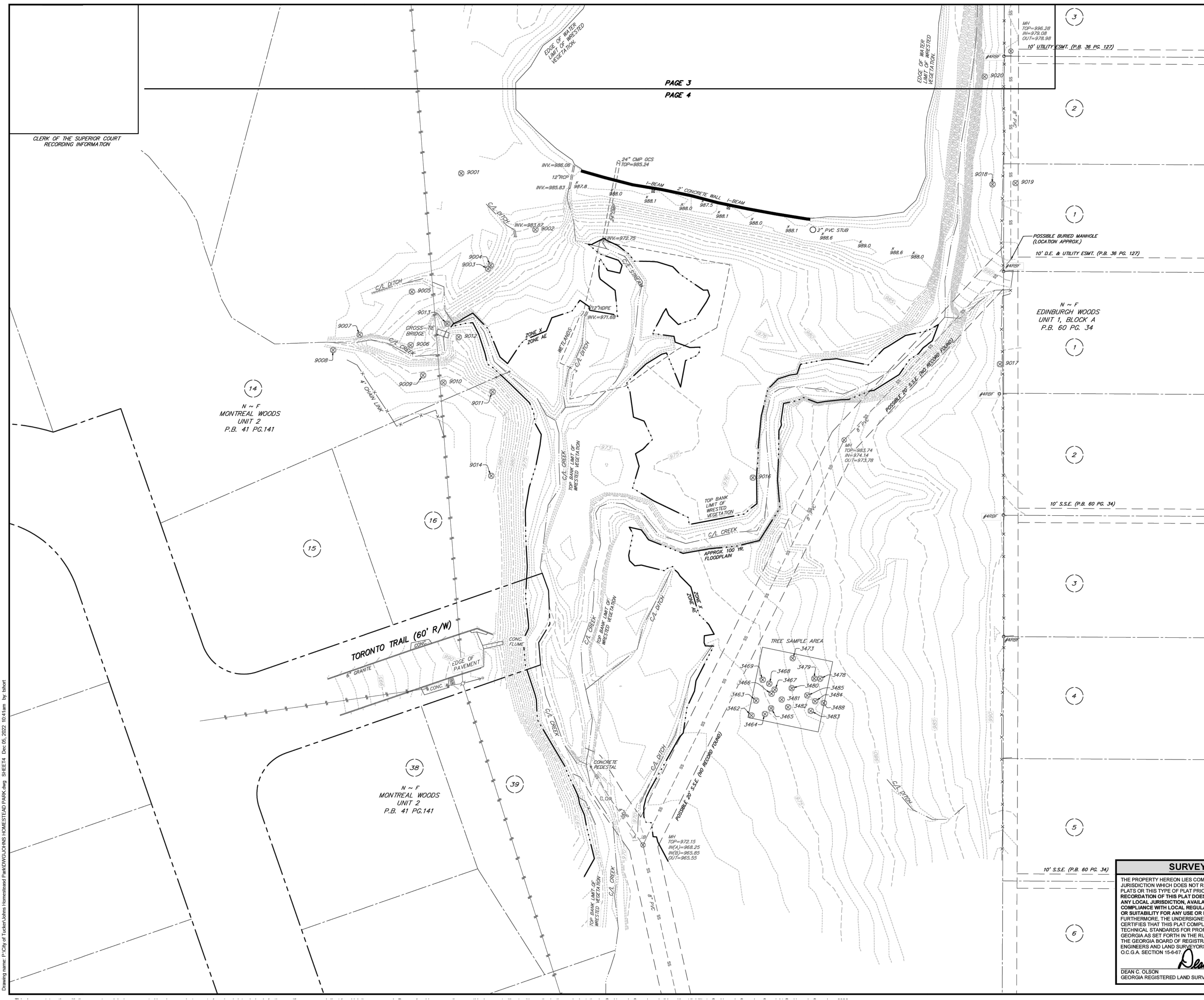
- ⊕ P.P. - POWER POLE
- ⊕ L.P. - LIGHT POLE
- ⊕ F.H. - FIRE HYDRANT
- ⊕ S.M. - SANITARY SEWER MANHOLE
- ⊕ W.M. - WATER METER
- ⊕ G.M. - GAS METER
- ⊕ R.B.S. - REINFORCING BAR SET
- ⊕ R.B.F. - REINFORCING BAR FOUND
- ⊕ C.T.P. - CRIMP TOP PIPE FOUND
- ⊕ O.T.P. - OPEN TOP PIPE FOUND
- ⊕ R/W MON. - RIGHT-OF-WAY MONUMENT
- TYPE OF FENCE
- ⊕ J.B. - JUNCTION BOX
- ⊕ D.I. - DROP INLET / YARD INLET
- ⊕ C.B. - CATCH BASIN
- == R.C.P. - REINFORCED CONCRETE PIPE
- == C.M.P. - CORRUGATED METAL PIPE
- F.F.E. - FINISHED FLOOR ELEVATION
- ⊕ WATER VALVE
- ⊕ C.O. SEWER CLEAN OUT
- ⊕ TELEPHONE MANHOLE
- OVERHEAD POWER LINES
- HW - HEADWALL
- ⊕ P.B.X. - POWERBOX
- ⊕ S34 - STREET ADDRESS
- W - WATER LINE
- T - UNDERGROUND TELEPHONE LINE
- G - GAS LINE
- E - UNDERGROUND ELECTRICAL LINE

**SURVEYOR'S CERTIFICATION**

THE PROPERTY HEREON LIES COMPLETELY WITHIN A JURISDICTION WHICH DOES NOT REVIEW OR APPROVE ANY PLATS OR THIS TYPE OF PLAT PRIOR TO RECORDING. RECORDATION OF THIS PLAT DOES NOT IMPLY APPROVAL OF ANY LOCAL JURISDICTION, AVAILABILITY OF PERMITS, COMPLIANCE WITH LOCAL REGULATIONS OR REQUIREMENTS, OR SUITABILITY FOR ANY USE OR PURPOSE OF THE LAND. FURTHERMORE, THE UNDERSIGNED LAND SURVEYOR CERTIFIES THAT THIS PLAT COMPLIES WITH THE MINIMUM TECHNICAL STANDARDS FOR PROPERTY SURVEYS IN GEORGIA AS SET FORTH IN THE RULES AND REGULATIONS OF THE GEORGIA BOARD OF REGISTRATION FOR PROFESSIONAL ENGINEERS AND LAND SURVEYORS AND AS SET FORTH IN O.C.G.A. SECTION 15-8-47.



*Dean C. Olson*  
DATE: 10/03/22  
AS SURVEYOR FOR GASKINS + LECRAW, INC.



Drawing name: P:\City of Tucker\Johns Homestead Park\DWG\JOHNS HOMESTEAD PARK.dwg, SHEET 4, Dec 05, 2022, 10:41am, by: abart



**GENERAL NOTES:**

1. IF ANY OF THESE NOTES ARE FOUND TO BE IN CONFLICT WITH LOCAL JURISDICTION NOTES AND SPECIFICATIONS, THEN LOCAL JURISDICTION NOTES AND SPECIFICATIONS TAKE PRECEDENCE.
2. ALL WORK AND MATERIALS SHALL COMPLY WITH ALL FEDERAL, STATE, AND LOCAL REGULATIONS AND CODES AS REQUIRED.
3. CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR PRECISE BUILDING DIMENSIONS, BUILDING UTILITY ENTRANCE LOCATIONS, EXACT LOCATIONS AND DIMENSIONS OF ENTRIES, DOWNSPOUTS, AND OTHER FEATURES RELATED TO BUILDINGS AND STRUCTURES.
4. UNLESS SHOWN OTHERWISE ON THE PLANS, CONTRACTOR SHALL APPLY 2" OF TOP SOIL TO ALL DISTURBED AREAS OF THE SITE. PLANT GRASS SEED OR SOD, APPLY STRAW, AND WATER. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH A HEALTHY STAND OF GRASS ON ALL SEEDED OR SODDED AREAS. IF A HEALTHY STAND OF GRASS CAN NOT BE ESTABLISHED BY THE TIME THE BUILDING BECOMES OCCUPIED, THEN SOD SHALL BE INSTALLED AND WATERED UNTIL GRASS IS ESTABLISHED.
5. ALL DIMENSIONS AND RADII ARE REFERENCED TO THE FACE OF CURB UNLESS OTHERWISE NOTED. ALL BUILDING DIMENSIONS ARE REFERENCED TO THE OUTSIDE FACE OF THE STRUCTURE UNLESS OTHERWISE NOTED.
6. IF REQUIRED, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL RELOCATIONS, INCLUDING BUT NOT LIMITED TO, ALL UTILITIES, STORM DRAINAGE, SIGNS, ETC. AS REQUIRED. ALL WORK SHALL BE IN ACCORDANCE WITH GOVERNING AUTHORITIES SPECIFICATIONS AND SHALL BE APPROVED BY SUCH. ALL COST SHALL BE INCLUDED IN BID. AREAS TO BE DISTURBED SHALL BE IMPROVED PER THESE PLANS OR RESTORED TO THEIR ORIGINAL OR BETTER CONDITION.
7. ALL HEIGHTS AND SETBACKS SHALL MEET THE MINIMUM STANDARDS SET FORTH IN THE LOCAL CODE.
8. THE CONTRACTOR SHALL PROTECT ALL MONUMENTS, IRON PINS, AND PROPERTY CORNERS DURING CONSTRUCTION.
9. CONTRACTOR AGREES TO REPAIR ANY DAMAGE TO THE PUBLIC RIGHT-OF-WAY IN ACCORDANCE WITH THE STANDARDS OF THE GDOT.
10. THE CONTRACTOR SHALL IMMEDIATELY REPORT TO THE OWNER ANY DISCREPANCIES FOUND BETWEEN THE ACTUAL FIELD CONDITIONS AND THE CONSTRUCTION DOCUMENTS AND SHALL WAIT FOR INSTRUCTION PRIOR TO PROCEEDING.
11. CONTRACTOR SHALL TAKE REASONABLE MEASURES TO PROTECT WILDLIFE ON PROJECT SITE FROM UNNECESSARY HARM DURING CONSTRUCTION ACTIVITIES.

**TRAFFIC CONTROL NOTES:**

1. REFER TO MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (LATEST EDITION) FOR DETAILS OF STANDARD TRAFFIC CONTROL SIGNS AND STANDARDS.
2. THE CONTRACTOR SHALL EMPLOY ALL NECESSARY BARRICADES, SIGNS, FENCES, FLASHING LIGHTS, TRAFFIC MEN, ETC. FOR MAINTENANCE AND PROTECTION OF TRAFFIC AS REQUIRED.

**GRADING NOTES:**

1. ALL CUT OR FILL SLOPES SHALL BE 2:1 OR FLATTER UNLESS OTHERWISE NOTED.
2. EXISTING AND PROPOSED GRADE CONTOUR INTERVALS SHOWN AT [1 FOOT].
3. CONTRACTOR SHALL ADJUST AND/OR CUT EXISTING PAVEMENT AS NECESSARY TO ASSURE A SMOOTH FIT AND CONTINUOUS GRADE.
4. CONTRACTOR SHALL ASSURE POSITIVE DRAINAGE AWAY FROM BUILDINGS FOR ALL NATURAL AND PAVED AREAS.
5. TOPOGRAPHIC INFORMATION TAKEN FROM A TOPOGRAPHIC SURVEY BY GASKINS & LECRAW, INC..
6. UNLESS OTHERWISE NOTED ON THE LANDSCAPE PLANS, ALL UNSURFACED AREAS DISTURBED BY GRADING OPERATION SHALL RECEIVE 2 INCHES OF TOPSOIL. CONTRACTOR SHALL APPLY STABILIZATION FABRIC TO ALL SLOPES 3H:1V OR STEEPER. CONTRACTOR SHALL GRASS AND MAINTAIN DISTURBED AREAS UNTIL A HEALTHY STAND OF GRASS IS OBTAINED.
7. ALL PROPOSED CONTOURS AND SPOT ELEVATIONS REFLECT FINISHED GRADES.
8. ALL ELEVATIONS ARE IN REFERENCE TO THE BENCHMARK, AND THIS MUST BE VERIFIED BY THE GENERAL CONTRACTOR PRIOR TO GROUND BREAKING.
9. THE CONTRACTOR SHALL IMMEDIATELY REPORT TO OWNER ANY DISCREPANCIES FOUND BETWEEN ACTUAL FIELD CONDITIONS AND CONSTRUCTION DOCUMENTS AND SHALL WAIT FOR INSTRUCTION PRIOR TO PROCEEDING.
10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING EXISTING UTILITIES, AND SHALL REPAIR ALL DAMAGE TO EXISTING UTILITIES THAT OCCUR DURING CONSTRUCTION.
11. CONTRACTOR SHALL BLEND NEW EARTHWORK SMOOTHLY TO TRANSITION BACK TO EXISTING GRADE.
12. ALL SITE PREPARATION AND UNSUITABLE SOIL REMOVAL, AS WELL AS THE PLACEMENT OF FILL MATERIALS SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL REPORT (BY OTHERS).
13. LIMITS OF CLEARING SHOWN ON GRADING PLAN ARE BASED UPON THE APPROXIMATE CUT AND FILL SLOPE LIMITS, OR OTHER GRADING REQUIREMENTS.
14. THE PROPOSED CONTOURS SHOWN IN DRIVES AND PARKING LOTS AND SIDEWALKS ARE FINISHED ELEVATIONS INCLUDING PAVEMENT. REFER TO PAVEMENT CROSS SECTION DATA TO ESTABLISH CORRECT SUBBASE OR AGGREGATE BASE COURSE ELEVATIONS.
15. CONTRACTOR SHALL ENSURE POSITIVE DRAINAGE SO THAT RUNOFF WILL DRAIN BY GRAVITY FLOW ACROSS NEW PAVEMENT AREAS TO NEW OR EXISTING DRAINAGE INLETS OR SHEET OVER LAND.
16. ANY GRADING BEYOND THE LIMITS OF CONSTRUCTION AS SHOWN ON THE GRADING PLAN IS PROHIBITED.
17. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND IMPLEMENTATION OF ALL SHEETING, SHORING, BRACING AND SPECIAL EXCAVATION MEASURES REQUIRED TO MEET OSHA, FEDERAL, STATE AND LOCAL REGULATIONS PURSUANT TO THE INSTALLATION OF THE WORK INDICATED ON THESE DRAWINGS. THE DESIGN ENGINEER ACCEPTS NO RESPONSIBILITY FOR THE DESIGN(S) TO INSTALL SAID ITEMS.
18. THE CONTRACTOR SHALL INCLUDE IN THE BID ANY DEWATERING AND MOISTURE CONDITIONING NECESSARY TO CONSTRUCT THE PROJECT AS SHOWN ON THE PLANS.
19. ALL FOUNDATION EXCAVATION SHALL BE INSPECTED BY A QUALIFIED GEOTECHNICAL ENGINEER TO DETERMINE WHETHER UNSUITABLE MATERIAL MUST BE REMOVED. ALL UNDESIRABLE MATTER SHALL BE REMOVED, BACKFILLED AND COMPACTED AS REQUIRED BY THE GEOTECHNICAL REPRESENTATIVE.
20. GRADES, ELEVATIONS AND LOCATIONS SHOWN ARE APPROXIMATE. AS DIRECTED BY THE ENGINEER, THEY MAY BE ADJUSTED TO ACCOMMODATE UNFORESEEN CONDITIONS. STATIONS, OFFSETS AND ELEVATIONS REFER TO THE CENTER OF DROP INLETS, MANHOLES AND JUNCTION BOXES, AND THE MIDPOINT OF THE LIP FOR CATCH BASINS.
21. REFER TO WALDEN ASHWORTH PLANS FOR POND AREA GRADING, SPILLWAY DETAILS AND BRIDGE DETAILS.



Date: 07/16/2024  
 Project No: 2022-024  
 Drawn By: PS  
 Checked By: MK

**Johns Homestead Park & Dam Improvements**  
 City of Tucker Parks and Recreation  
 4898 Lavista Road  
 Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**General Notes**

Sheet No:  
**C-000**

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Date: 07/16/2024  
Project No: 2022-024  
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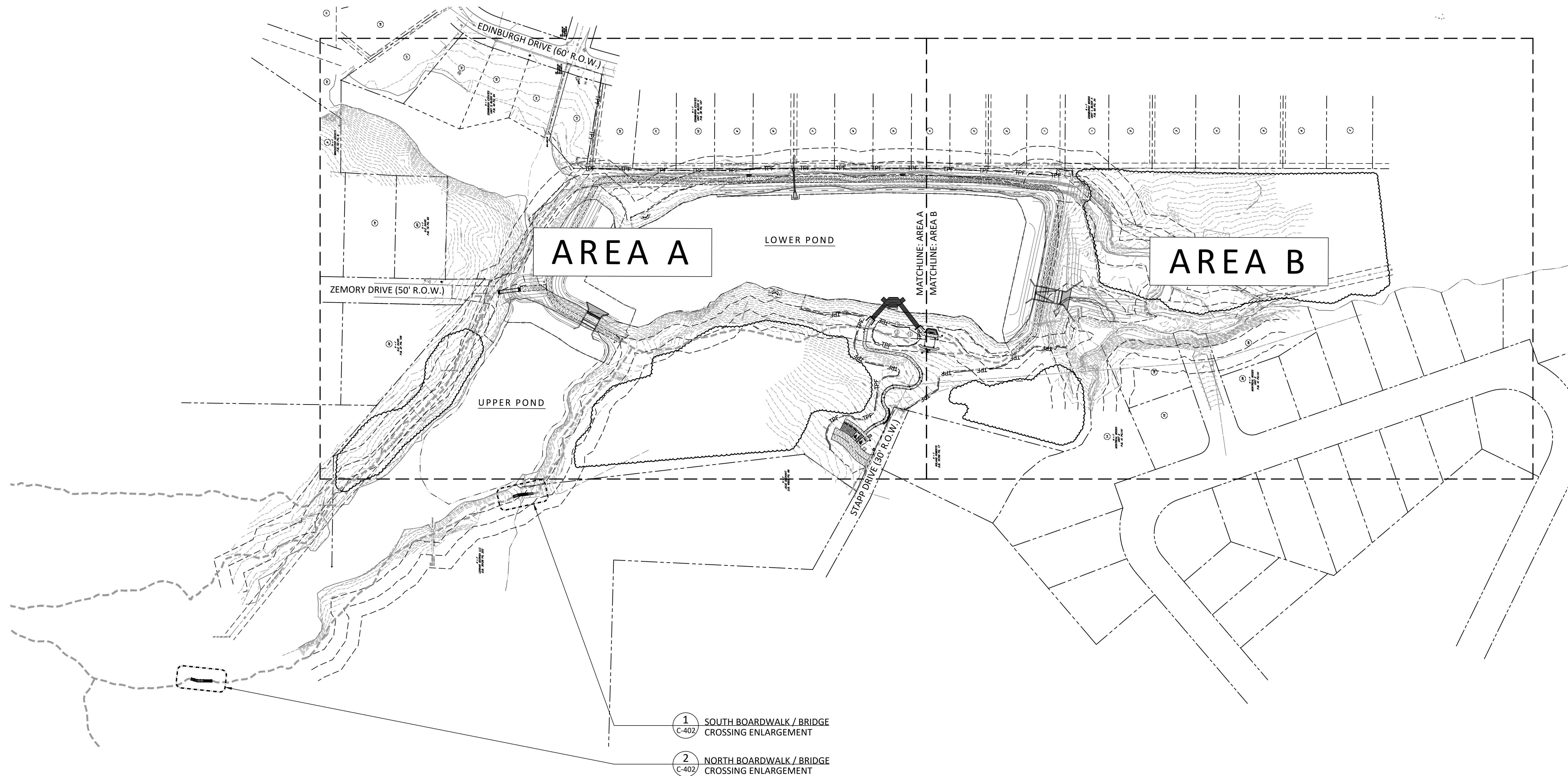
**Johns Homestead Park & Recreation Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

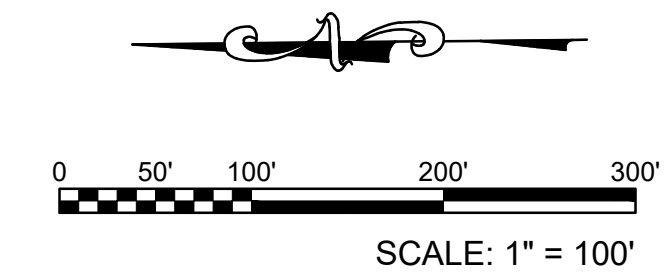
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Sheet Title:  
**Key Plan**

Sheet No:  
**C-001**

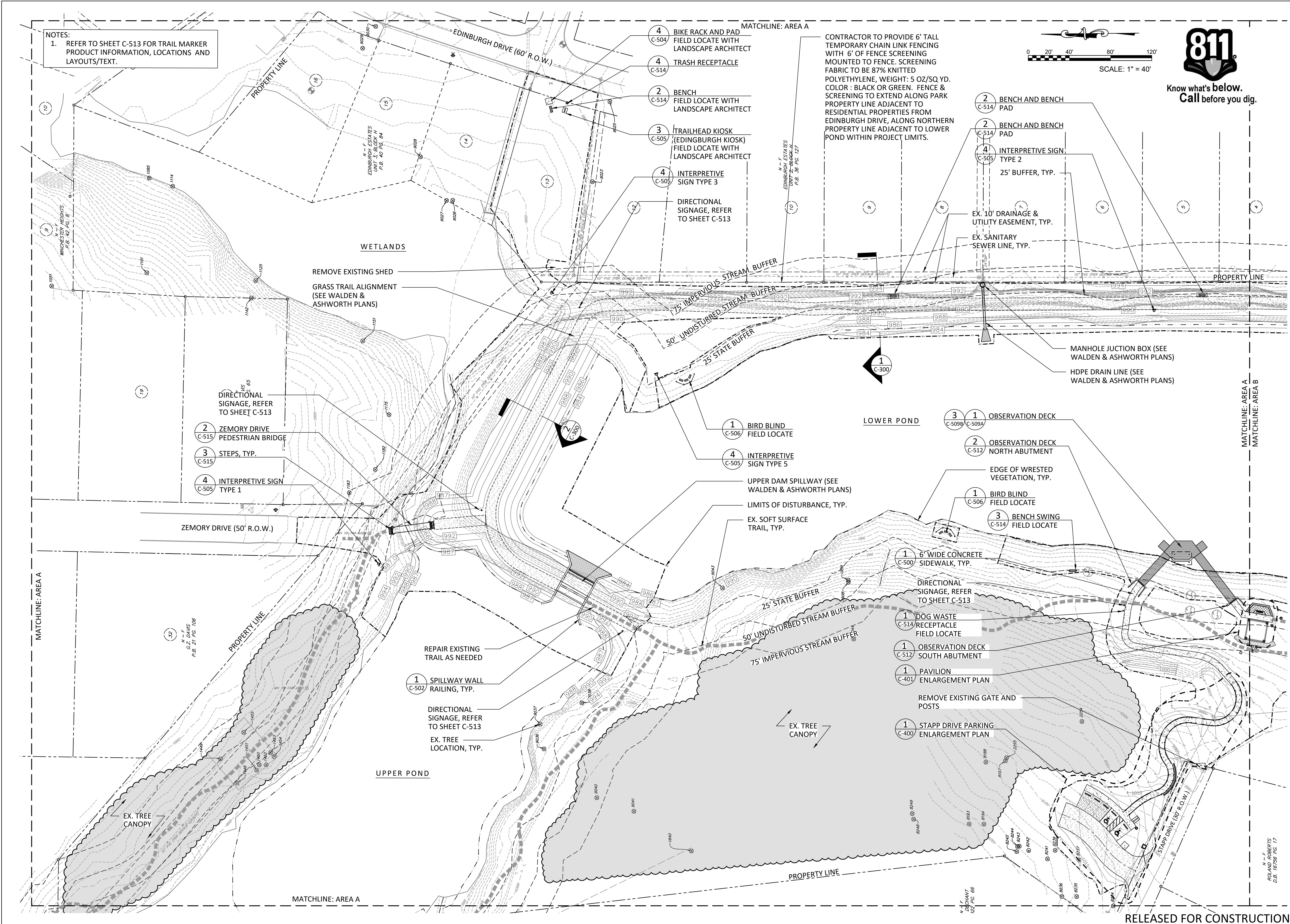


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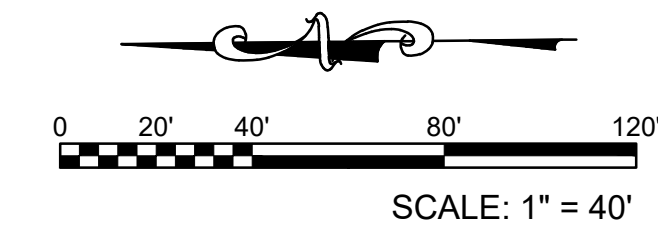


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NOTES:  
 1. REFER TO SHEET C-513 FOR TRAIL MARKER PRODUCT INFORMATION, LOCATIONS AND LAYOUTS/TEXT.



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 Project No: 2022-024  
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**Johns Homestead Park & Dam Improvements**  
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 4898 Lavista Road  
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Revisions:

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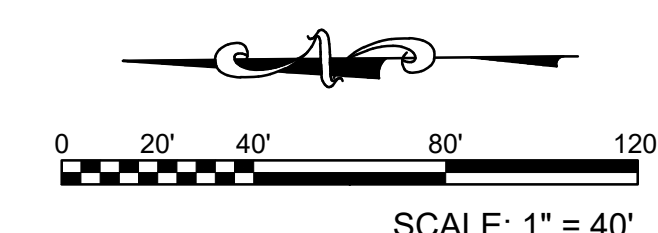
Sheet Title:  
**Site Plan: Area A**

Sheet No:  
**C-100**

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MATCHLINE: AREA B



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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Site Plan: Area B**

Sheet No:  
**C-101**

CONTRACTOR TO PROVIDE 6' TALL TEMPORARY CHAIN LINK FENCING WITH 6' OF FENCE SCREENING MOUNTED TO FENCE. SCREENING FABRIC TO BE 87% KNITTED POLYETHYLENE, WEIGHT: 5 OZ/SQ. YD. COLOR : BLACK OR GREEN. FENCE & SCREENING TO EXTEND ALONG PARK PROPERTY LINE ADJACENT TO RESIDENTIAL PROPERTIES FROM EDINBURGH DRIVE, ALONG NORTHERN PROPERTY LINE ADJACENT TO LOWER POND WITHIN PROJECT LIMITS.

N = F  
EDINBURGH WOODS  
UNIT 1, BLOCK A  
P.B. 60 PG. 34



MATCHLINE: AREA A  
MATCHLINE: AREA B

MATCHLINE: AREA B

MATCHLINE: AREA B

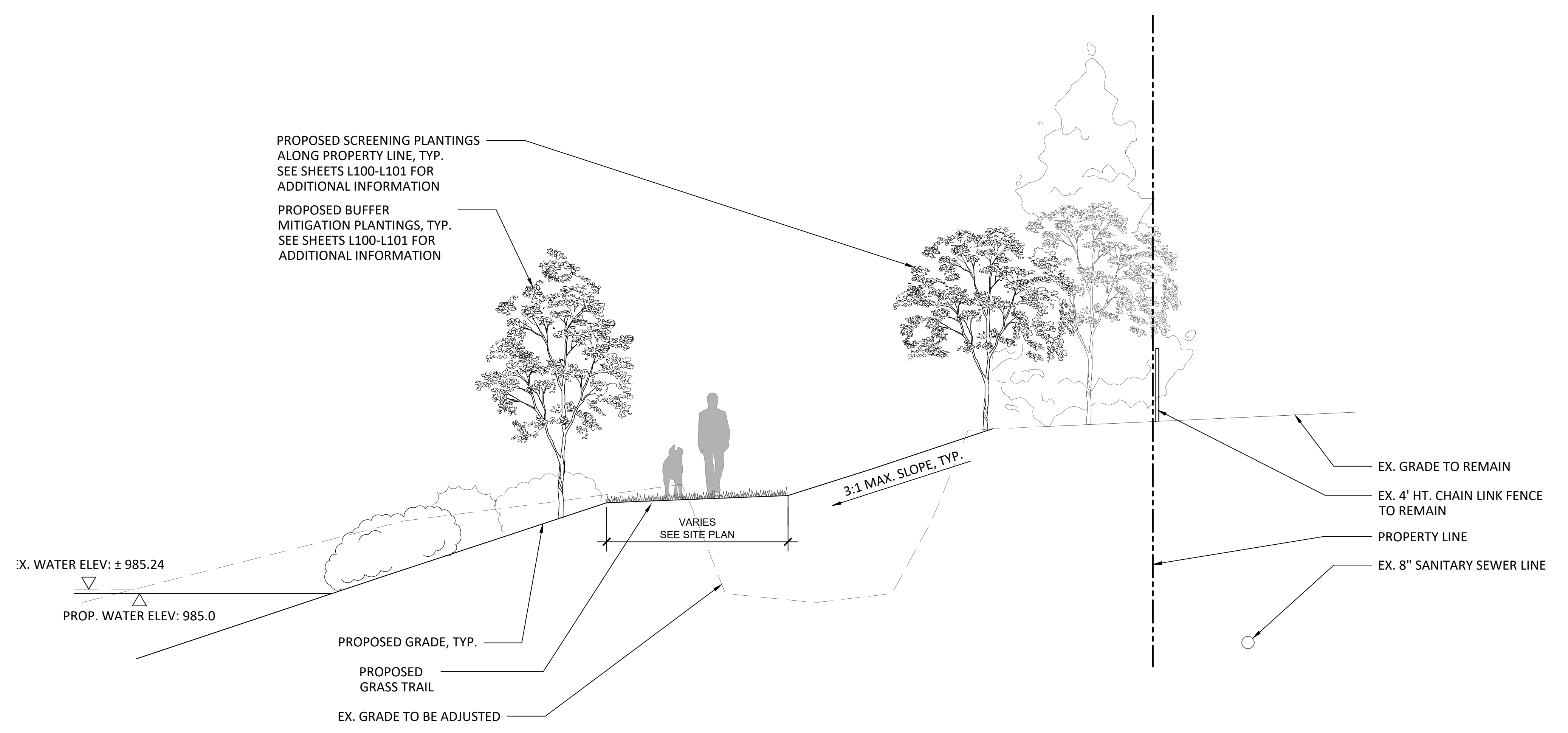
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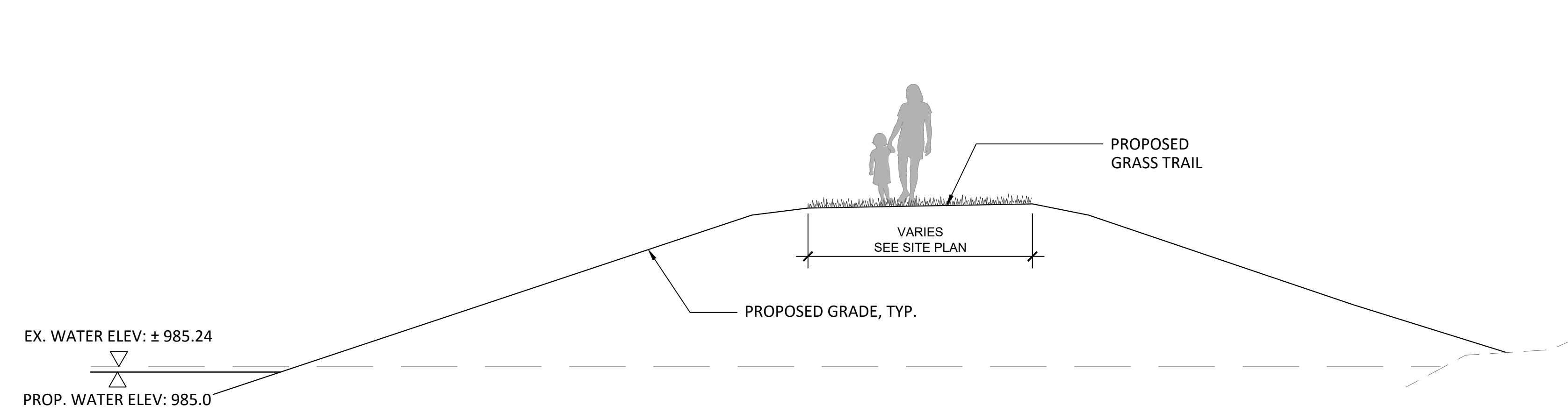


Date: 07/16/2024  
Project No: 2022-024  
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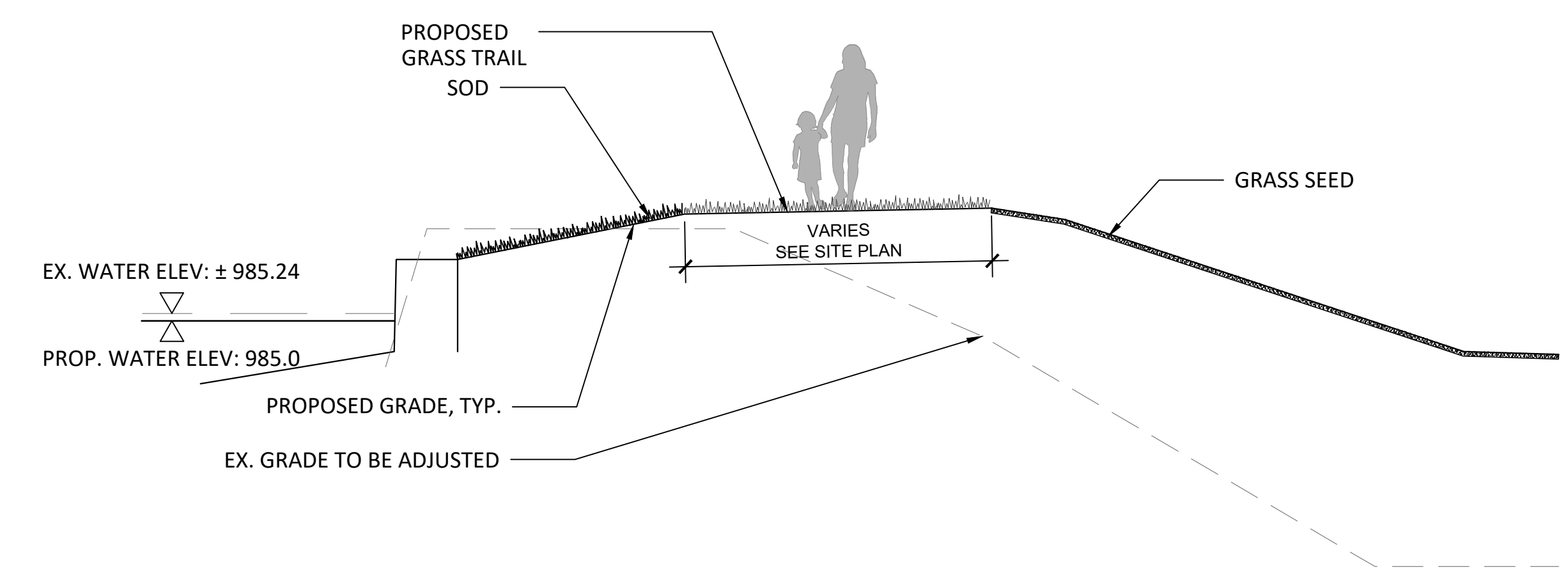
**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
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Tucker, GA 30084



**1** SECTION THROUGH EAST BANK OF LOWER POND  
SCALE: 1/4" = 1'-0"



**2** SECTION THROUGH NORTH BANK OF LOWER POND  
SCALE: 1/4" = 1'-0"



**3** SECTION THROUGH SOUTH BANK OF LOWER POND  
SCALE: 1/4" = 1'-0"

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Trail Sections**

Sheet No:  
**C-300**

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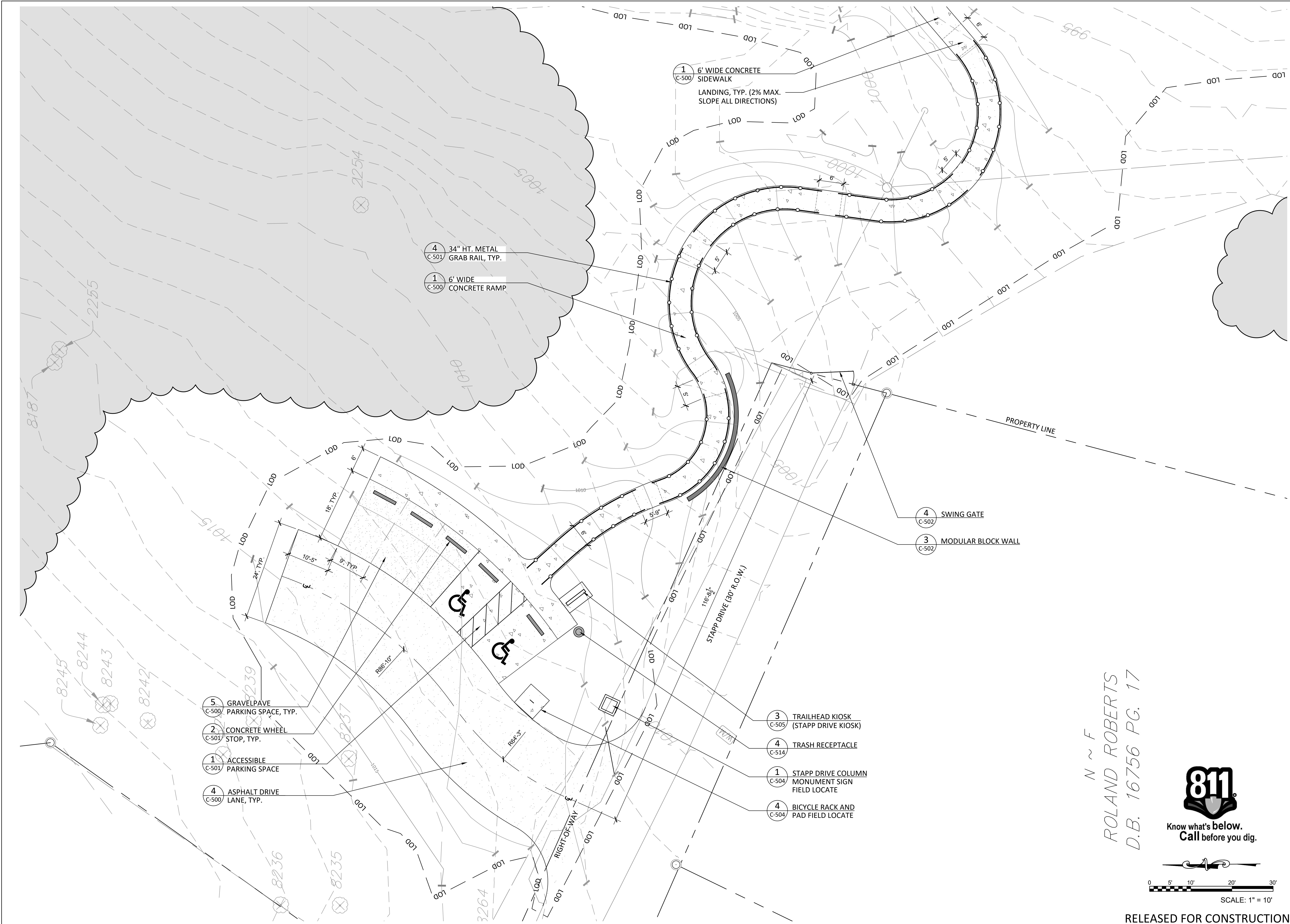
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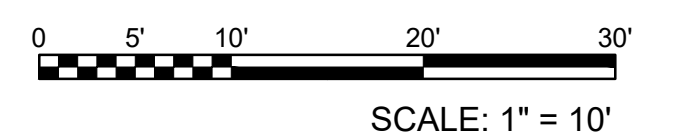
NO.	DATE	DESCRIPTION

Sheet Title:  
**Stapp Drive Parking Enlargement Plan**

Sheet No:  
**C-400**

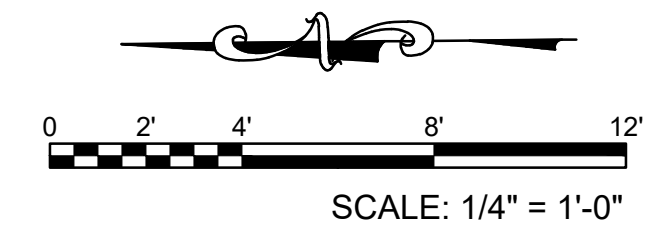


N ~ F  
ROLAND ROBERTS  
D.B. 16756 PG. 17

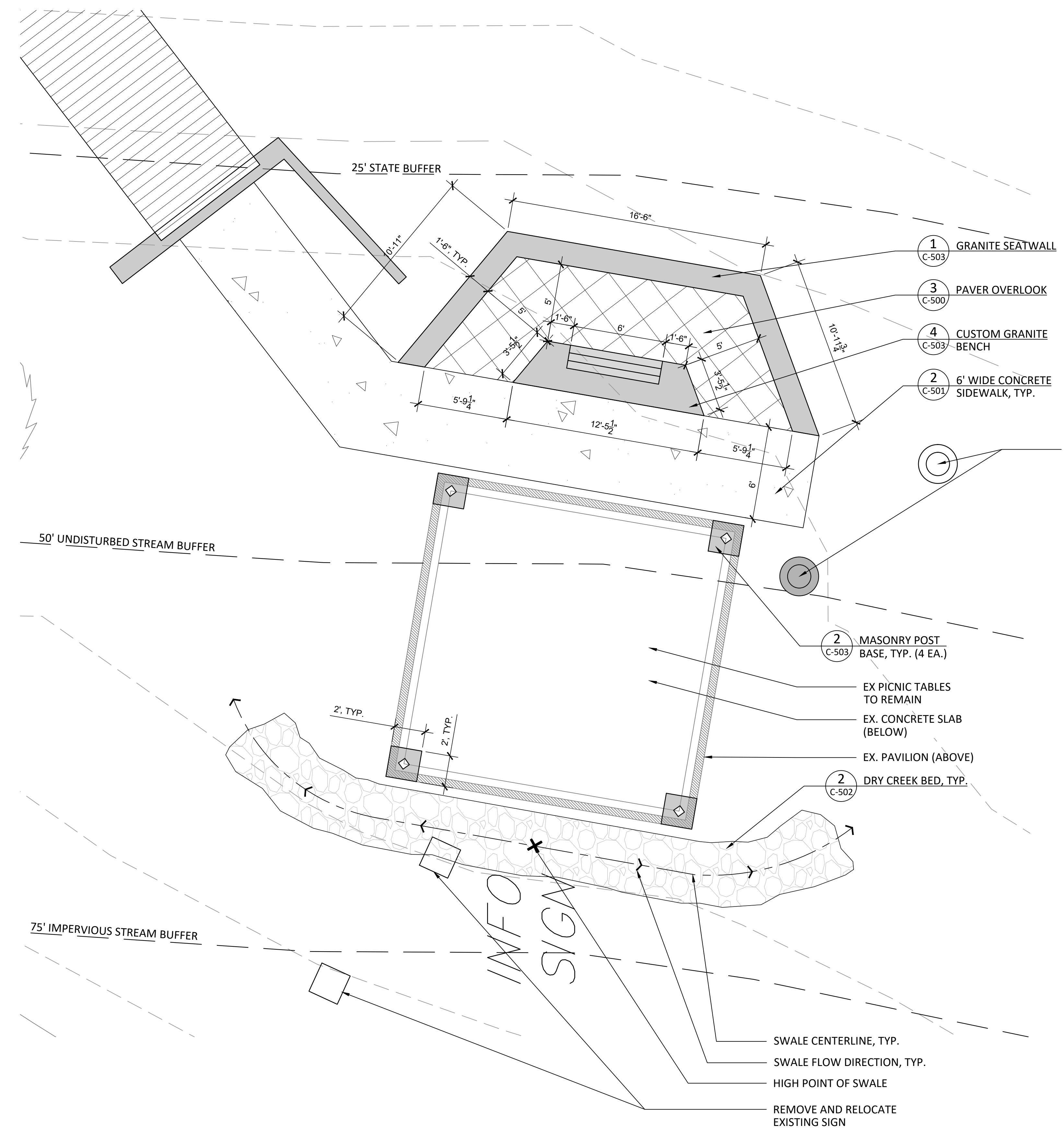


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- ① GRANITE SEATWALL  
C-503
- ③ PAVER OVERLOOK  
C-500
- ④ CUSTOM GRANITE BENCH  
C-503
- ② 6' WIDE CONCRETE SIDEWALK, TYP.  
C-501

- ② MASONRY POST BASE, TYP. (4 EA.)  
C-503
- EX PICNIC TABLES TO REMAIN
- EX. CONCRETE SLAB (BELOW)
- EX. PAVILION (ABOVE)
- ② DRY CREEK BED, TYP.  
C-502

- SWALE CENTERLINE, TYP.
- SWALE FLOW DIRECTION, TYP.
- HIGH POINT OF SWALE
- REMOVE AND RELOCATE EXISTING SIGN

**Johns Homestead Park & Dam Improvements**  
 City of Tucker Parks and Recreation  
 4898 Lavista Road  
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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Pavilion Enlargement Plan**

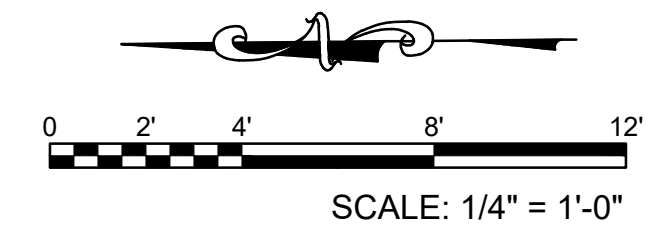
Sheet No:  
**C-401**

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**NOTES**

- COORDINATE WITH LANDSCAPE ARCHITECT FOR FINAL LAYOUT AND ALIGNMENT PRIOR TO CONSTRUCTION.
- BRIDGE DIMENSIONS:  
BRIDGE 1 15' LONG  
BRIDGE 2 15' LONG  
  
BRIDGE 1 4' WIDE  
BRIDGE 2 4' WIDE

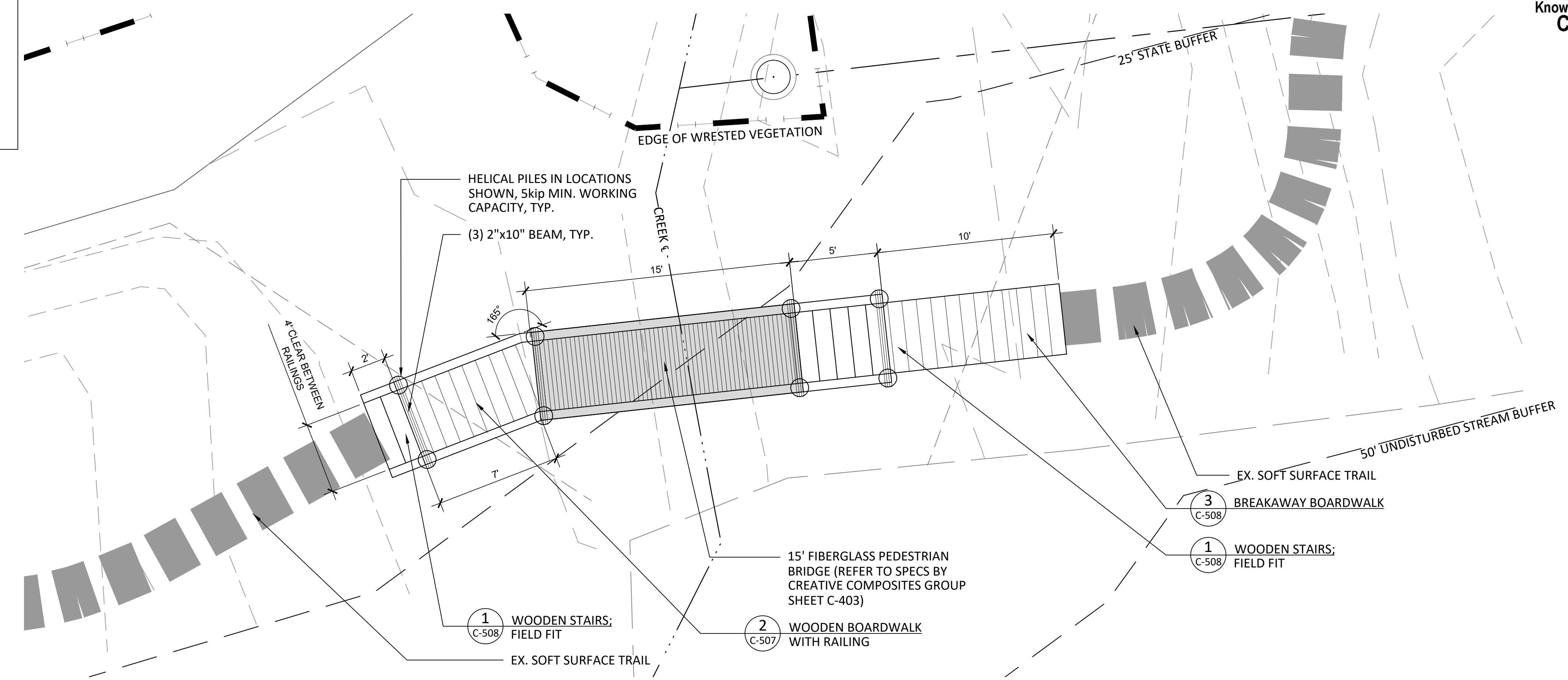


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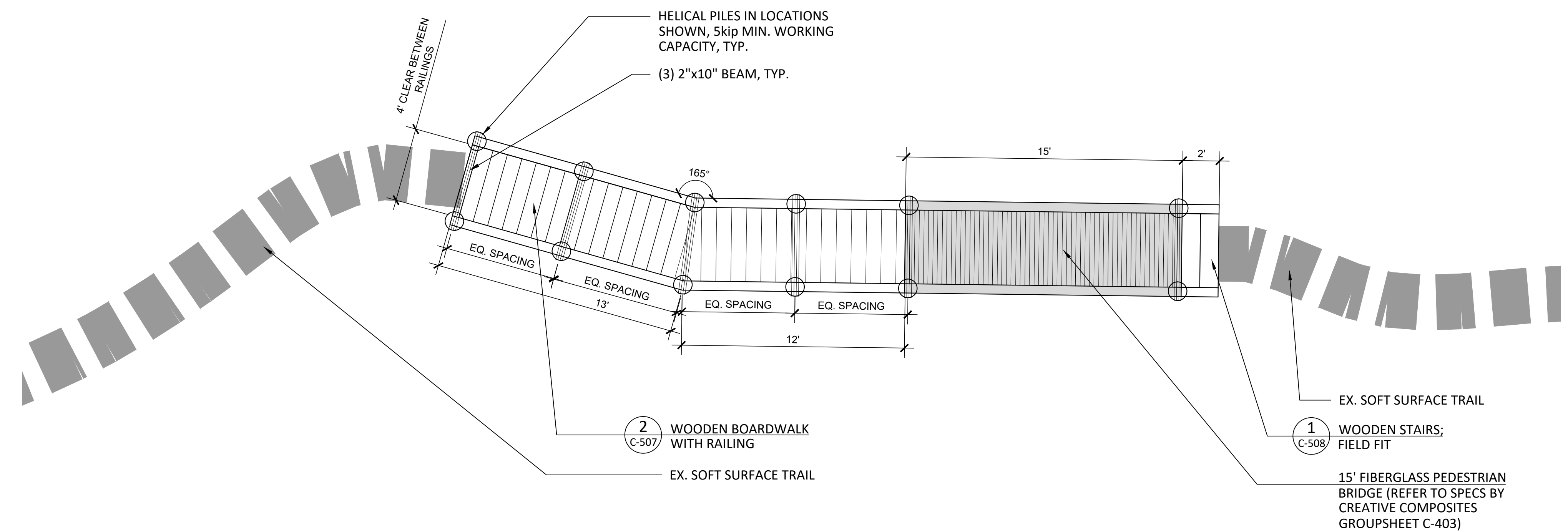
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Project No: 2022-024  
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Checked By: MK



**1 SOUTH BOARDWALK / BRIDGE CROSSING ENLARGEMENT PLAN**  
SCALE: 1/4" = 1'-0"



**2 NORTH BOARDWALK / BRIDGE CROSSING ENLARGEMENT PLAN**  
SCALE: 1/4" = 1'-0"

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Boardwalk / Bridge Crossings Enlargement Plan**

Sheet No:  
**C-402**

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## Prefabricated Bridge Specifications International Building Code (IBC)

### 1.0 GENERAL

#### 1.1 Scope

These specifications are for a fully engineered clear span bridge of Fiber Reinforced Polymer (FRP) composite construction and shall be regarded as the minimum standards for design and construction. The FRP structure shall be manufactured by Creative Pultrusions, Inc. (CPI), 214 Industrial Lane, Alum Bank, PA, phone 814-839-4186 (Toll-free 888-274-7855), or approved equal.

#### 1.2 Qualified Suppliers

The FRP bridge manufacturer shall be an ISO9001:2015 accredited company for the design and manufacture of FRP structural components and systems. The company shall have been in the business of design and fabrication of bridges for a minimum of ten years. Company shall provide a list of five successful bridge projects, of similar construction, each of which has been in service at least three years. List the location, bridge size, owner and contact reference for each bridge.

### 2.0 GENERAL FEATURES OF DESIGN

#### 2.1 Span

Bridge span will be 15' 0" (straight line dimension) and shall be measured from each end of the bridge structure.

#### 2.2 Width

Bridge width shall be 4' 0" and shall be measured from the inside face of structural elements at deck level.

#### 2.3 Bridge System Type

Bridge must be designed as a FRP Composite Truss Span.

#### 2.4 Member Components

All members shall be fabricated from pultruded FRP composite profiles and structural shapes as required.

#### 2.5 Camber

Bridges to be precambered to eliminate initial dead load deflection.

### 3.0 ENGINEERING

Structural design of the bridge structure(s) shall be performed by or under the direct supervision of a Licensed Professional Engineer and done in accordance with recognized engineering practices and principles.

#### 3.1 Uniform Live Load

All bridge spans to be designed for 90 psf.

#### 3.2 Vehicle Load (as required)

A specified vehicle configuration determined by the Operating Agency may be used for the design vehicle. If an Agency design vehicle is not specified, the loads conforming to the AASHTO Standard H-5 Truck is used. A 4'x4' wheel base is assumed. Wheel loads are distributed per AASHTO unless otherwise specified. The maintenance vehicle live load shall not be placed in combination with the pedestrian live load. A vehicle impact allowance is not required.

### 3.0 ENGINEERING (cont'd)

#### 3.3 Wind Load

All bridges shall be designed for a minimum wind load of 25 psf. Wind loading is calculated based on Publication ASCE 7. The wind is calculated on the entire vertical surface of the bridge as if fully enclosed.

#### 3.4 Seismic Load

Seismic loads shall be determined according to the criteria specified in the building codes unless otherwise requested. Response Spectrum Analysis shall be performed in those designs that require complex seismic investigation. All necessary response spectra information will be provided by the client for evaluation.

#### 3.5 Design Approach

An Allowable Stress Design (ASD) approach is used for the design of all structural members. Factors of safety used by CPI in the design of FRP bridges are as follows unless otherwise specified: (Based on the Ultimate Strength of the FRP material):

Tension	2.5
Compression	2.5
Shear	2.5
Bending	2.5
End Bearing	2.5
Connections	3.0

Above information is based on E.T. Technotics 5 year test program funded by the National Science Foundation.

#### 3.6 Serviceability Criteria

Service loads are used for the design of all structural members when addressing deflection and vibration issues. Criteria used by CPI in the design of FRP bridges are as follows:

Deflection:  
Live Load (L) deflection = L/360

Dead Load (D) + Live Load (L) deflection = L/240

Lateral Wind Load (W) deflection = L/360

Vertical Frequency (fn): = 5.0 Hz

The fundamental frequency of the pedestrian bridge (in the vertical direction) without live load should be greater than 5.0 hertz (Hz) to avoid any issues with the first and second harmonics.

Horizontal Frequency (fn): = 3.0 Hz

The fundamental frequency of the pedestrian bridge, in the horizontal direction without live load, should be greater than 3.0 hertz (Hz) to avoid any issues due to side to side motion involving the first and second harmonics.

#### 3.7 Snow Load

Sustained snow load conditions shall be evaluated for time dependent effects (creep and relaxation) and expected recovery behavior. The use of ground snow loads per ASCE 7 shall be applied to the structure with consideration of local conditions.

3.

### 4.0 MATERIALS

#### 4.1 FRP Composites

FRP bridges shall be fabricated from high-strength E-glass and isophthalic polyester resin unless otherwise specified. Weathering and ultraviolet light protection shall be provided by addition of a veil to the laminate construction. Minimum characteristic design strengths, per ASTM D7290 are as follows:

	CH662	CH860	TQ240	
Tension (LW) (psi)	67,236	59,150	63,968	ASTM D638
Compression (LW) (psi)	71,285	70,888	43,363	ASTM D6641
Shear (In-plane) (psi)	9,954	9,773	9,977	ASTM D5379
Shear (interlaminar) (psi)	4,442	3,969	4,189	ASTM D2344
				ASTM D6641 & D638
Young's Modulus (LW) (psi)	4.35E+06	4.41E+06	3.87E+06	(Taken as the mean of the lessor of the two)

The minimum thickness of FRP Composite shapes shall be as follows unless otherwise specified: Square tube members (closed type shape) shall be 0.25 in. Wide-flange beams, channel sections, and angles (open type shapes) shall be a minimum thickness of 0.25 in. Standard plate shall be a minimum thickness of 0.25 in.

#### 4.2 Decking FRP Connections

All FRP bridge connections shall be classified as concentric bolt bearing & contain at least (2) bolts for load transfer.

All connections shall be experimentally determined via full section joint component testing.

- Tubes/solids shall be investigated @ 0-deg. with 3/4" A307 bolts
- Channels shall be investigated @ 0,45,90-deg. with 3/4" A307 bolts
- Channels shall be investigated @ 45,90 -deg. with 3/8" plate & 3/4" A325 bolts
- All tube/solid capacities shall utilize a tube with bonded solid plug in the bearing area
- All channel capacities shall utilize (2) channels
- Tests with plates shall use a section of channel as the doubler plate in the bearing area
- All capacities shall be measured utilizing (2) bolts except CH860 0 -deg. (3 bolts)

The FRP bridge manufacturer shall provide test data showing the joint configurations achieve all stipulated characteristic values when analyzed in accordance with ASTM D7290. The minimum allowable characteristic values are as follows:

Component / Orientation	Characteristic Value
TQ240/SQ024 @ 0 -deg. (Compression)	73,433 lbs.
TQ240/SQ024 @ 0 -deg. (Tension)	29,186 lbs.
CH662 @ 0 - deg. (Tension)	33,886 lbs.
CH662 @ 90 - deg. (Tension)	33,253 lbs.
CH662 @ 90 - deg. w/ 3/8" Plate (Tension)	51,094 lbs.
CH662 @ 45 - deg. (Tension & Compression)	36,018 lbs.
CH662 @ 45 - deg. w/ 3/8" Plate (Tension & Compression)	65,810 lbs.
CH860 @ 0 - deg. (Tension)	50,151 lbs.
CH860 @ 90 - deg. (Tension)	30,439 lbs.
CH860 @ 90 - deg. w/ 3/8" Plate (Tension)	53,981 lbs.
CH860 @ 45 - deg. (Tension)	39,247 lbs.
CH860 @ 45 - deg. w/ 3/8" Plate (Tension)	72,439 lbs.
CH860 @ 45 - deg. (Compression)	48,886 lbs.
CH860 @ 45 - deg. w/ 3/8" Plate (Compression)	62,308 lbs.

TQ240 = 2"x2"x1/4" Tube SQ024 = 1.5"x1.5"x.5" Solid  
CH662 = 6"x1-11/16"x3/8" Channel CH860 = 8"x2-3/16"x3/8" Channel

4.

### 4.0 MATERIALS (cont'd)

#### 4.3 Decking

Wood decking is No. 2 Southern Yellow Pine treated according to the American Wood Preservers Bureau. Standard 3 in. x 12 in. (nominal) planks can be provided for equestrian and light vehicle type loading conditions as required. High strength fiberglass decking can also be provided as required.

#### 4.4 Hardware

Bolted connections shall be A307 hot-dipped galvanized steel unless otherwise specified. Mounting devices shall be galvanized or stainless steel.

### 5.0 SUBMITTALS

#### 5.1 Submittal Drawings

Schematic drawings and diagrams shall be submitted to the client for their review after receipt of order. As required, all drawings shall be signed and sealed by a licensed Professional Engineer.

#### 5.2 Submittal Calculations

As required, structural calculations shall be submitted to the client. All calculations will be signed and sealed by a licensed Professional Engineer.

### 6.0 FABRICATION

#### 6.1 General

Fabrication of the bridge structure shall be per ANSI/ACMA Code of Standard Practice, Industry Guidelines for Fabrication and Installation of Pultruded FRP Structures unless otherwise noted

#### 6.1 Tolerances

All cutting and drilling fabrication to be done by experienced fiberglass workers using carbide or diamond-tipped tooling to a tolerance of 1/16". No material deviations beyond industry standards are accepted. All cut edges to be cleaned and sealed.

#### 6.2 Profile Tolerances

Pultruded profiles shall be manufactured to the dimensional requirements as set forth in ASTM D3917 and the visual requirements as set forth in ASTM D4385.

### 7.0 RAILINGS

#### 7.1 Railings

Railings should be a minimum of 42" above the floor deck for pedestrian and bicycle use and should be a minimum of 54" above the floor deck for equestrian use.

#### 7.2 Safety Rails

Continuous horizontal safety rails of 3" channel shall be located on the inside of the trusses. The maximum opening between the safety rails shall not be greater than 4". If preferred, vertical pickets can be provided upon request.

#### 7.3 Toeplates (optional)

Continuous horizontal toe plates of 3" channel shall be located on the inside of the trusses near deck level.

5.

### 8.0 FINISHING

Bridge color shall be determined by the client with green (1500 series) and grey (1525 series) as standard options. No painting is required as the color is added during the manufacturing process. Green is recommended for park and trail bridge applications. Custom colors can be provided upon request for an additional setup fee.

### 9.0 DELIVERY AND ERECTION

Delivery is made by truck to a location nearest the site accessible by roads. CPI will notify the client in advance of the expected time of arrival at the site. Bridges are usually shipped to the site in component parts or partially assembled depending on site requirements. The spans can then be completely assembled using standard hand tools. Upon request, bridges can also be shipped totally assembled to the site. Unloading, splicing (if required) and placement of the bridge will be the responsibility of the client.

#### 9.1 Erection Direction

For bridges shipped in component parts or partially assembled, CPI shall provide assembly drawings and a recommended assembly procedure for building the bridge. Temporary supports or rigging equipment, if needed, is the responsibility of the client. For bridges shipped assembled, CPI shall advise the client of the actual lifting weights, attachment points and all necessary information to install the bridge.

#### 9.2 Site Issues and Foundation Design

The client shall procure all necessary information about the site and soil conditions. Soil tests shall be procured by the client. The engineering design and construction of the bridge abutments, piers and/or footing shall be by the client. CPI will provide the necessary information pertaining to the bridge support reactions. The client shall install the anchor bolts in accordance with CPI's anchor bolt spacing dimensions.

### 10.0 WARRANTY

The Bridge Manufacturer shall warrant the structural integrity of all FRP materials, design, and workmanship for 15 years. This warranty shall not cover defects in the bridge caused by foundation failures, abuse, misuse, overloading, accident, faulty construction or alteration, or other cause not the result of defective materials or workmanship. This warranty shall be limited to the repair or replacement of structural defects and shall not include liability for consequential or incidental damages.

Creative Pultrusions, Inc. reserves the right to edit and modify literature, please consult the web site for the most current version of this document.

ETTechnotics.com  
(888) CPI - PULL (274-7855) Ext. 265

ETPPS101816  
01.11.21

Seal:



Date: 07/16/2024

Project No: 2022-024

Drawn By: PS

Checked By: MK

Johns Homestead Park &  
Dam Improvements  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:

Pedestrian Bridge  
Specifications

Sheet No:

C-403

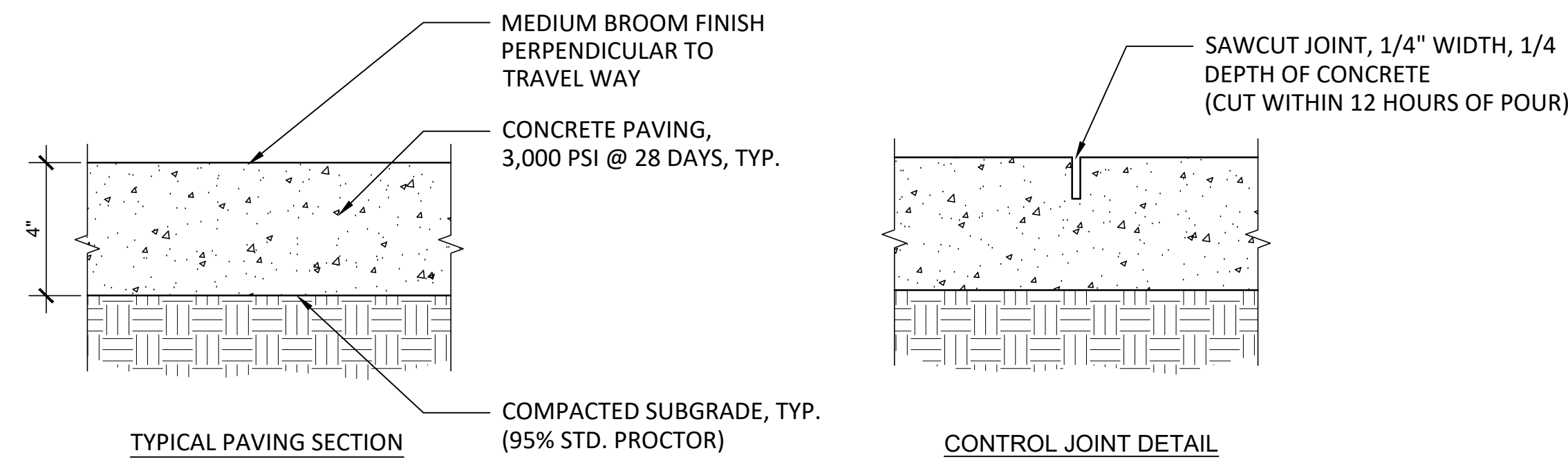
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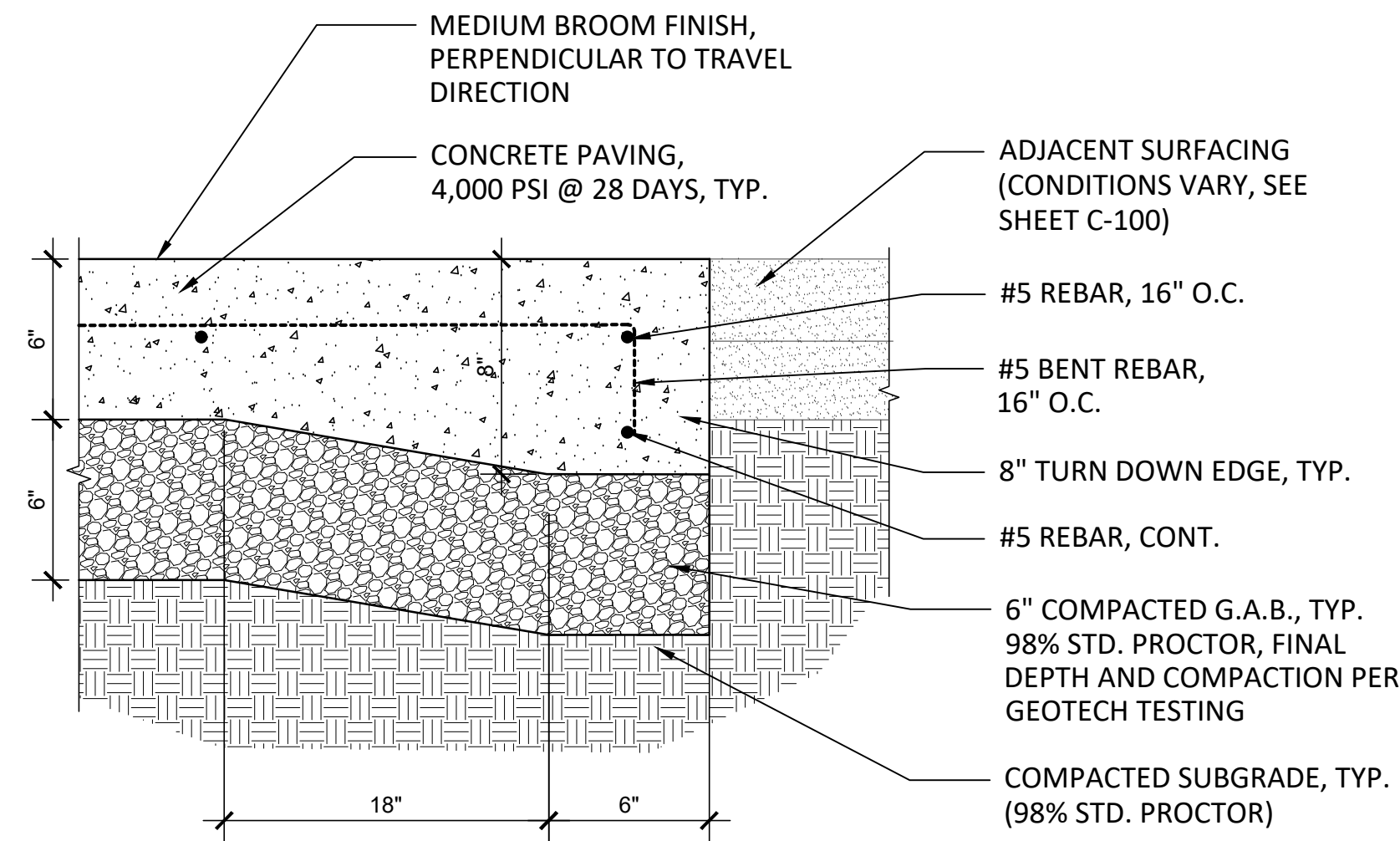


Date: 07/16/2024  
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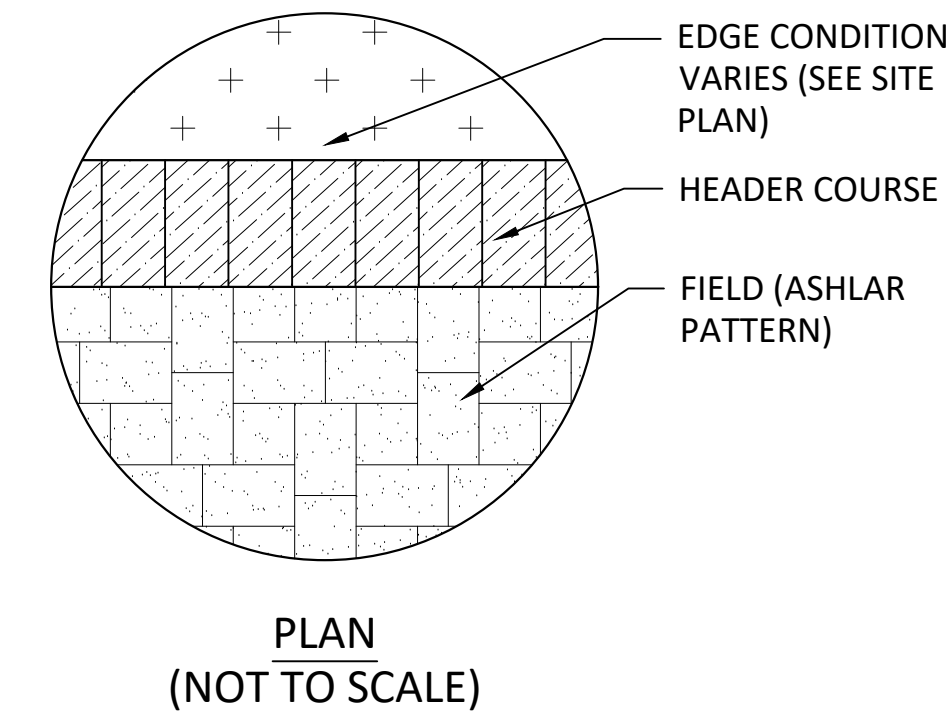
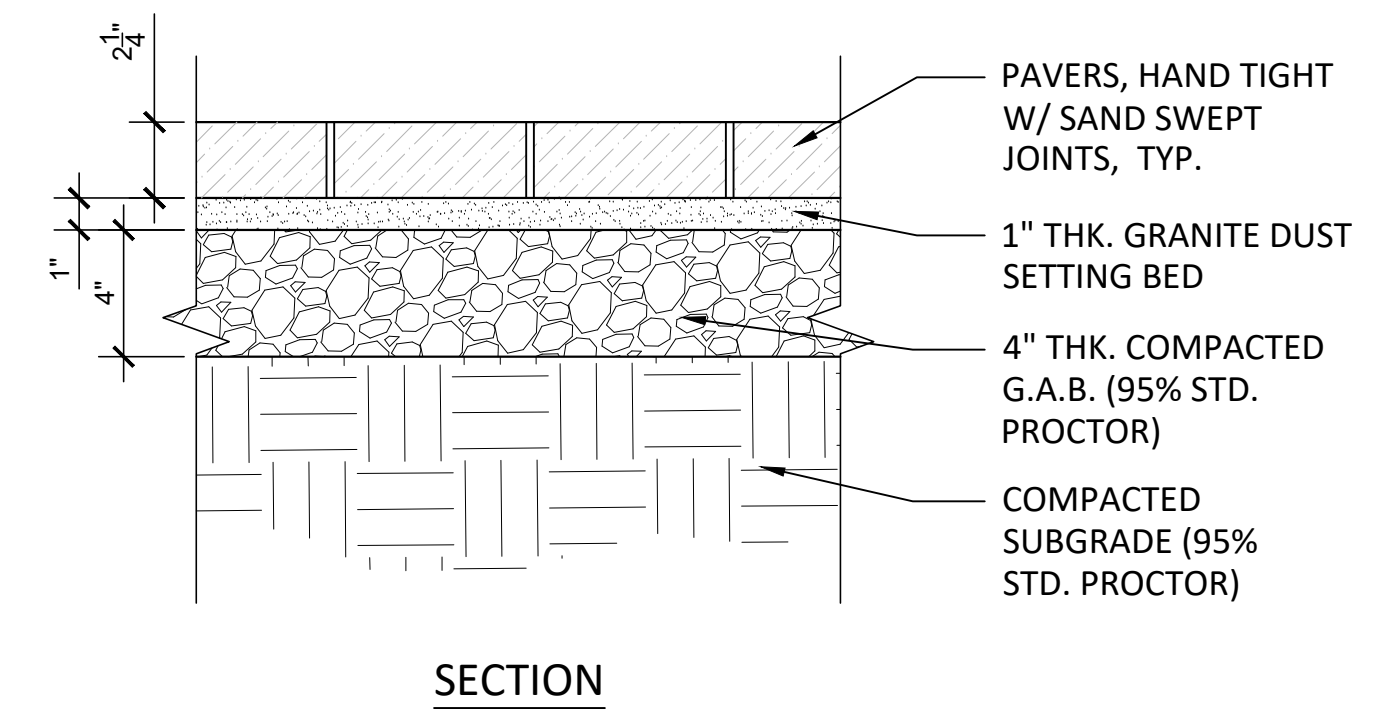
**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084



- CONCRETE PAVING NOTES:**
- PAVEMENT WIDTH VARIES. REFER TO SITE PLAN.
  - CONTROL JOINT SPACING EQUAL TO WIDTH OF WALK UNLESS OTHERWISE INDICATED ON THE PLANS.
  - EXPANSION JOINTS @ 40' MAX., AND WHEN ABUTTING ADJACENT RIGID PAVEMENTS, CURBS, AND STRUCTURES, OR AS SHOWN ON PLANS.
  - INSTALL A TEMPORARY SNAP/CAP OVER THE TOP OF THE EXPANSION JOINT AFTER THE EXPANSION JOINT BOARD IS INSTALLED. ONCE CURED, REMOVE THE CAP, INSTALL BACKER ROD, AND CAULK THE TOP OF JOINT.
  - ALL JOINTS SHALL BE PERPENDICULAR WITH EDGES OF WALK. IF WALK IS CURVED, JOINTS SHALL EXTEND FROM RADIUS POINT.
  - CONCRETE TO BE 3,000 PSI @ 28 DAYS, AIR ENTRAINED. CONCRETE TO BE LOW CARBON VERTUA BY CEMEX (READY MIX USA) OR EQUIVALENT WITH 30% OR GREATER REDUCTION IN EMISSIONS COMPARED TO STRAIGHT CONCRETE MIX.
  - SEE GENERAL NOTES AND/OR SPECIFICATIONS FOR ADDITIONAL INFORMATION.



**NOTE:**  
CONCRETE TO BE 4,000 PSI @ 28 DAYS, AIR ENTRAINED. CONCRETE TO BE LOW CARBON VERTUA BY CEMEX (READY MIX USA) OR EQUIVALENT WITH 30% OR GREATER REDUCTION IN EMISSIONS COMPARED TO STRAIGHT CONCRETE MIX.  
REFER TO DETAIL 1 / SHEET C-500 FOR ADDITIONAL INFORMATION, INCLUDING CONTROL AND ISOLATION JOINTS.

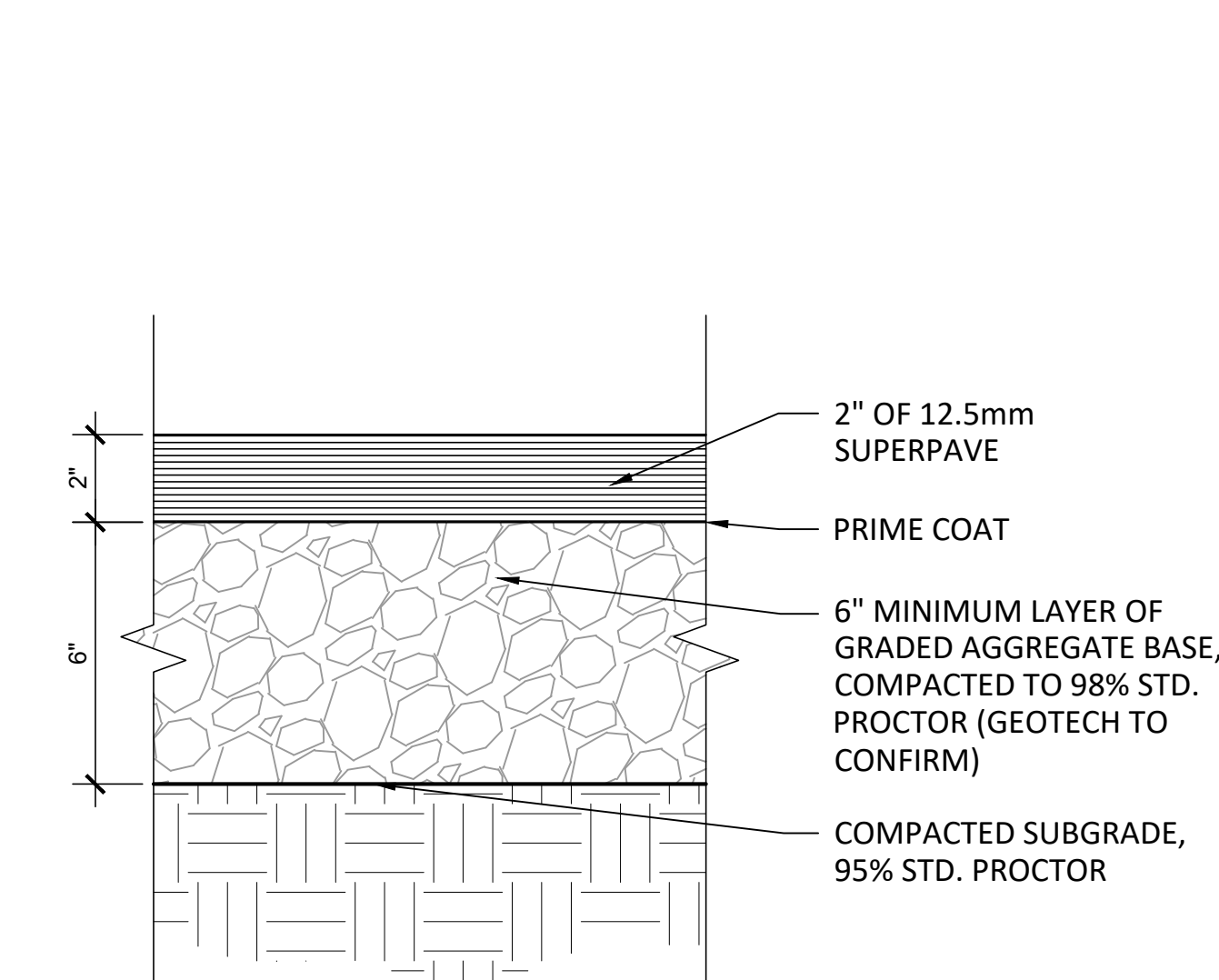


- PAVER NOTE:**
- HEADER COURSE PAVES:
    - Holland Stone by Belgard Pavers (or Approved Equal)
    - 60 MM Thick
    - Gray Granite Color
    - Soldier Course Pattern
  - FIELD PAVES:
    - Dublin Cobble by Belgard Pavers (or Approved Equal)
    - 60 MM Thick
    - Ardennes Grey Color
    - Ashlar Pattern

**1 TYPICAL CONCRETE PAVING**  
SCALE: 3" = 1'-0"

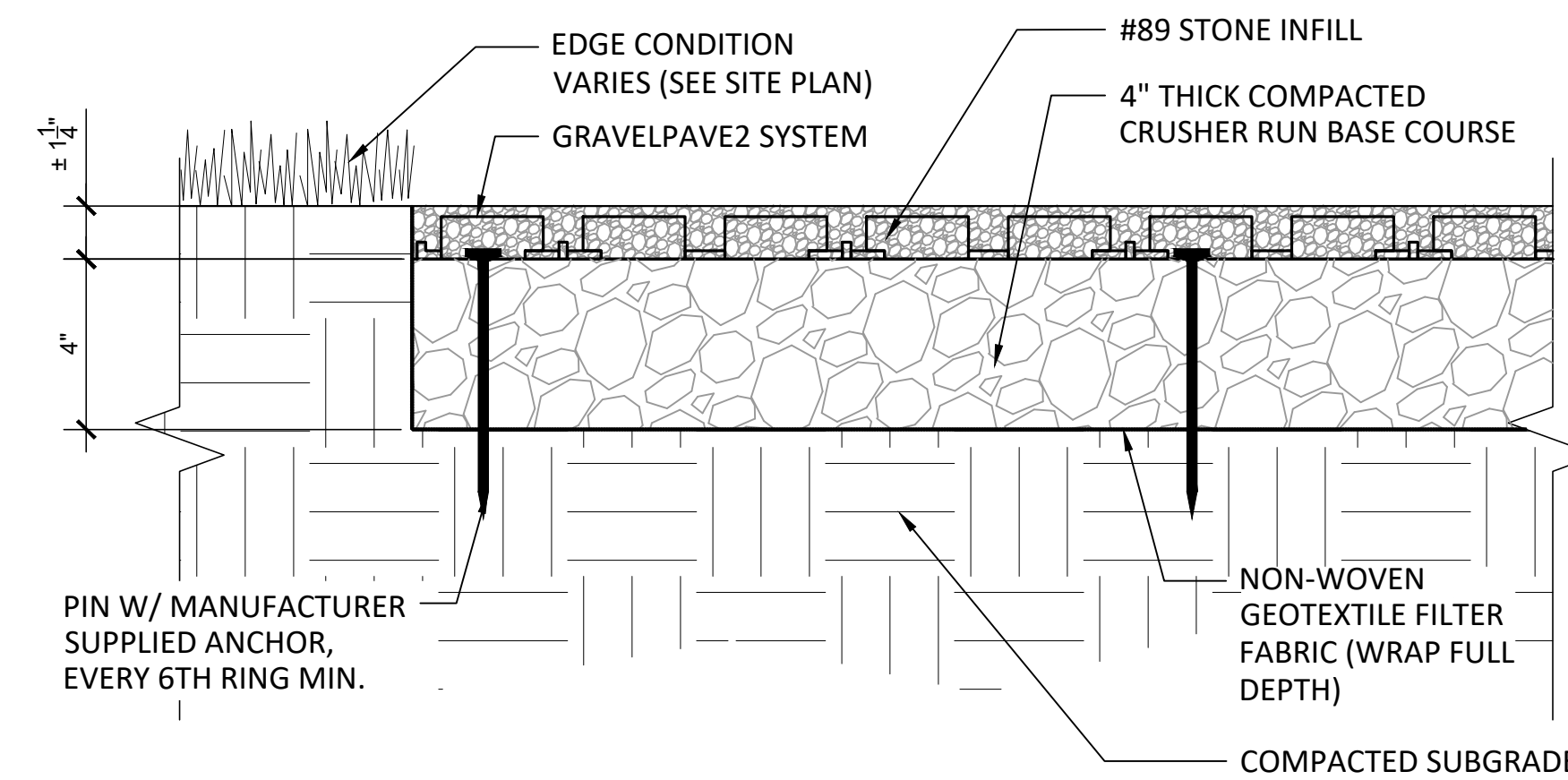
**2 VEHICULAR CONCRETE PAVING**  
SCALE: 2" = 1'-0"

**3 PAVER OVERLOOK DETAIL**  
SCALE: 2" = 1'-0"



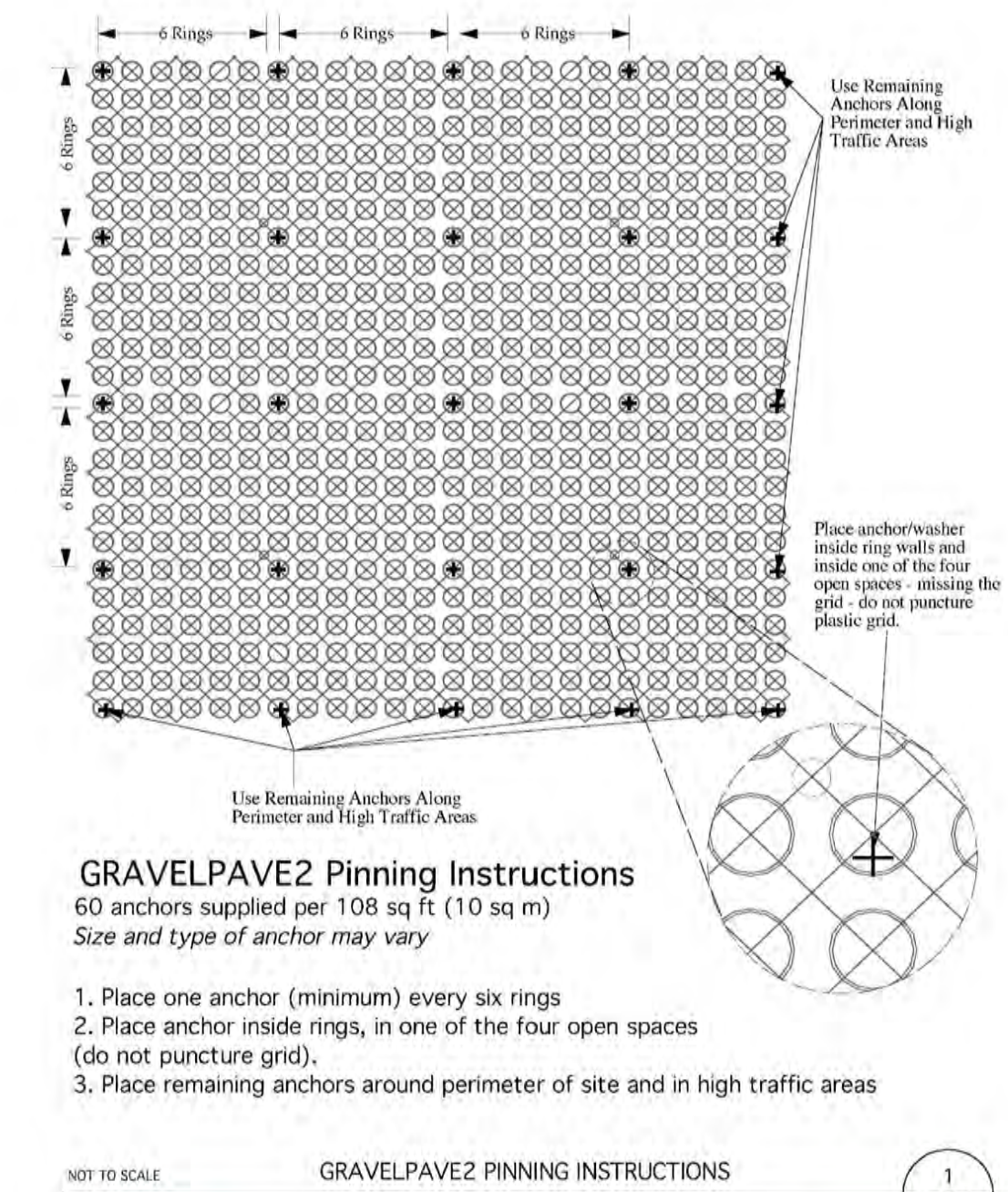
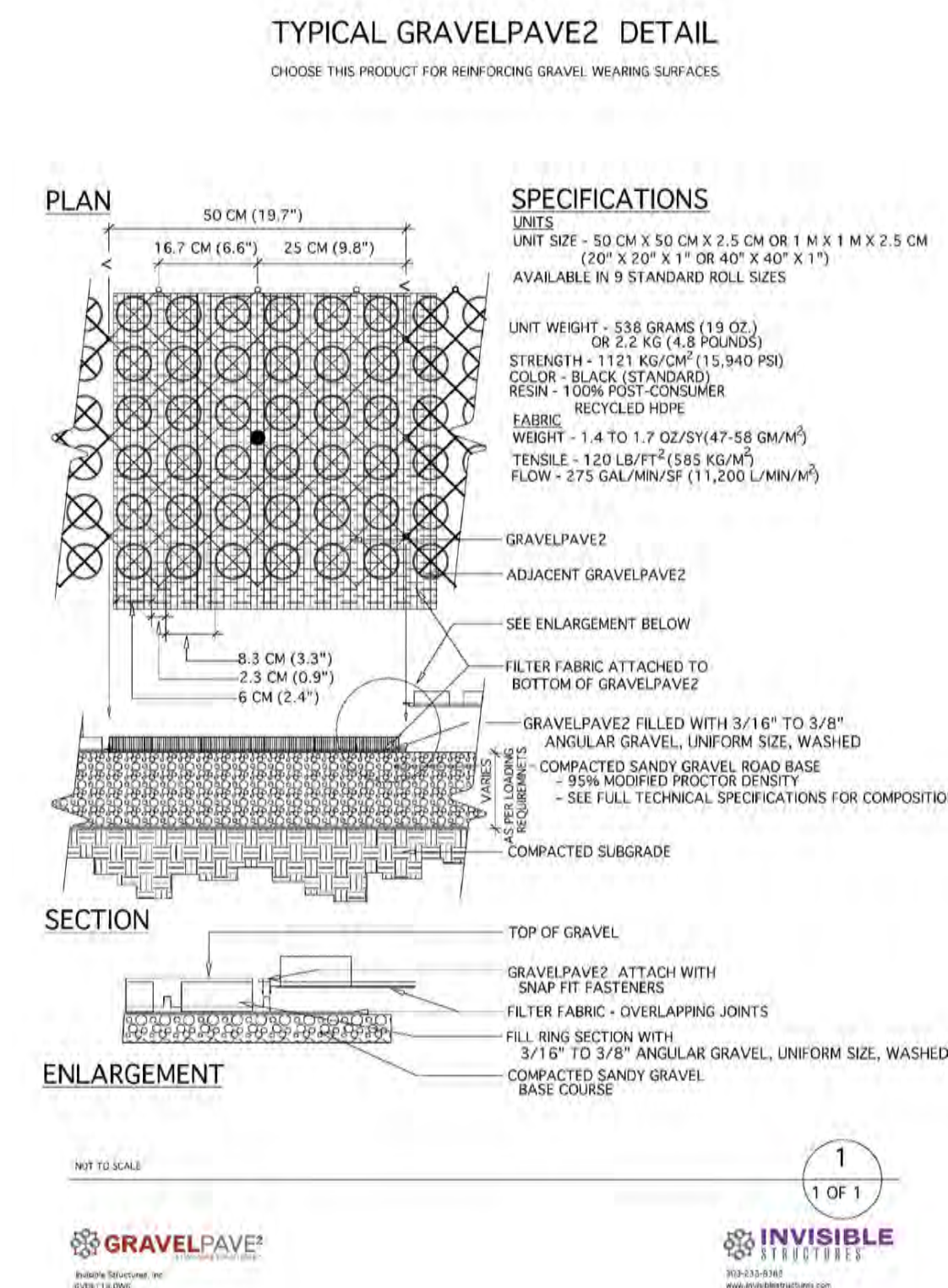
**ASPHALT PAVING NOTE:**  
ALL CONSTRUCTION METHODS AND MATERIALS SHALL BE IN ACCORDANCE WITH THE LATEST GEORGIA DOT SPECIFICATIONS.

**4 ASPHALT DRIVE LANE**  
SCALE: 3" = 1'-0"



- GRAVELPAVE NOTE:**
- INSTALL PER MANUFACTURER'S INSTALLATION GUIDELINES. CONTRACTOR TO PROVIDE ALL PRODUCTS, MATERIALS AND INCIDENTAL ITEMS AS REQUIRED.
  - MANUFACTURER INFO:  
INVISIBLE STRUCTURES  
3510 Himalaya Road, Suite 200  
Aurora, CO 80011  
(303) 233-8383  
www.invisiblestructures.com
  - CONTRACTOR MAY SUBMIT ALTERNATIVE EQUIVALENT PRODUCTS FOR APPROVAL.

**5 GRAVELPAVE PARKING DETAIL**  
SCALE: VARIES



NOT TO SCALE  
GRAVELPAVE2 PINNING INSTRUCTIONS  
1 OF 1



Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
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**C-500**

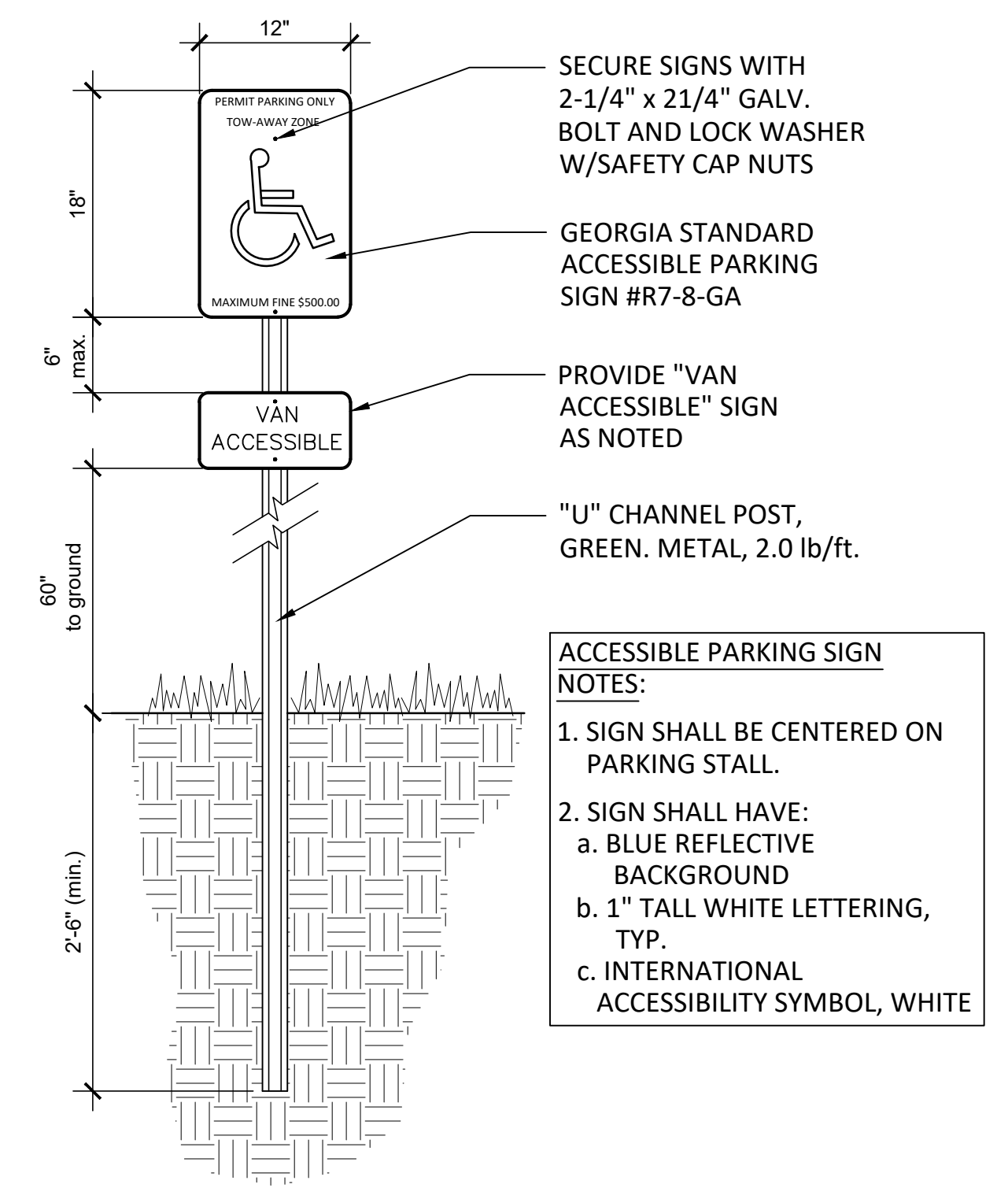
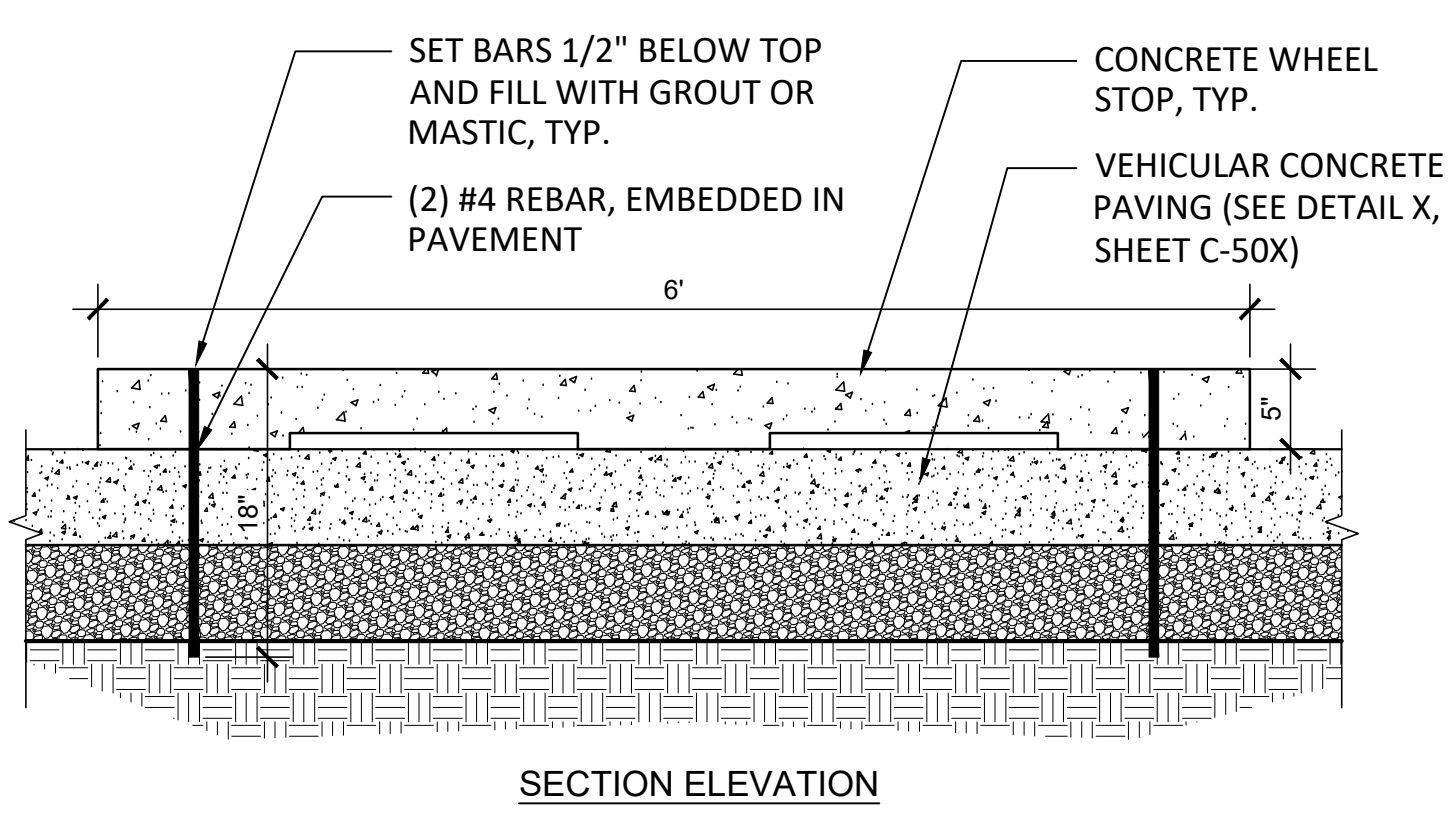
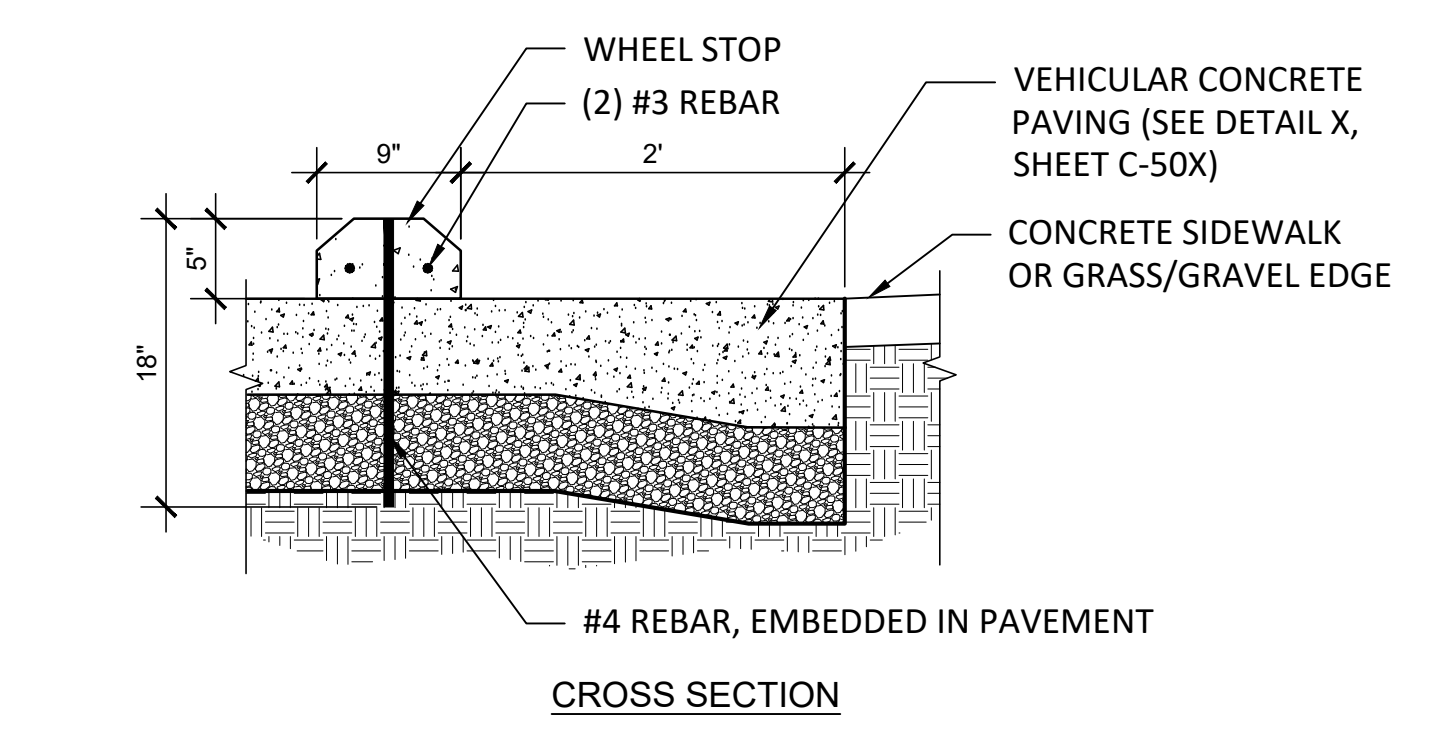
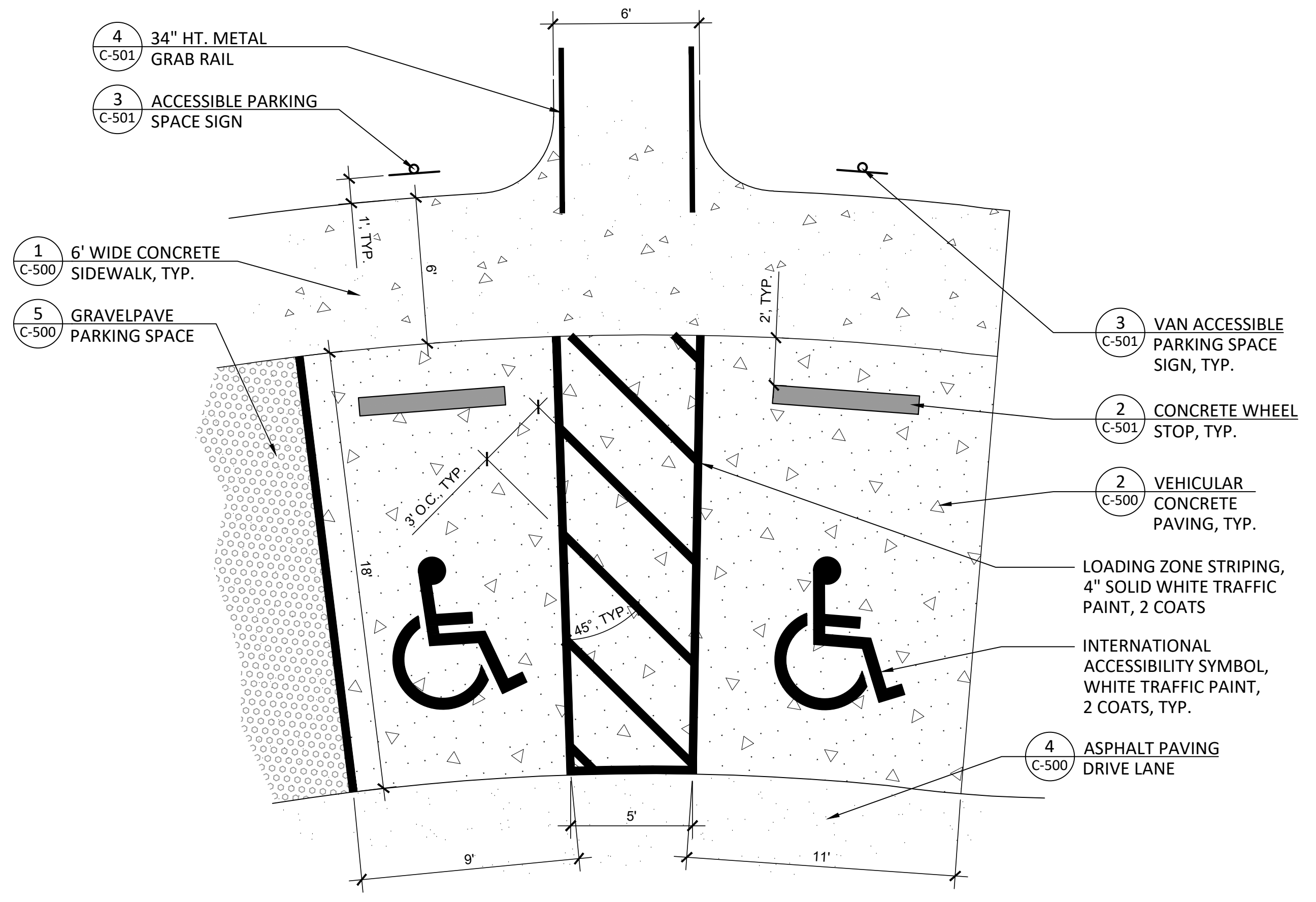


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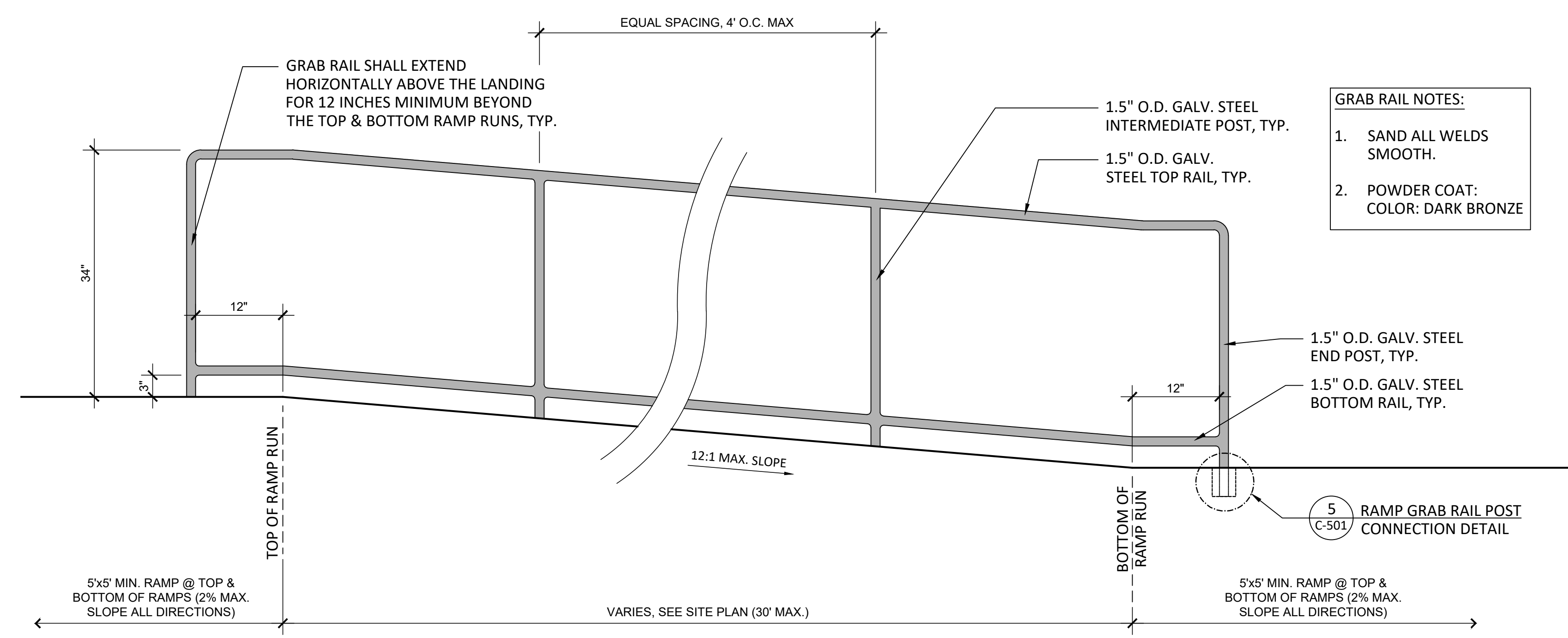
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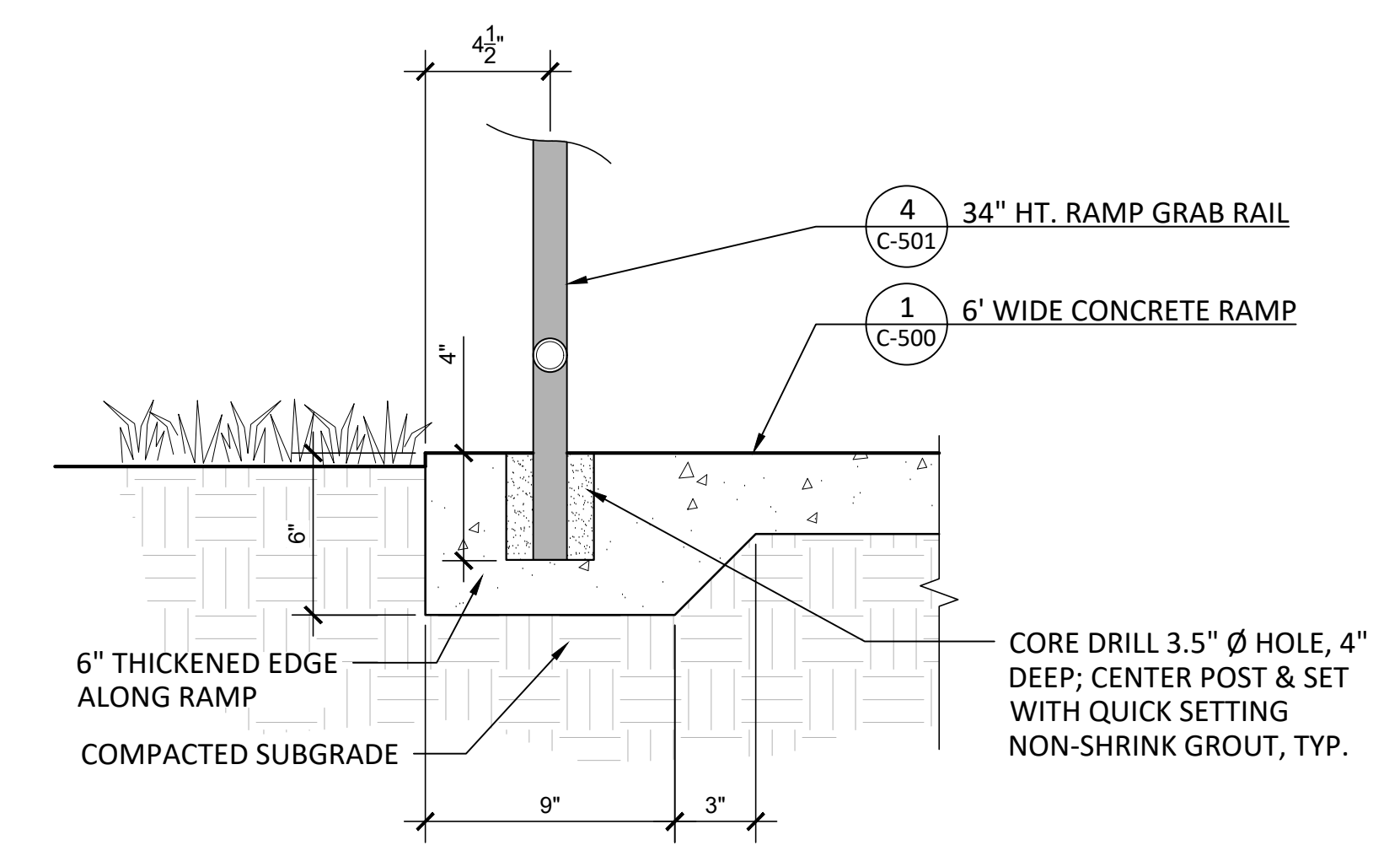
**1 ADA PARKING SPACES**  
 SCALE: VARIES

**2 CONCRETE WHEEL STOP**  
 SCALE: 1" = 1'-0"

**3 ACCESSIBLE PARKING SPACE SIGN**  
 SCALE: 1" = 1'-0"



**GRAB RAIL NOTES:**  
 1. SAND ALL WELDS SMOOTH.  
 2. POWDER COAT: COLOR: DARK BRONZE



**4 34" HT. METAL GRAB RAIL**  
 SCALE: 1" = 1'-0"

**5 RAMP GRAB RAIL POST CONNECTION DETAIL**  
 SCALE: 2" = 1'-0"



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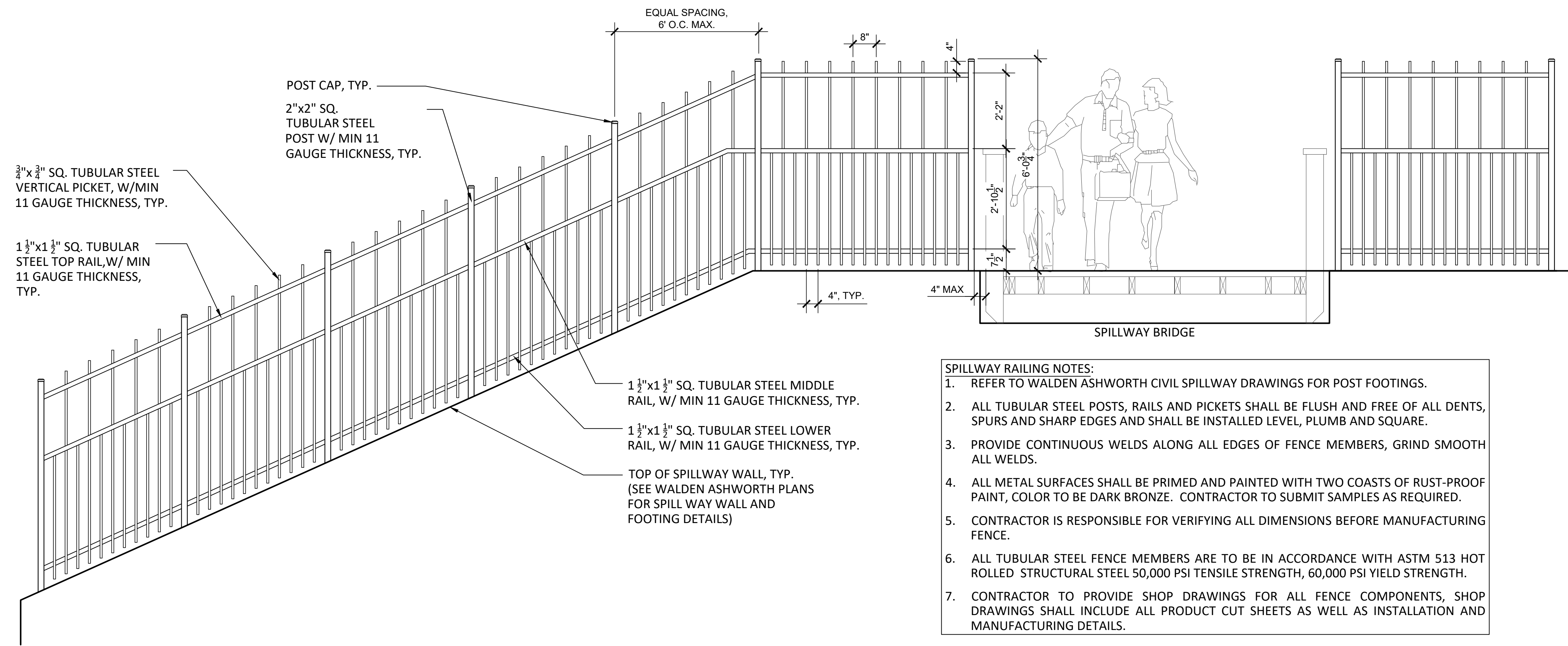
Date: 07/16/2024

Project No: 2022-024

Drawn By: PS

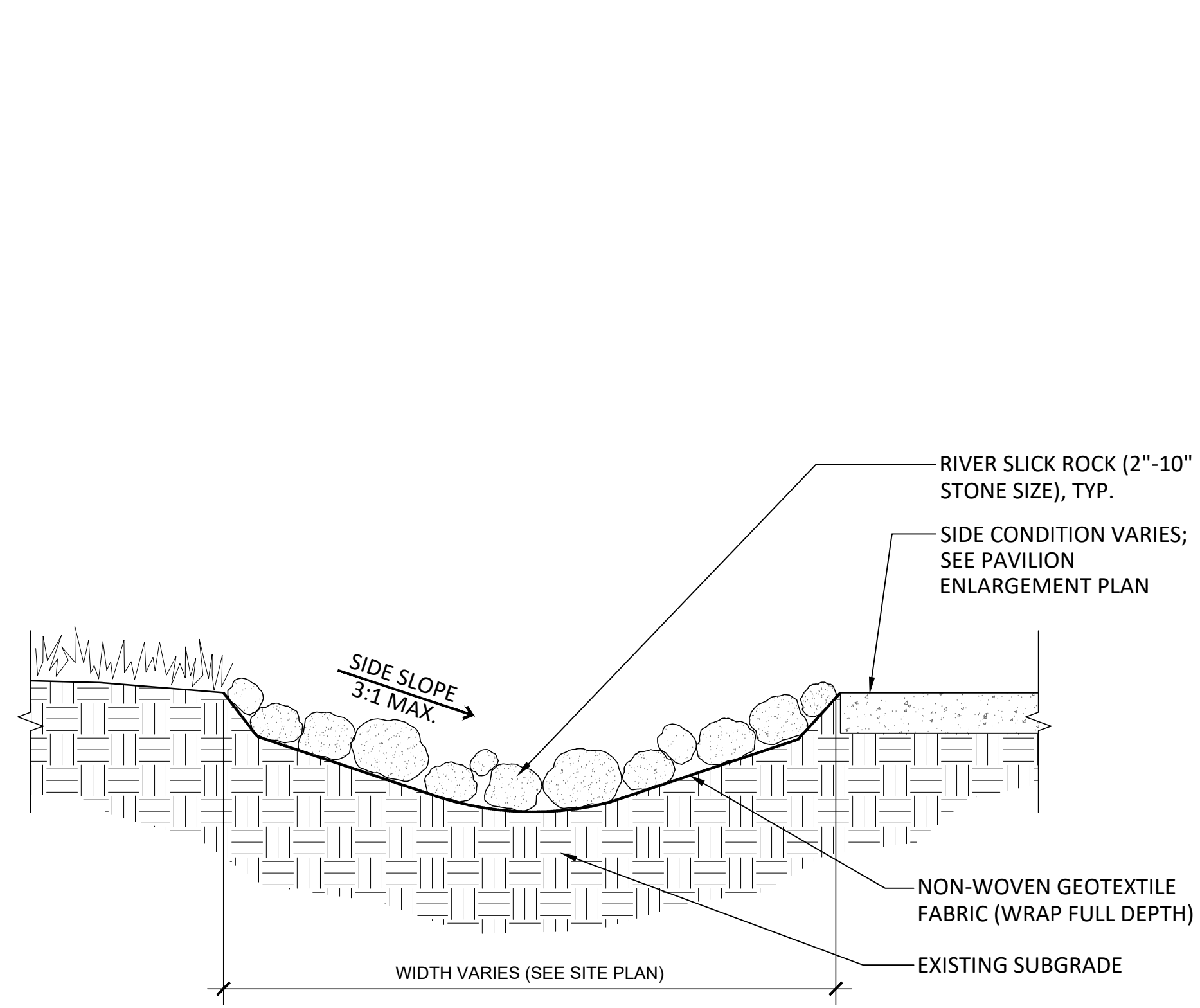
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**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
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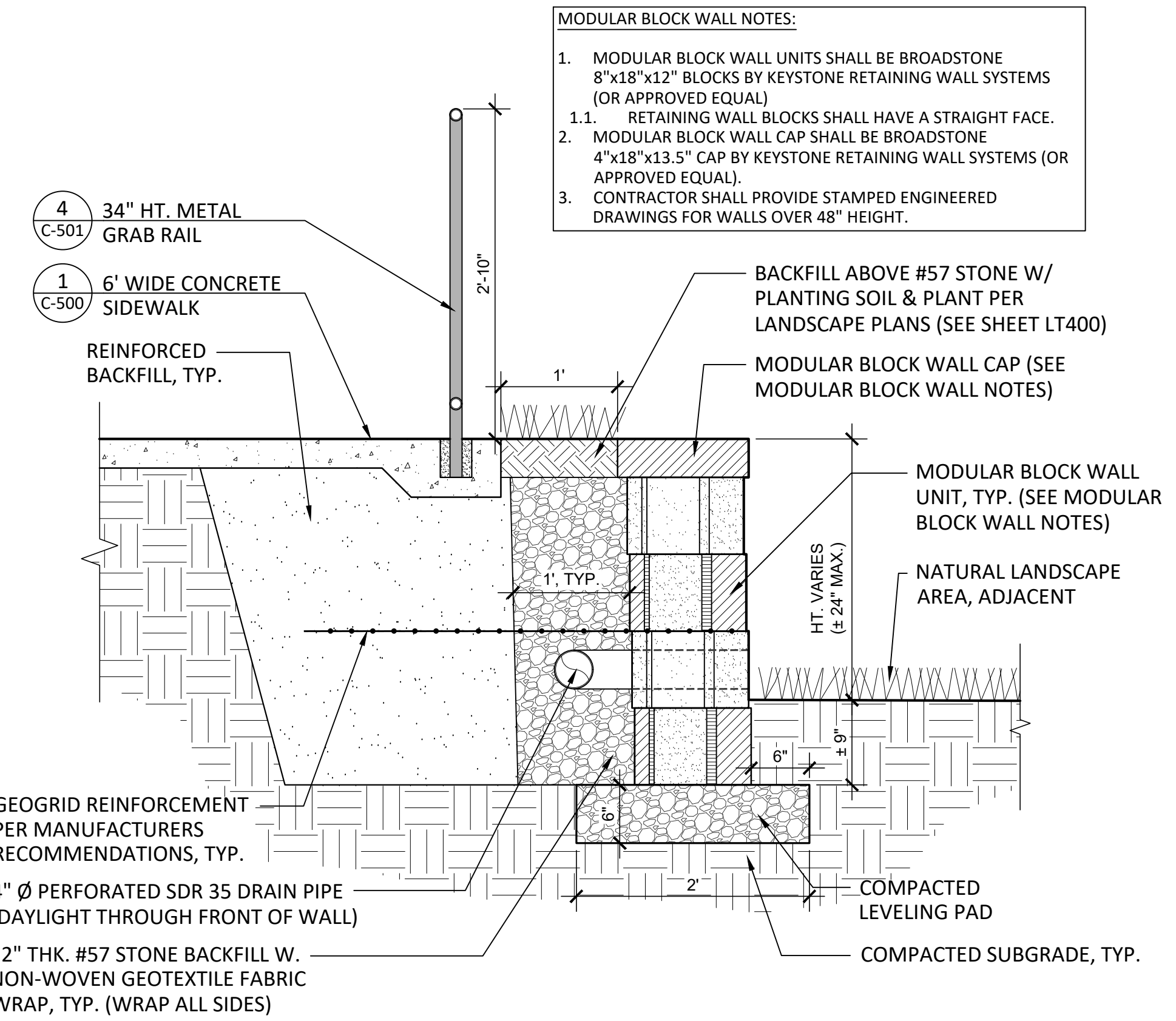


- SPILLWAY RAILING NOTES:**
- REFER TO WALDEN ASHWORTH CIVIL SPILLWAY DRAWINGS FOR POST FOOTINGS.
  - ALL TUBULAR STEEL POSTS, RAILS AND PICKETS SHALL BE FLUSH AND FREE OF ALL DENTS, SPURS AND SHARP EDGES AND SHALL BE INSTALLED LEVEL, PLUMB AND SQUARE.
  - PROVIDE CONTINUOUS WELDS ALONG ALL EDGES OF FENCE MEMBERS, GRIND SMOOTH ALL WELDS.
  - ALL METAL SURFACES SHALL BE PRIMED AND PAINTED WITH TWO COATS OF RUST-PROOF PAINT, COLOR TO BE DARK BRONZE. CONTRACTOR TO SUBMIT SAMPLES AS REQUIRED.
  - CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS BEFORE MANUFACTURING FENCE.
  - ALL TUBULAR STEEL FENCE MEMBERS ARE TO BE IN ACCORDANCE WITH ASTM 513 HOT ROLLED STRUCTURAL STEEL 50,000 PSI TENSILE STRENGTH, 60,000 PSI YIELD STRENGTH.
  - CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR ALL FENCE COMPONENTS, SHOP DRAWINGS SHALL INCLUDE ALL PRODUCT CUT SHEETS AS WELL AS INSTALLATION AND MANUFACTURING DETAILS.

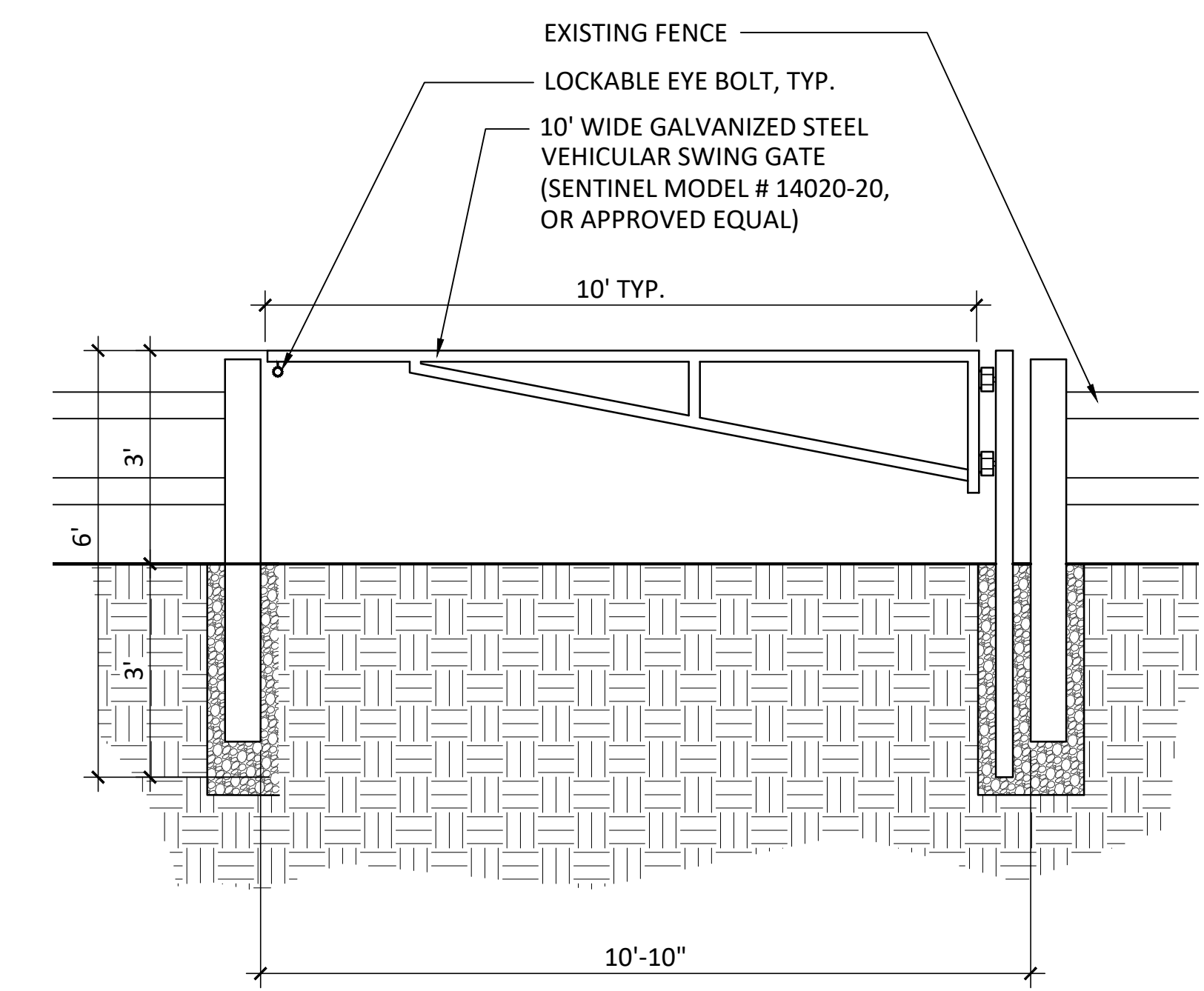
**1 SPILLWAY WALL RAILING**  
SCALE: 1/2" = 1'-0"



**2 DRY CREEK BED**  
SCALE: 1" = 1'-0"



**3 MODULAR BLOCK RETAINING WALL**  
SCALE: 1" = 1'-0"



**4 VEHICULAR SWING GATE**  
SCALE: 1/2" = 1'-0"

Revisions:

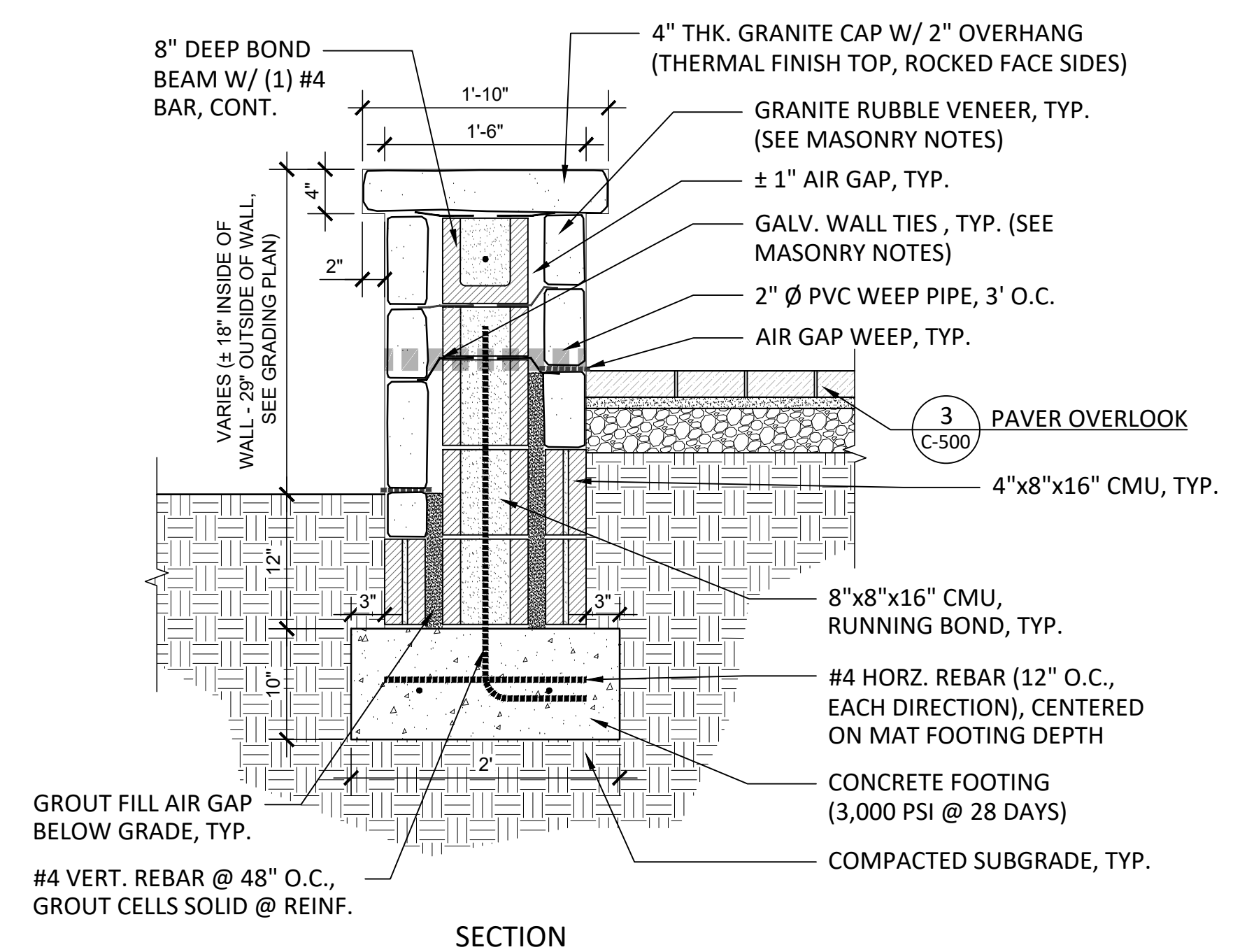
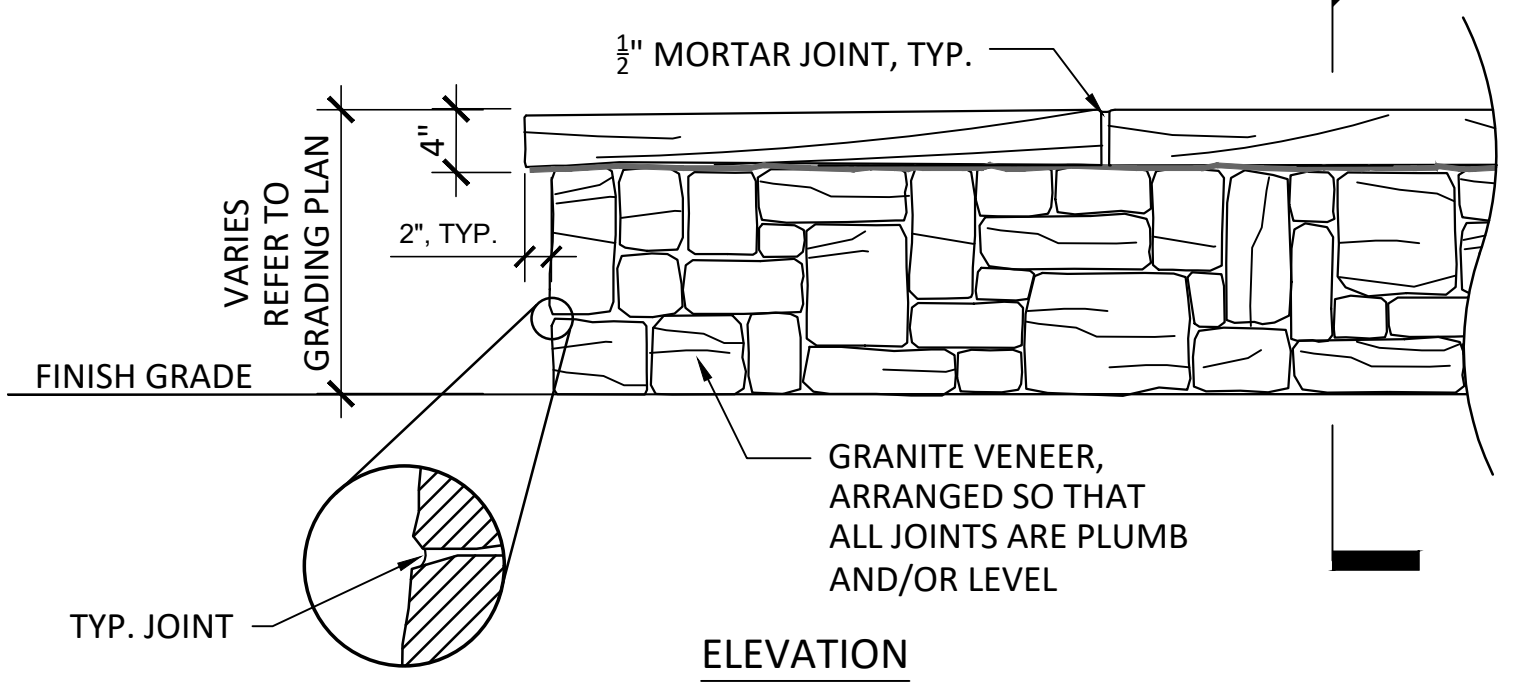
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**Construction Details**

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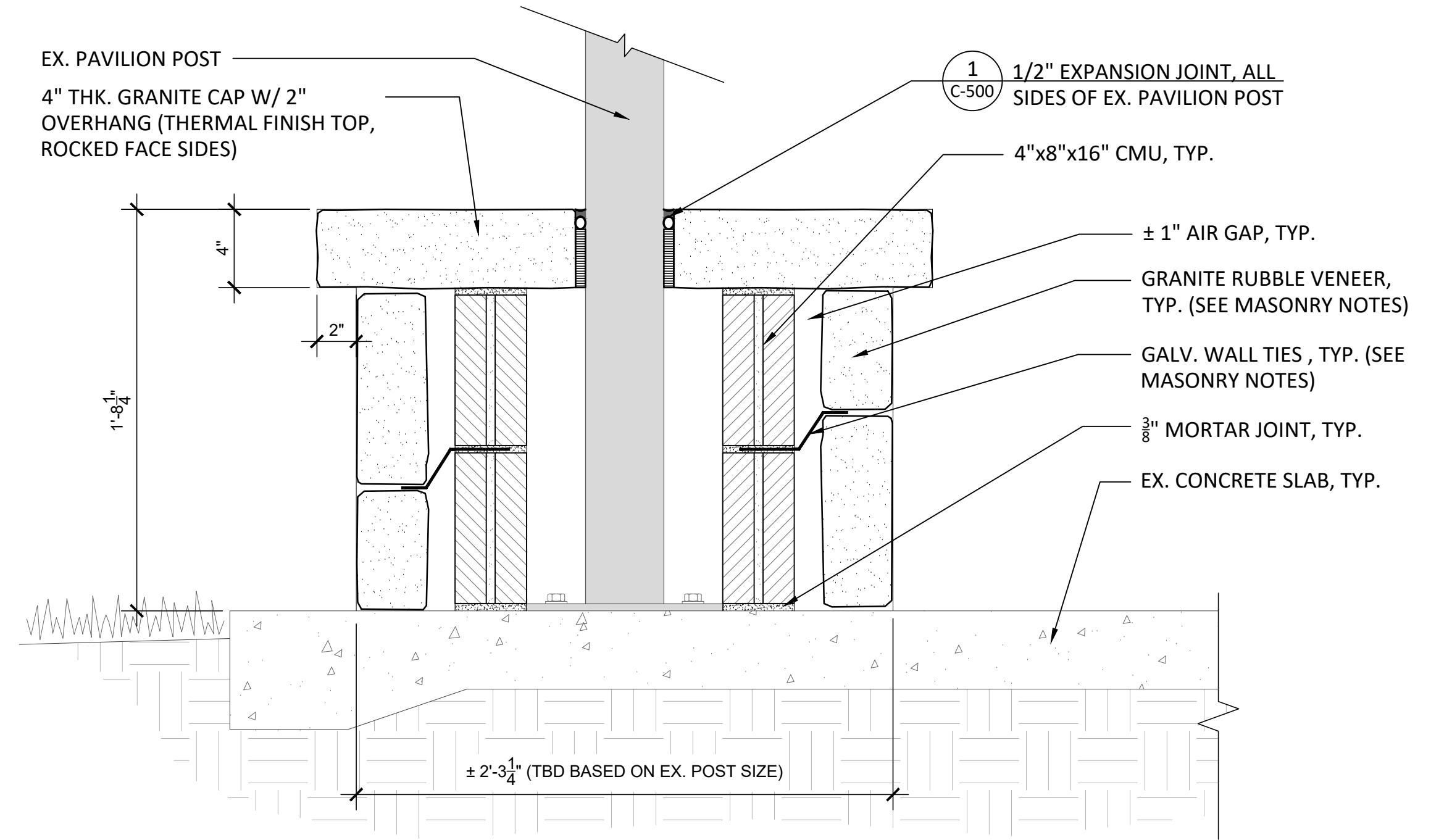


ALL STONE SHALL BE LAID IN A HORIZONTAL FASHION. JOINTS SHALL BE VERTICAL AND HORIZONTAL. VERTICAL JOINTS SHALL EXTEND NO MORE THAN 2 STONES HIGH. HORIZONTAL JOINTS SHALL EXTEND NO MORE THAN 3 STONES IN LENGTH.

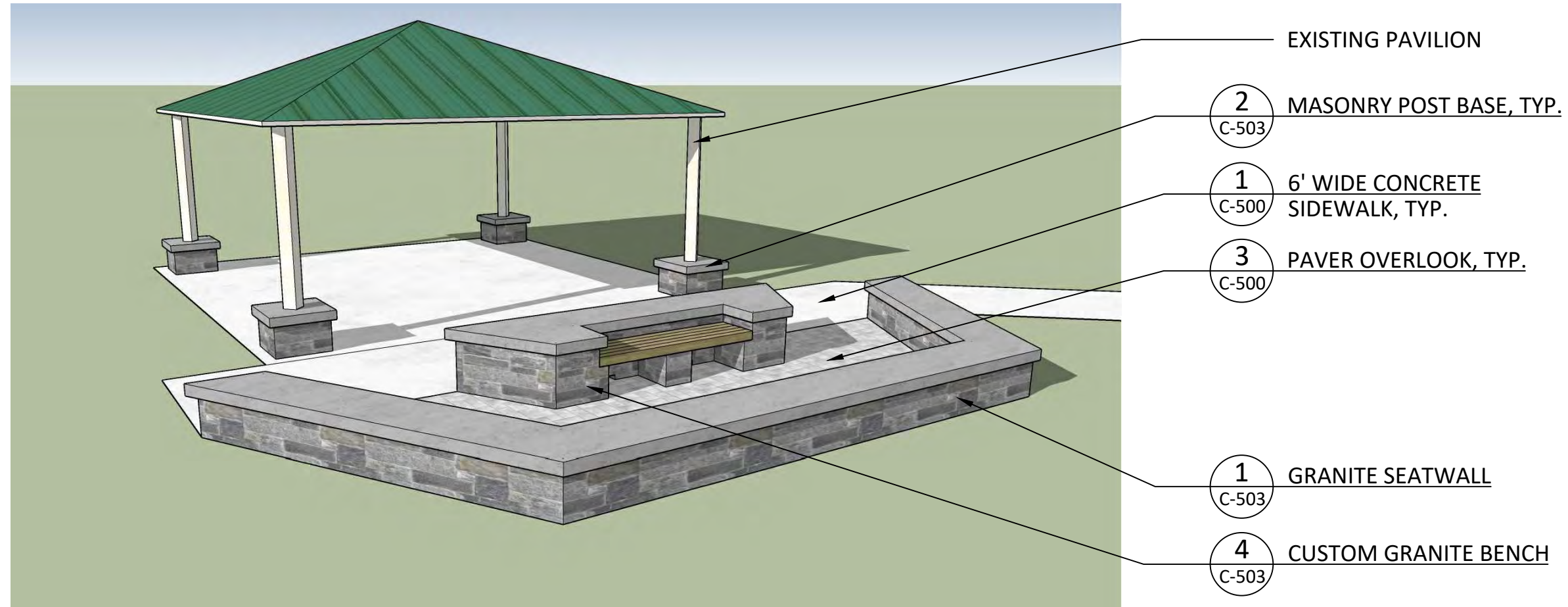


- MASONRY NOTES:**
- MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF ACI 530.1, "SPECIFICATION FOR MASONRY STRUCTURES" EXCEPT AS NOTED ON THE DRAWINGS.
  - ALL MASONRY SHALL HAVE STANDARD HORIZONTAL REINFORCING PLACED @ 16" O.C. VERTICAL SPACING UNLESS NOTED OTHERWISE. JOINT REINFORCEMENT FOR SINGLE WYTHE WALLS SHALL BE LOX ALL LADDER-MESH AS MANUFACTURED BY HOHMANN & BARNARD, INC., HAUPPAUGE, NY, OR APPROVED EQUAL. REINFORCEMENT SHALL BE FABRICATED FROM COLD-DRAWN STEEL WIRE CONFORMING TO ASTM A82 WITH SMOOTH 9GA CROSS RODS BUTT WELDED NOT MORE THAN 16" O.C. TO DEFORMED 9GA SIDE RODS. FACTORY PREFABRICATED CORNERS AND TEES SHALL BE USED AT ALL CORNERS AND INTERSECTING WALLS AND SHALL BE OF THE SAME GAUGE, FINISH AND DESIGN AS THE CONTINUOUS JOINT REINFORCEMENT.
  - MASONRY LAP SPLICE AS DETERMINED BY IBC 2018 SECTION 2107.2.3
  - CONCRETE MASONRY UNITS (CMU) SHALL BE 4"x8"x16" AND 8"x8"x16" NOMINAL DIMENSIONS, U.N.O., WITH 1,900 PSI COMPRESSIVE STRENGTH, STACKED IN RUNNING BOND.
  - MORTAR FOR CMU WALLS SHALL BE TYPE "S".
  - GRANITE VENEER SHALL BE: 4" THICK, ELBERTON GRAY RUBBLE (OR SIMILAR), ROUGH FINISH, RANDOM SIZES, ROUGHLY SQUARE TO RECTANGULAR.
  - VENEER SHALL BE SECURED W/GALVANIZED WALL TIES. MAX. HORIZONTAL SPACING = 32"; MAX. VERTICAL SPACING = 16"; MIN. 5/8" GROUT COVER TO OUTSIDE FACE.
  - MORTAR FOR GRANITE VENEER SHALL BE TYPE "N", GRAY. TOOL ALL JOINTS CONCAVE. 1" MAX. JOINTS BETWEEN RUBBLE. JOINTS SHALL BE VERTICAL AND HORIZONTAL.
  - PROVIDE 2" MIN. COVER OVER ALL STEEL REINFORCEMENT, U.N.O.
  - MINIMUM VERTICAL REINFORCING TO BE #5 @ 48" O.C. UNLESS NOTED OTHERWISE. PROVIDE ONE #5 BARS VERTICAL (FULL WALL HEIGHT) AT EACH CORNER AND TWO #5 BARS VERTICAL (FROM FOOTING TO BEARING) IN FIRST CELL WITHIN 16" OF OPENINGS, WITHIN 8" OF EACH SIDE OF A CONTROL JOINT/EXPANSION JOINT. AT ALL VERTICAL MASONRY REBARS, LAP SAME SIZE HOOKED DOWELS INTO FOUNDATION AND FILL CELLS CONTAINING REBARS SOLID WITH 3000 PSI GROUT.
  - AT TOP COURSE OF ALL MASONRY WALLS, PROVIDE A MINIMUM OF 8" BOND BEAM WITH (1) #4 CONTINUOUS REBARS. FILL BOND BEAM SOLID WITH 3000 PSI GROUT.
  - PROVIDE VERTICAL CONTROL JOINT @ 25'-0" O.C. MAXIMUM, AS NEEDED.

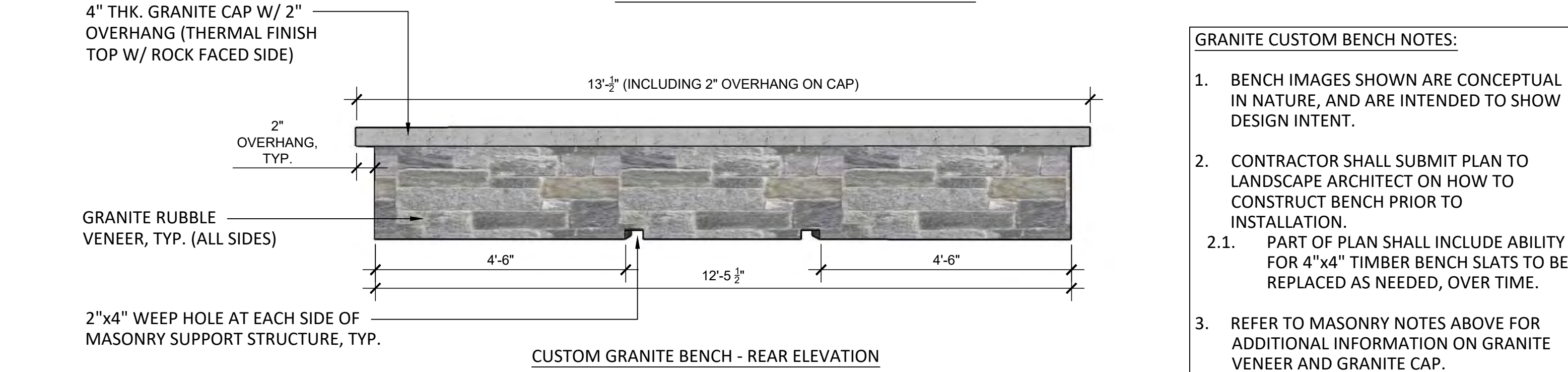
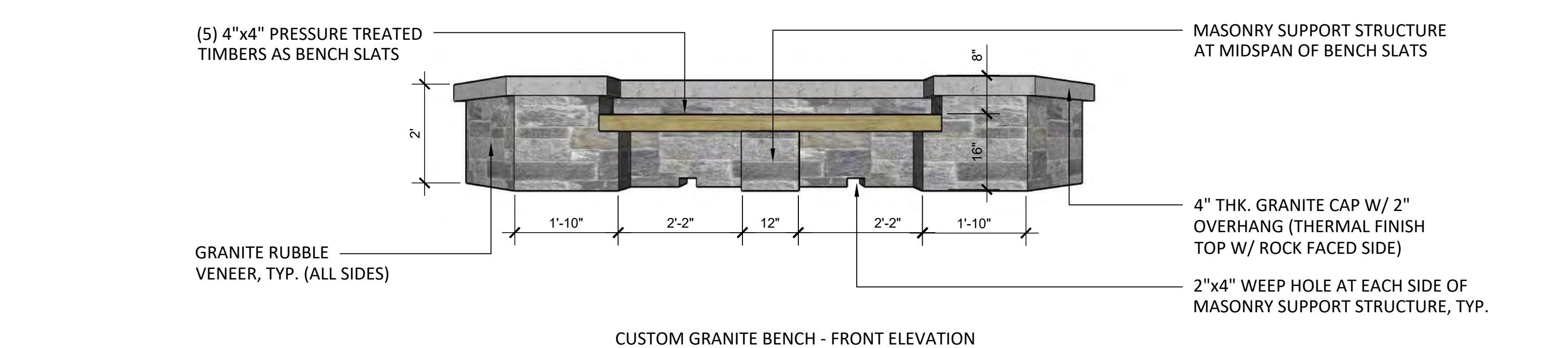
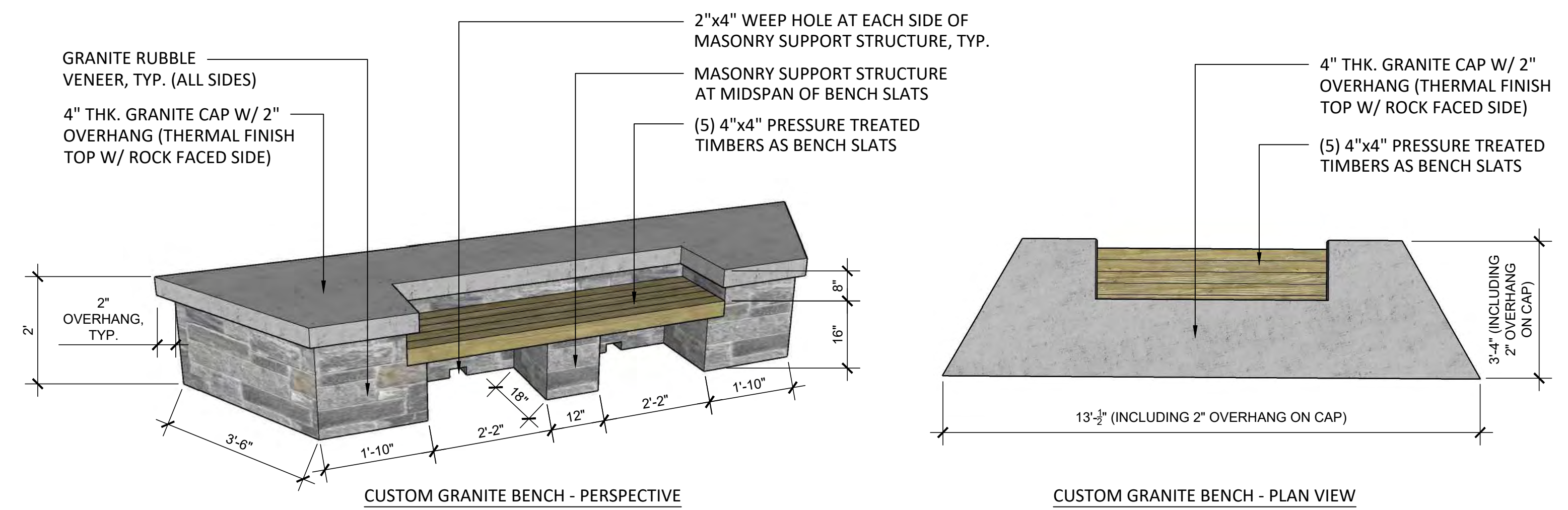
**1 GRANITE SEAT WALL DETAIL**  
SCALE: 1" = 1'-0"



**2 MASONRY POST BASE DETAIL**  
SCALE: VARIES

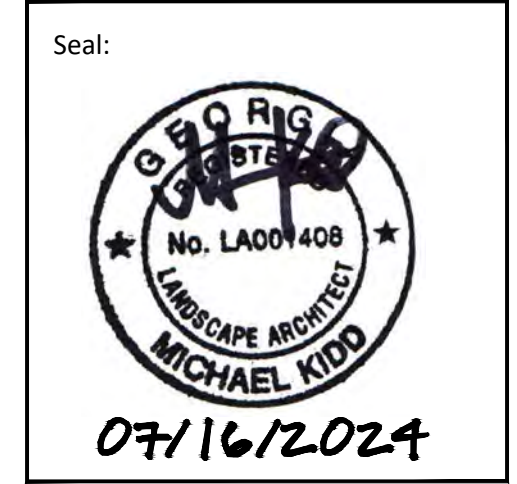


**3 CUSTOM GRANITE BENCH EXAMPLE MODEL**  
SCALE: N.T.S.



**4 CUSTOM GRANITE BENCH**  
SCALE: N.T.S.

- GRANITE CUSTOM BENCH NOTES:**
- BENCH IMAGES SHOWN ARE CONCEPTUAL IN NATURE, AND ARE INTENDED TO SHOW DESIGN INTENT.
  - CONTRACTOR SHALL SUBMIT PLAN TO LANDSCAPE ARCHITECT ON HOW TO CONSTRUCT BENCH PRIOR TO INSTALLATION.
    - PART OF PLAN SHALL INCLUDE ABILITY FOR 4"x4" TIMBER BENCH SLATS TO BE REPLACED AS NEEDED, OVER TIME.
  - REFER TO MASONRY NOTES ABOVE FOR ADDITIONAL INFORMATION ON GRANITE VENEER AND GRANITE CAP.



Date: 07/16/2024  
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Checked By: MK

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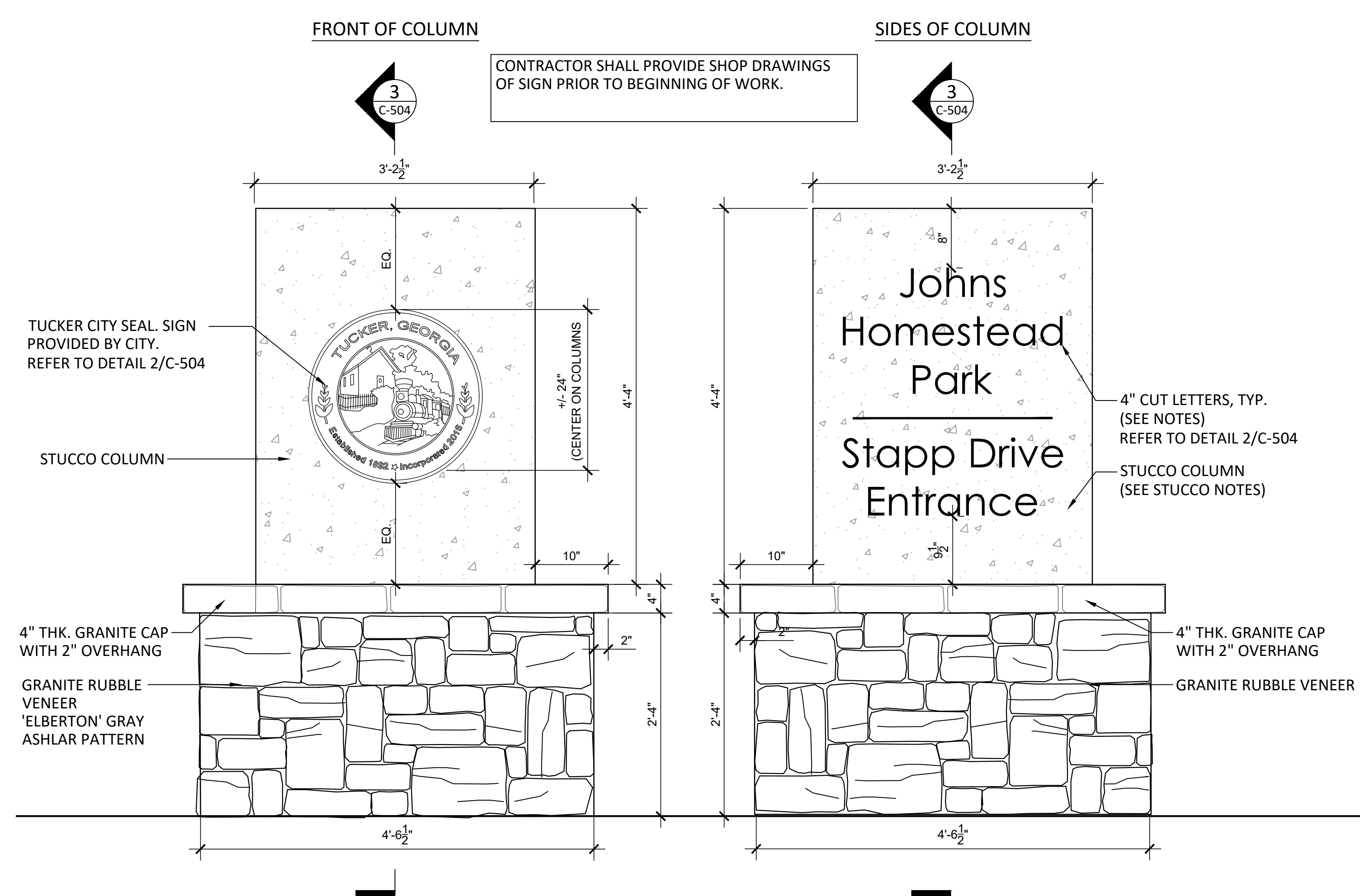
Revisions:

NO.	DATE	DESCRIPTION

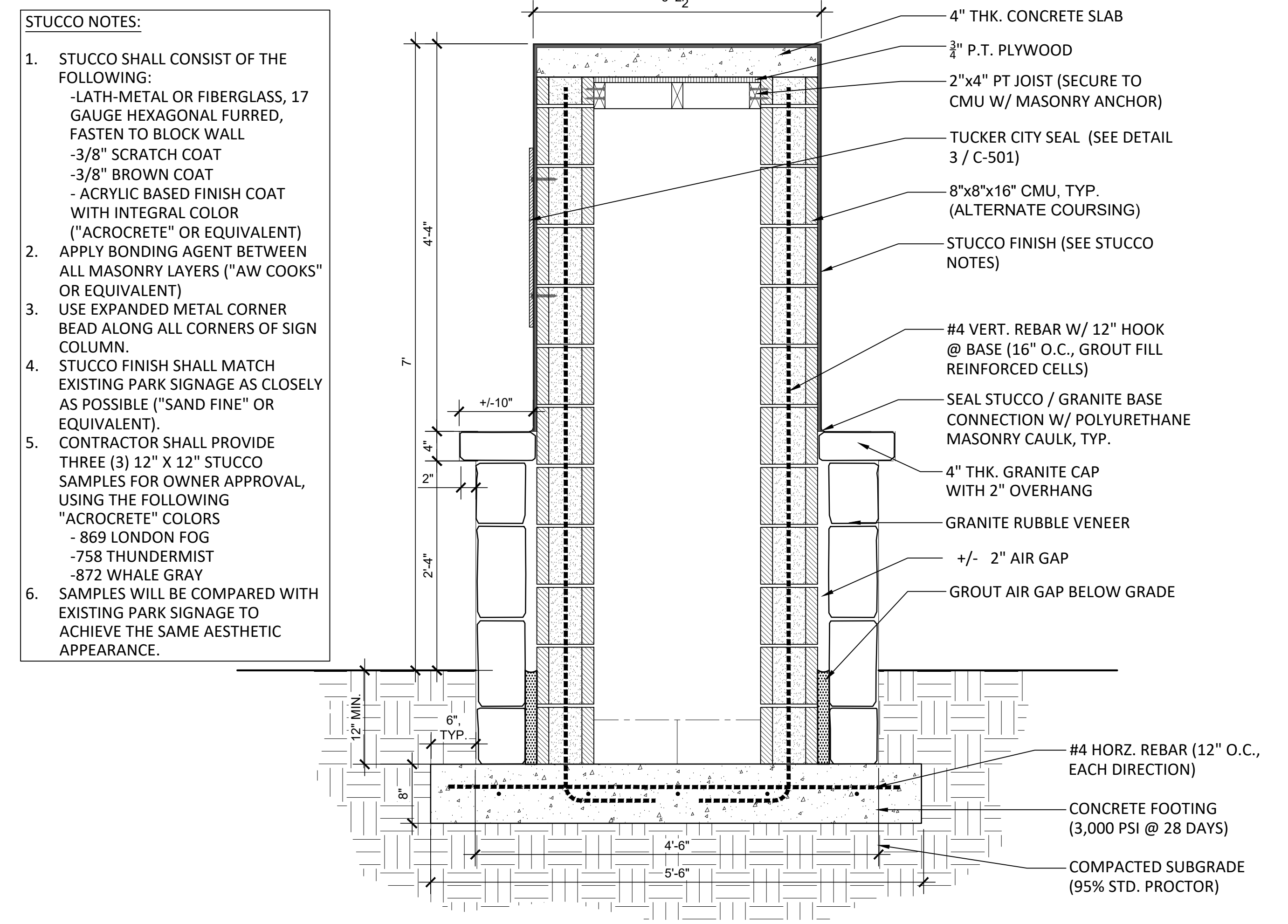
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**Construction Details**

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**C-503**

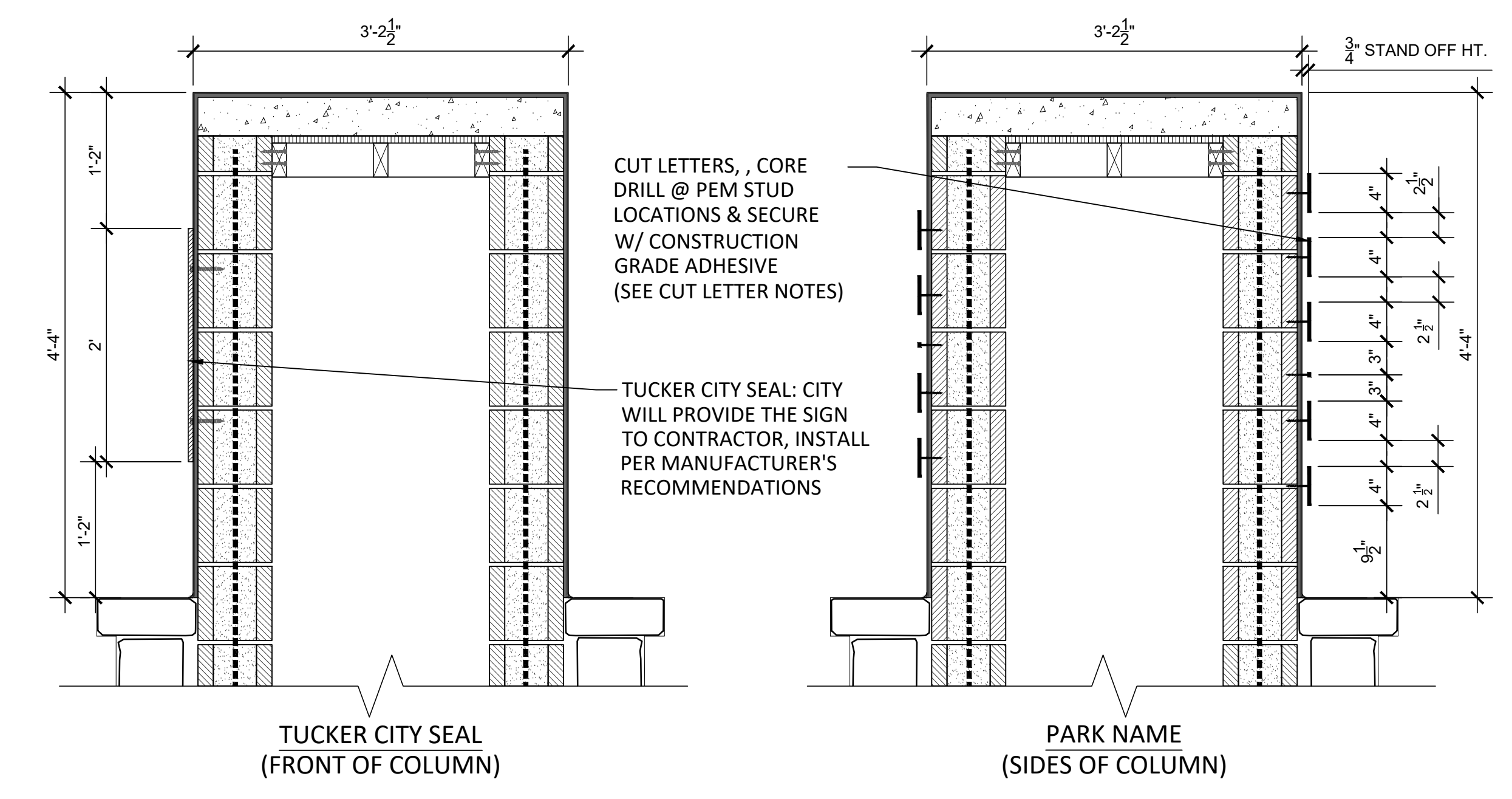




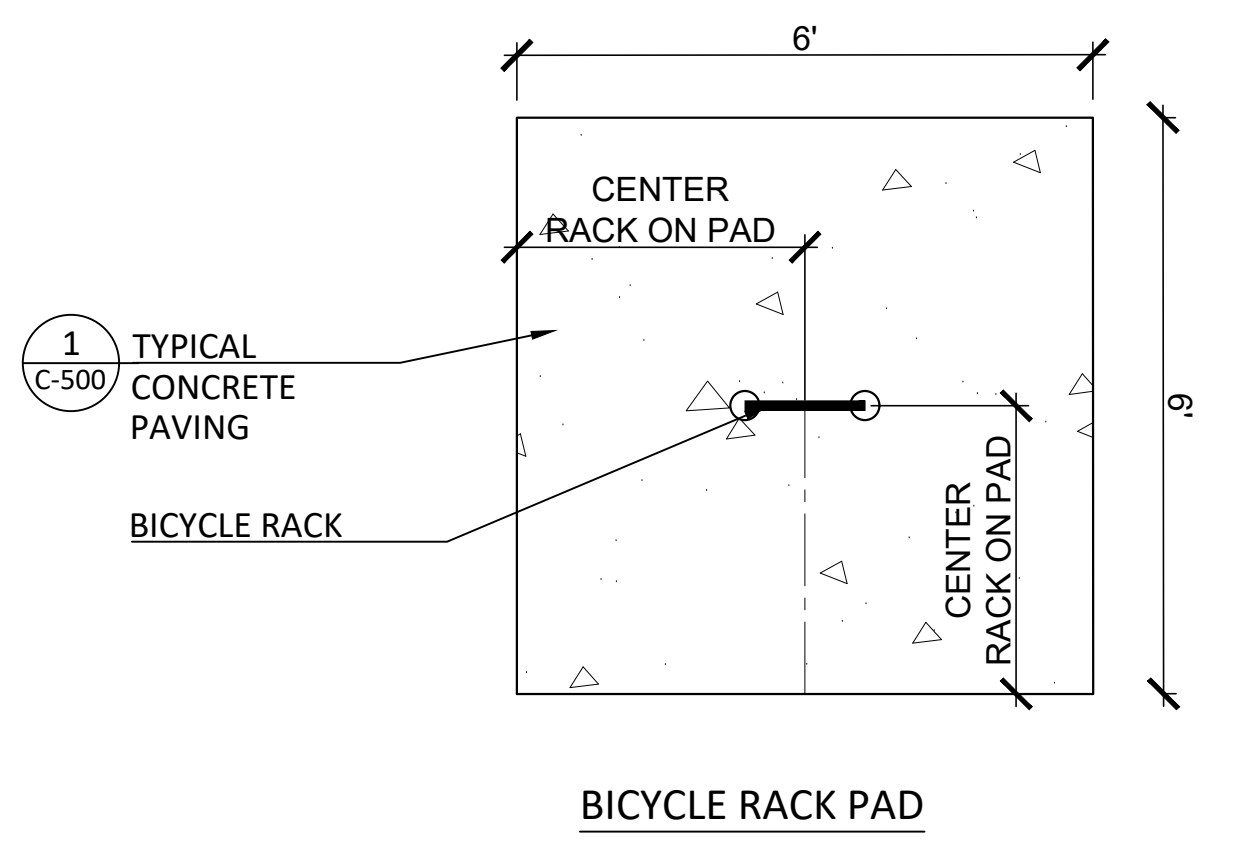
**1 STAPP DRIVE COLUMN MONUMENT SIGN ELEVATION**  
SCALE: 1" = 1'-0"



**3 COLUMN MONUMENT SIGN SECTION**  
SCALE: 1" = 1'-0"



**2 COLUMN MONUMENT SIGN - PARK NAME / CITY SEAL MOUNTING DETAIL**  
SCALE: 1" = 1'-0"



**4 BICYCLE RACK AND PAD**  
SCALE: 1" = 2'-0"

- BIKE RACK NOTES:**
- BIKE RACK TO BE MODEL # 12707 (IN GROUND), OR # 12700 (SURFACE) BY CYCLESAFE (cyclesafe.com , 616-954-9977) OR APPROVED EQUAL.
  - BIKE RACK TO BE BLACK IN COLOR W/ PLASTISOL OR POWDER COAT FINISH (TBD).
  - PROVIDE 2 EA.
  - BIKE RACK TO BE SURFACE MOUNTED TO CONCRETE PAVING, PER MANUFACTURER RECOMMENDATIONS.
  - CONTRACTOR SHALL SUBMIT PRODUCT INFORMATION FOR APPROVAL PRIOR TO INSTALLATION.

Revisions:

NO.	DATE	DESCRIPTION

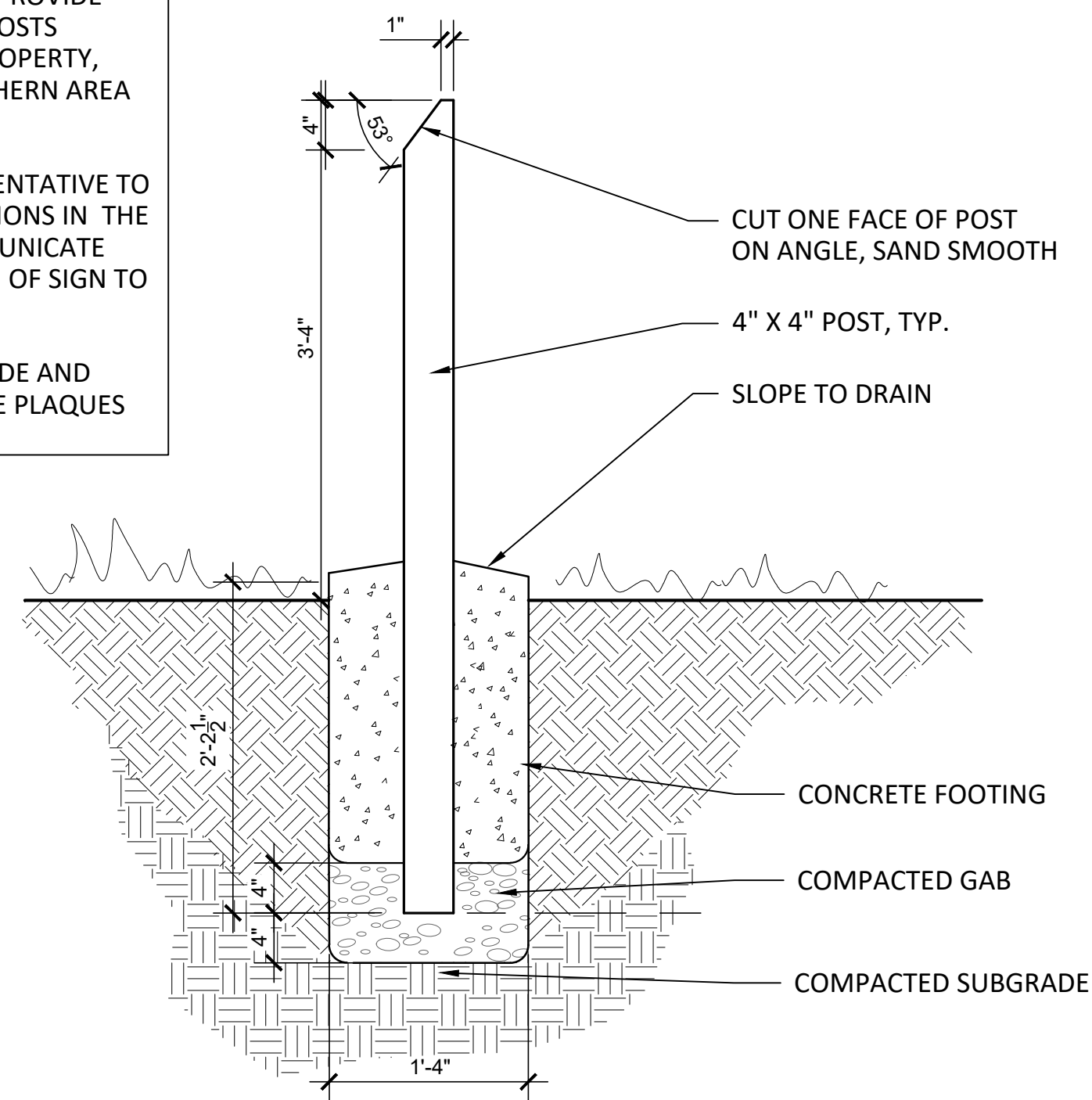
Sheet Title:  
**Construction Details**

Sheet No:  
**C-504**



**ORIENTEERING POST NOTES**

1. CONTRACTOR TO PROVIDE AND INSTALL 15 POSTS AROUND PARK PROPERTY, INCLUDING NORTHERN AREA OF PARK.
2. OWNER'S REPRESENTATIVE TO FLAG POST LOCATIONS IN THE FIELD AND COMMUNICATE INTENDED FACING OF SIGN TO CONTRACTOR
3. OWNER TO PROVIDE AND INSTALL Q.R. CODE PLAQUES ON POSTS.



**1 ORIENTEERING POST DETAIL**  
SCALE: 1" = 1'-0"



REFERENCE IMAGE

**KIOSK NOTES:**

- Kiosk 1 Stapp Drive :**
1. Manufacturer: Terrabilt or Approved Equivalent 914-341-1500 Info@terrabilt.com
  2. Model: 2 Post Mini Kiosk with 48" x 36" Shear Profile Bulletin Case (Bulletin Case to be 2 sided), Recycled HDPE Frame & Shingles
  3. Mounting: Surface Mount
  4. Colors: Standard Colors

- Kiosk 2 Edinburgh:**
1. Manufacturer: Terrabilt or Approved Equivalent 914-341-1500 Info@terrabilt.com
  2. Model: 2 Post Trailhead Kiosk with 32" x 36" Shear Profile Bulletin Case: (Bulletin Case to be 2 sided) Recycled HDPE Frame & Shingles
  3. Mounting: Surface Mount
  4. Colors: Standard Colors

**Footing:**

1. 2 Post Mini Kiosk 18" wide x 36" deep - 2 each Mount with xx
2. 2 Post Trail Kiosk 18" wide x 36" deep

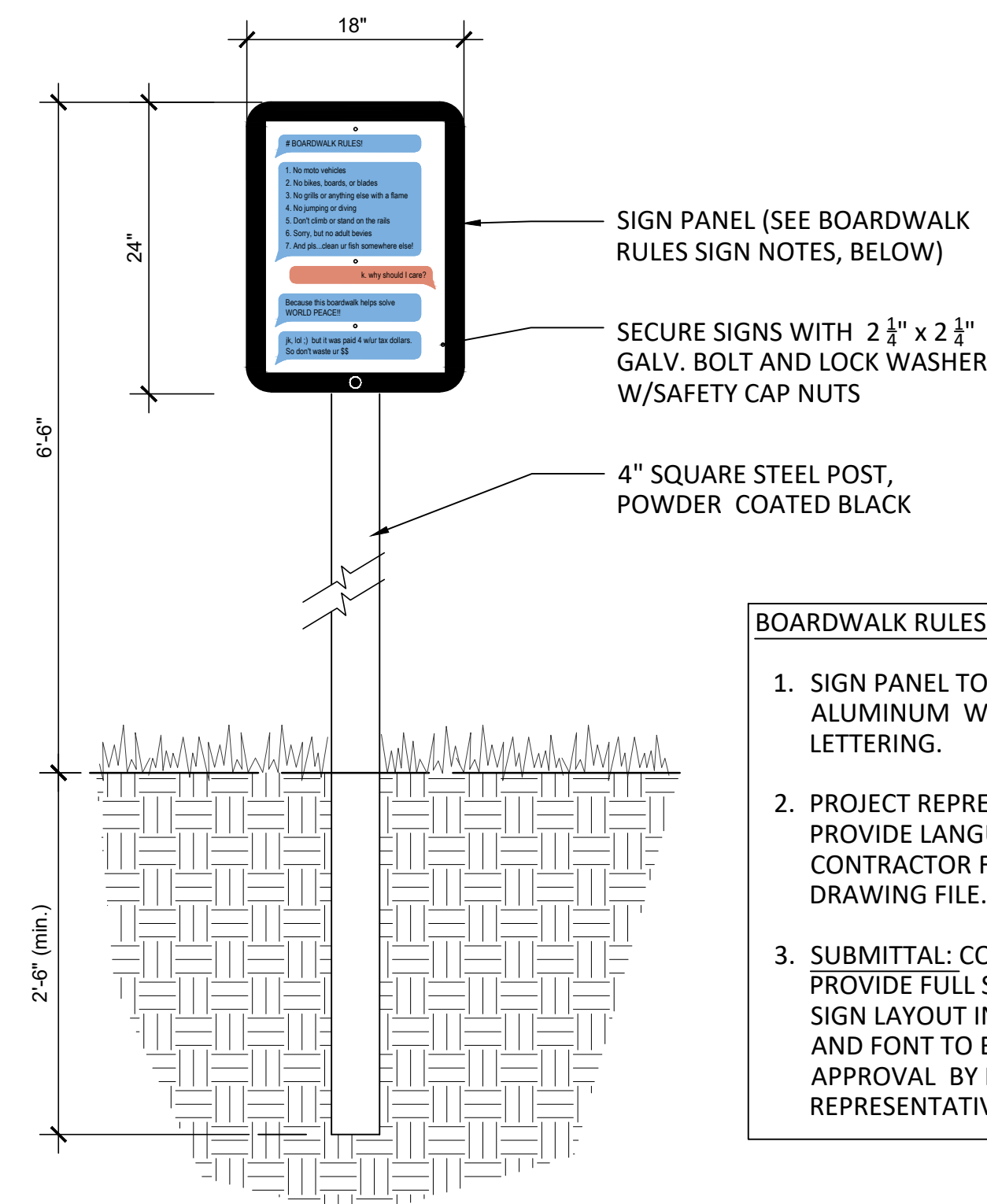
**3 PARK KIOSK DETAIL**  
SCALE: NOT TO SCALE

**TYPICAL INTERPRETIVE SIGN EXAMPLE**  
(INFORMATION ON SIZE VARIES)



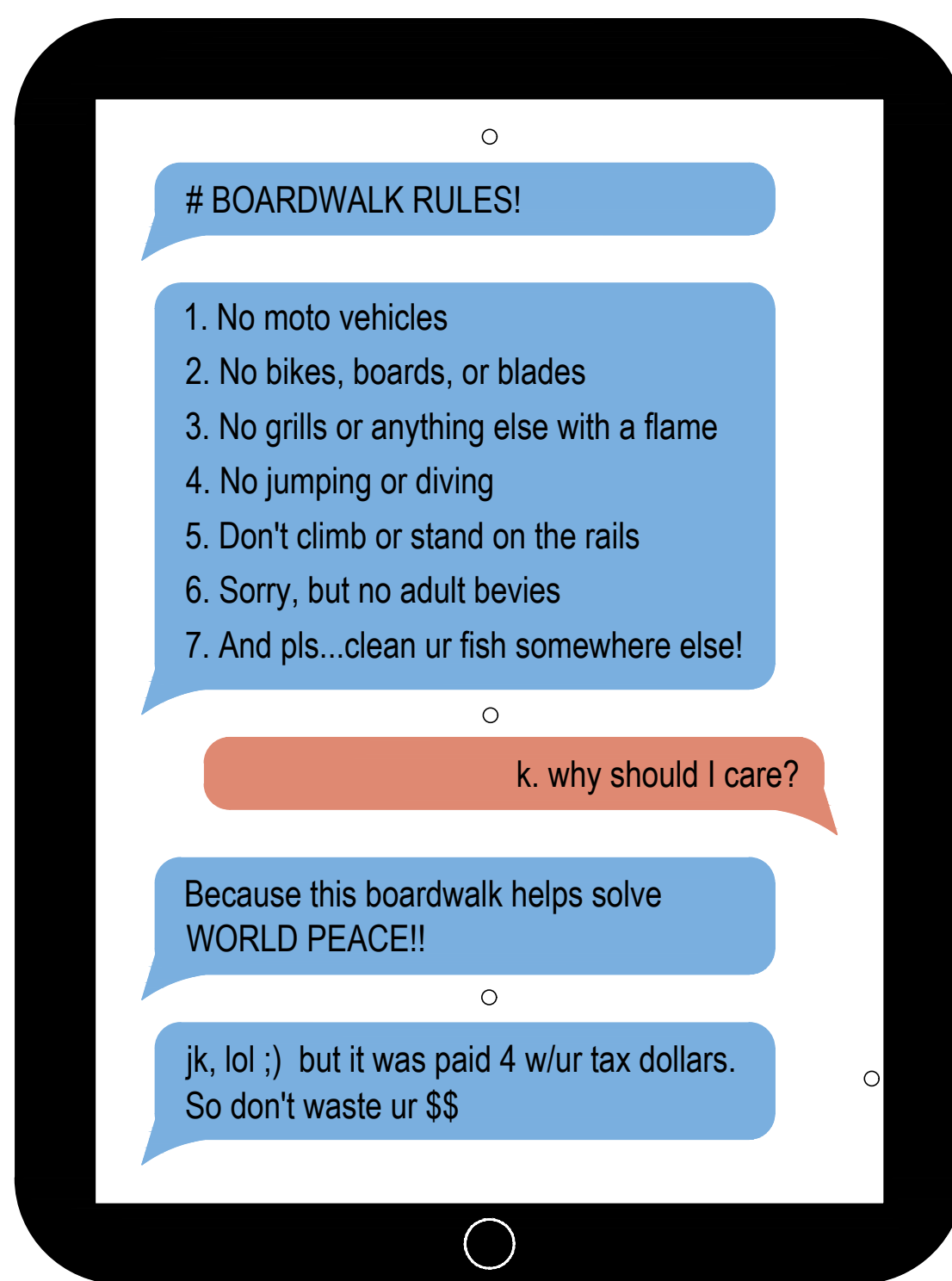
**INTERPRETIVE SIGN NOTES**

1. MANUFACTURER: VÄCKER SIGNS OR APPROVED EQUAL VACKERSIGN.COM TEL: (877) 487-3103 948 SHERREN ST W ROSEVILLE, MN 55113
2. SIGN MOUNT: FRAMELESS PEDESTAL WITH 12X18 MOUNT PLATE AND 60" ALUMINUM POST FOR IN GROUND INSTALLATION. COLOR: BLACK
3. SIGN MATERIALS & SIZE: 3 EA GENERAL THEMED SIGNS. 18" X 24" IN SIZE HIGH PRESSURE LAMINATE, 1/2" THICK SIGN PANEL 2 EA SEMI-CUSTOM THEMED SIGN. 18" X 24" IN SIZE HIGH PRESSURE LAMINATE, 1/2" THICK SIGN PANEL
4. SIGN CONTENT AND QUANTITIES: 1 EA- (NW-27SS) INVASIVE PLANTS 1 EA- (NW-23SS) NORTH AMERICAN BEAVER 1 EA- (NW-20SS) GREAT BLUE HERON 1 EA- (GSC21) NATIVE PLANTS -SEMI CUSTOM: PROJECT DESIGNER TO PROVIDE 3 SUBSTITUTE PHOTOS 1 EA- (NW-35SS) SHORELINE HABITAT - SEMI CUSTOM: PROJECT DESIGNER TO PROVIDE 1 SUBSTITUTE PHOTO.
5. SUBMITTALS CONTRACTOR TO SUBMIT FULL SIZE COLOR PRINT OUT OF PROPOSED SIGNS INCLUDING ALL GRAPHICS AND TEXT FOR REVIEW BY PROJECT DESIGNER PRIOR TO SIGN FABRICATION.



**BOARDWALK RULES SIGN NOTES:**

1. SIGN PANEL TO BE BROWN ALUMINUM WITH WHITE LETTERING.
2. PROJECT REPRESENTATIVE TO PROVIDE LANGUAGE TO CONTRACTOR FOR SIGN IN DRAWING FILE.
3. SUBMITTAL: CONTRACTOR TO PROVIDE FULL SCALE PDF OF SIGN LAYOUT INCLUDING TEXT AND FONT TO BE USED FOR APPROVAL BY PROJECT REPRESENTATIVE.

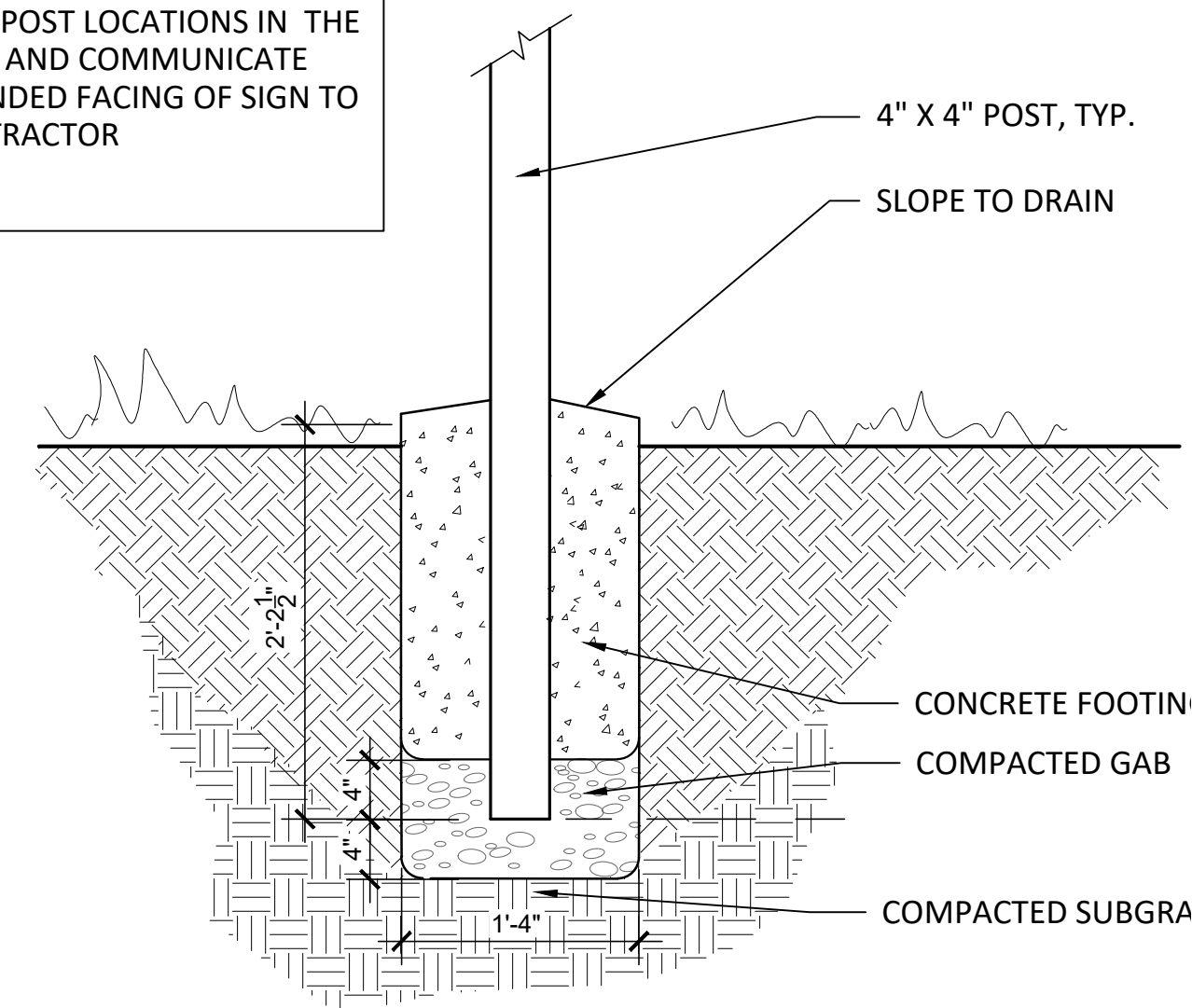


SIGN ENLARGEMENT  
N.T.S.

**2 BOARDWALK RULES SIGN DETAIL**  
SCALE: 1" = 1'-0"

**POST MOUNTING DETAIL NOTES**

1. OWNER'S REPRESENTATIVE TO FLAG POST LOCATIONS IN THE FIELD AND COMMUNICATE INTENDED FACING OF SIGN TO CONTRACTOR



**4 INTERPRETIVE SIGN DETAIL**  
SCALE: NOT TO SCALE



Date: 07/16/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MK

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

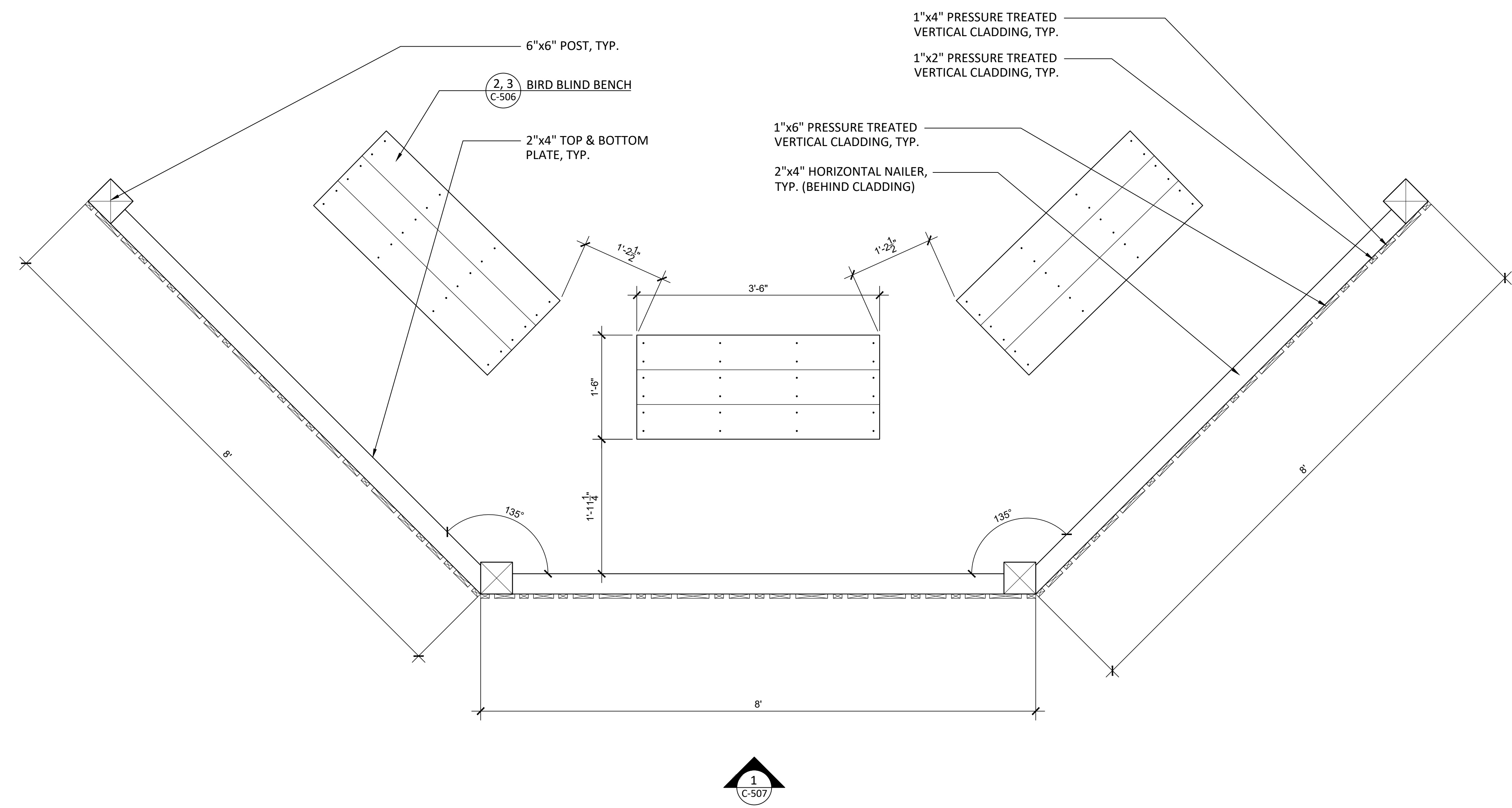
Revisions:

NO.	DATE	DESCRIPTION

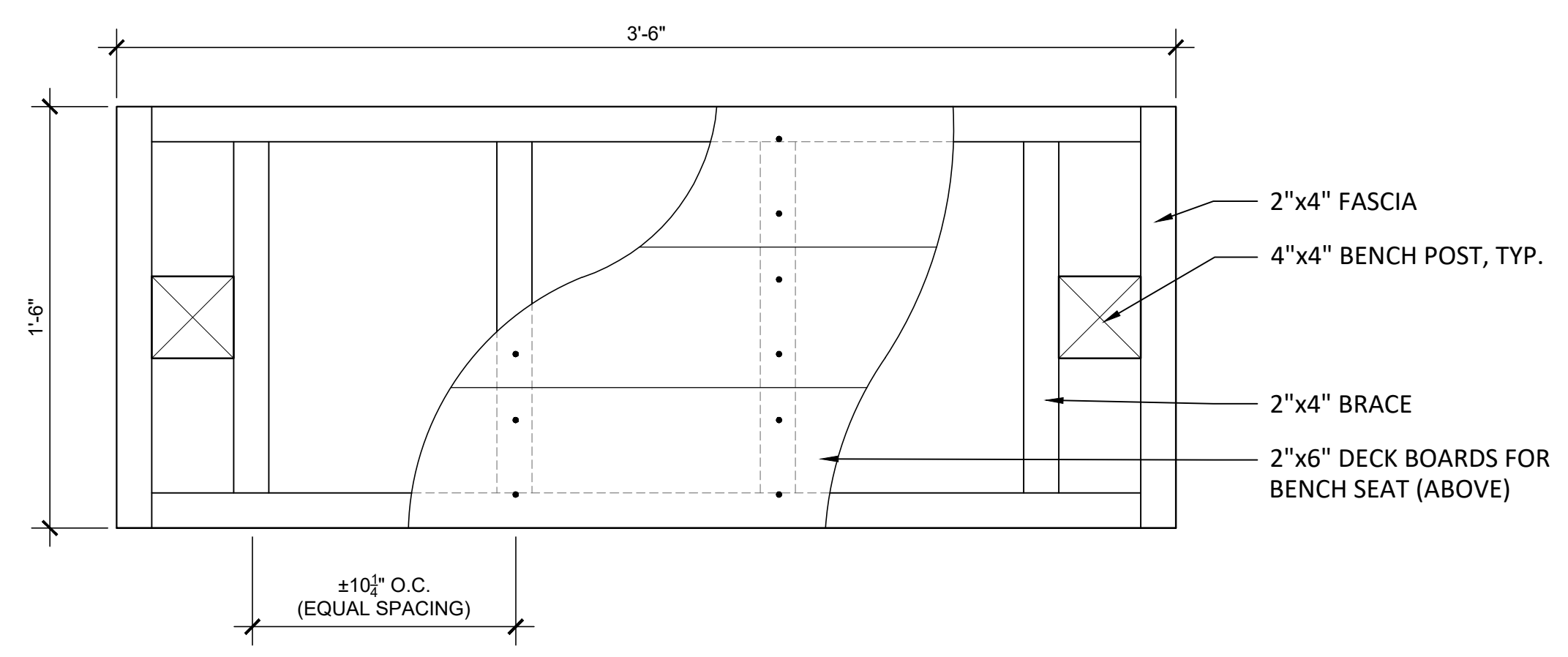
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**Construction Details**

Sheet No:  
**C-505**

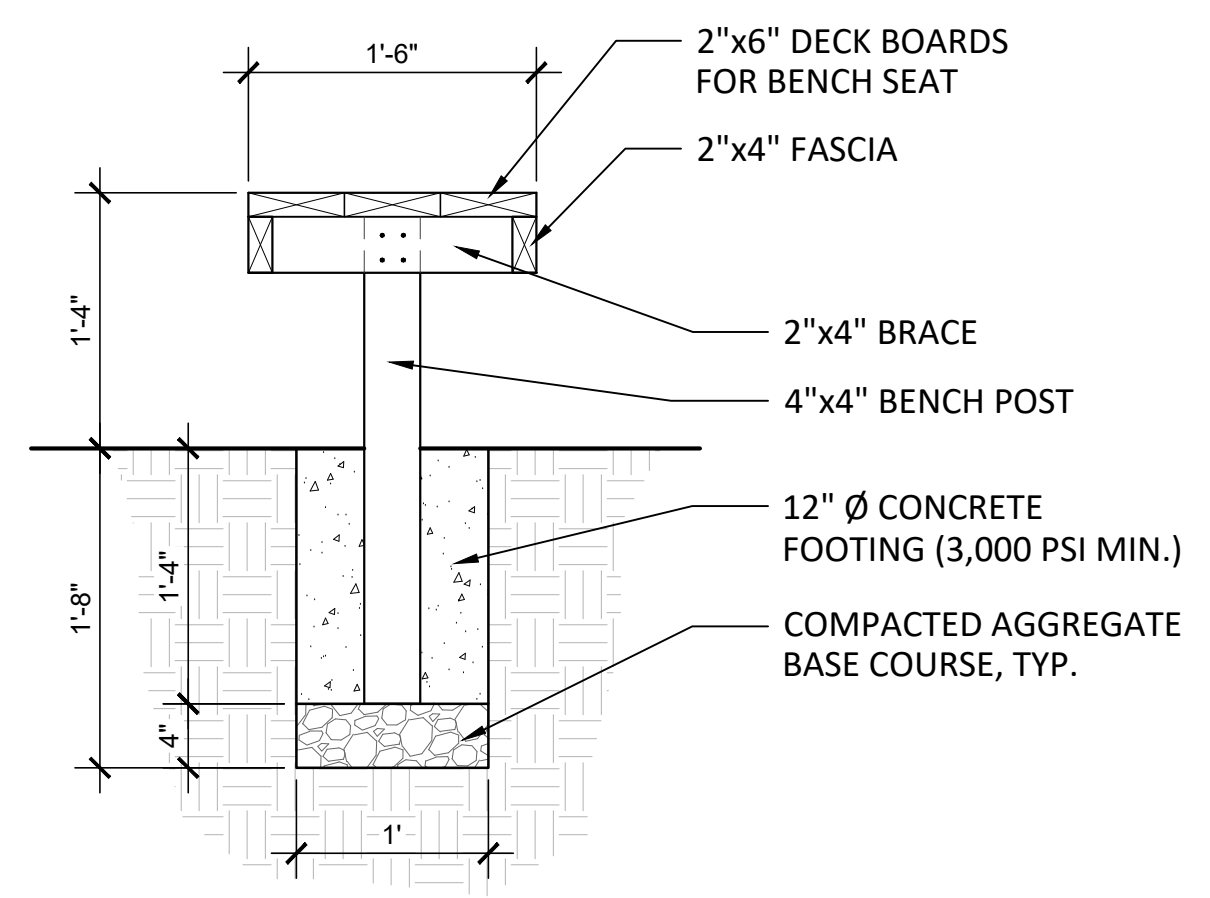




**1 BIRD BLIND PLATFORM FRAMING PLAN**  
SCALE: 1" = 1'-0"



**2 BIRD BLIND BENCH FRAMING PLAN**  
SCALE: 2" = 1'-0"



**3 BIRD BLIND BENCH SECTION**  
SCALE: 1" = 1'-0"

- BIRD BLIND BENCH NOTES:**
1. FASTEN ALL LUMBER W/ #10 HDG WOOD SCREWS, UNLESS OTHERWISE NOTED.
  2. ALL LUMBER SHALL BE #2 SYP, PRESSURE TREATED.
  3. SAND ALL EDGES SMOOTH.

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Construction Details**

Sheet No:  
**C-506**





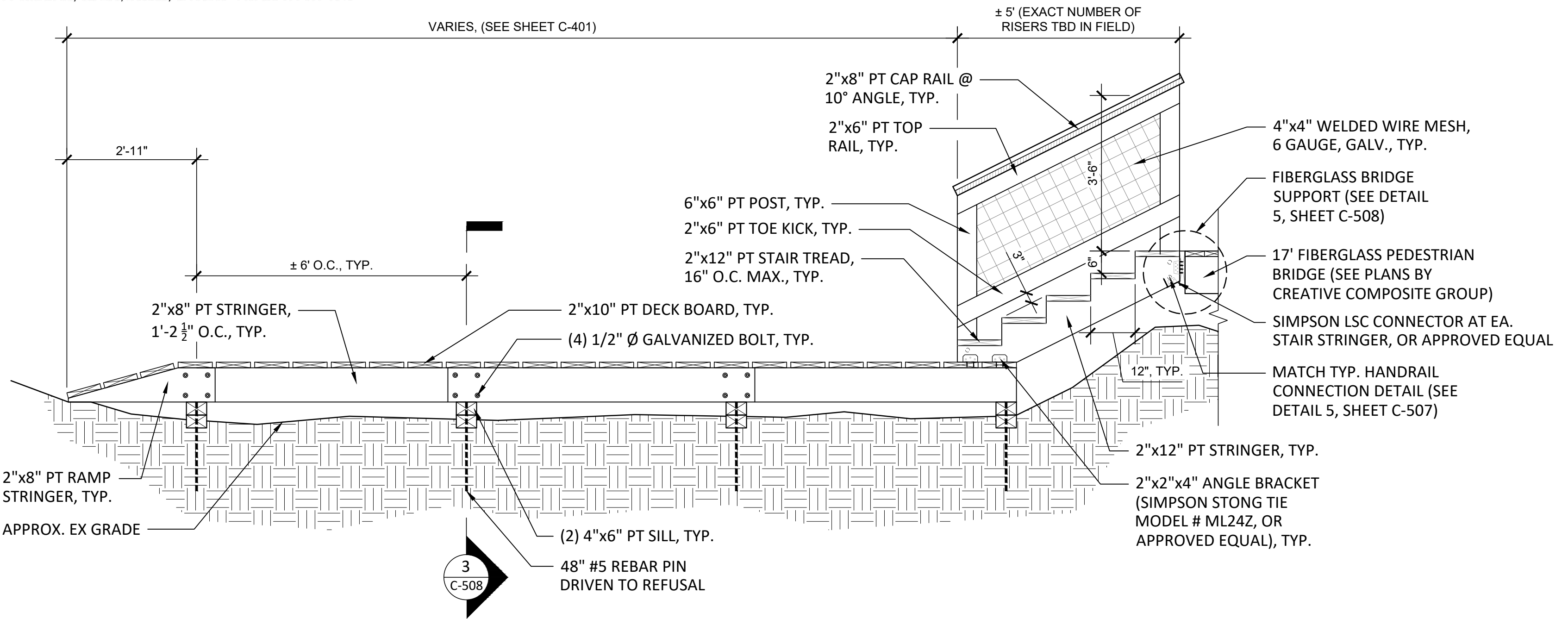


Revisions:

NO.	DATE	DESCRIPTION

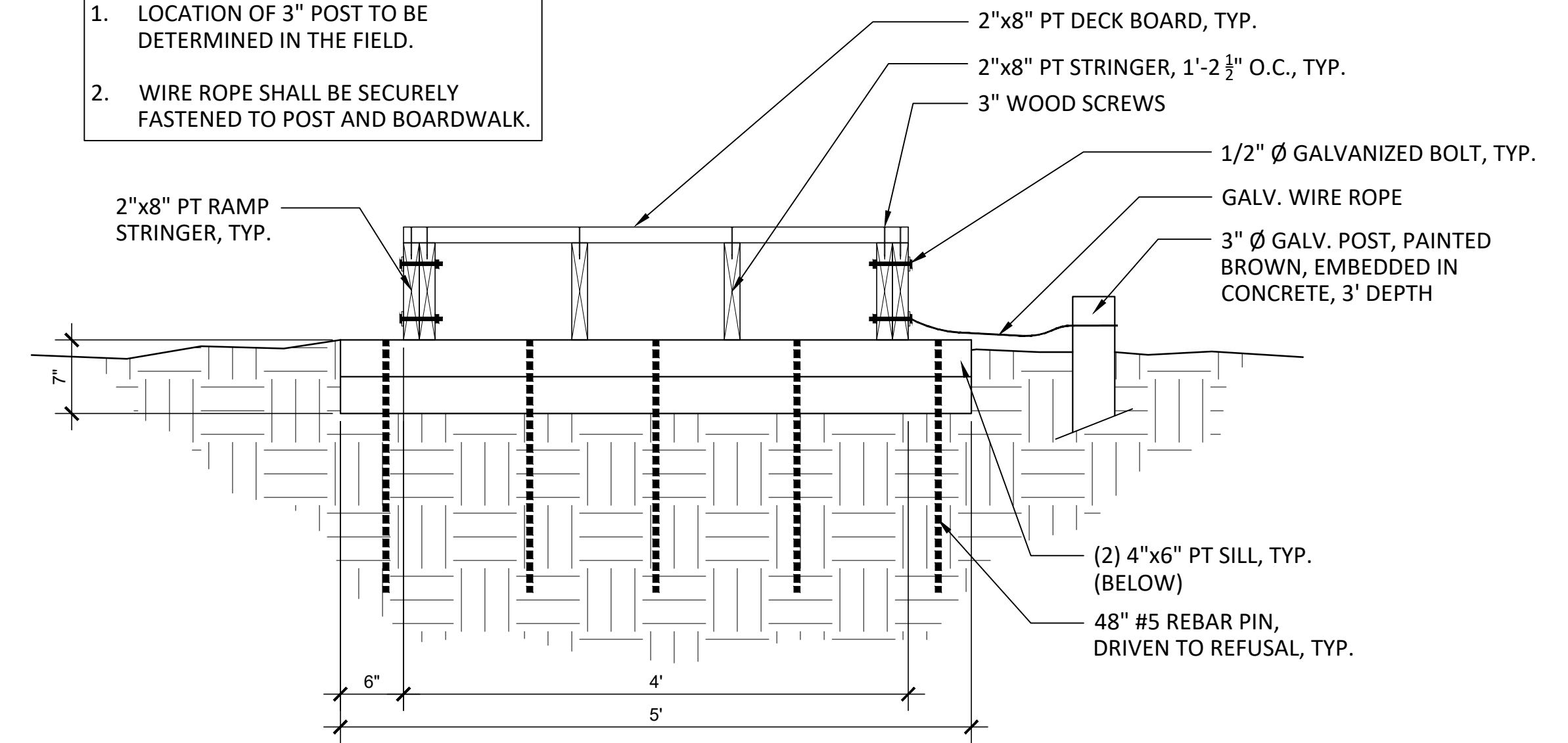
Sheet Title:  
**Construction Details**

Sheet No:  
**C-508**



**1 BREAKAWAY BOARDWALK ELEVATION & WOODEN STAIRS DETAIL**  
SCALE: 1/2" = 1'-0"

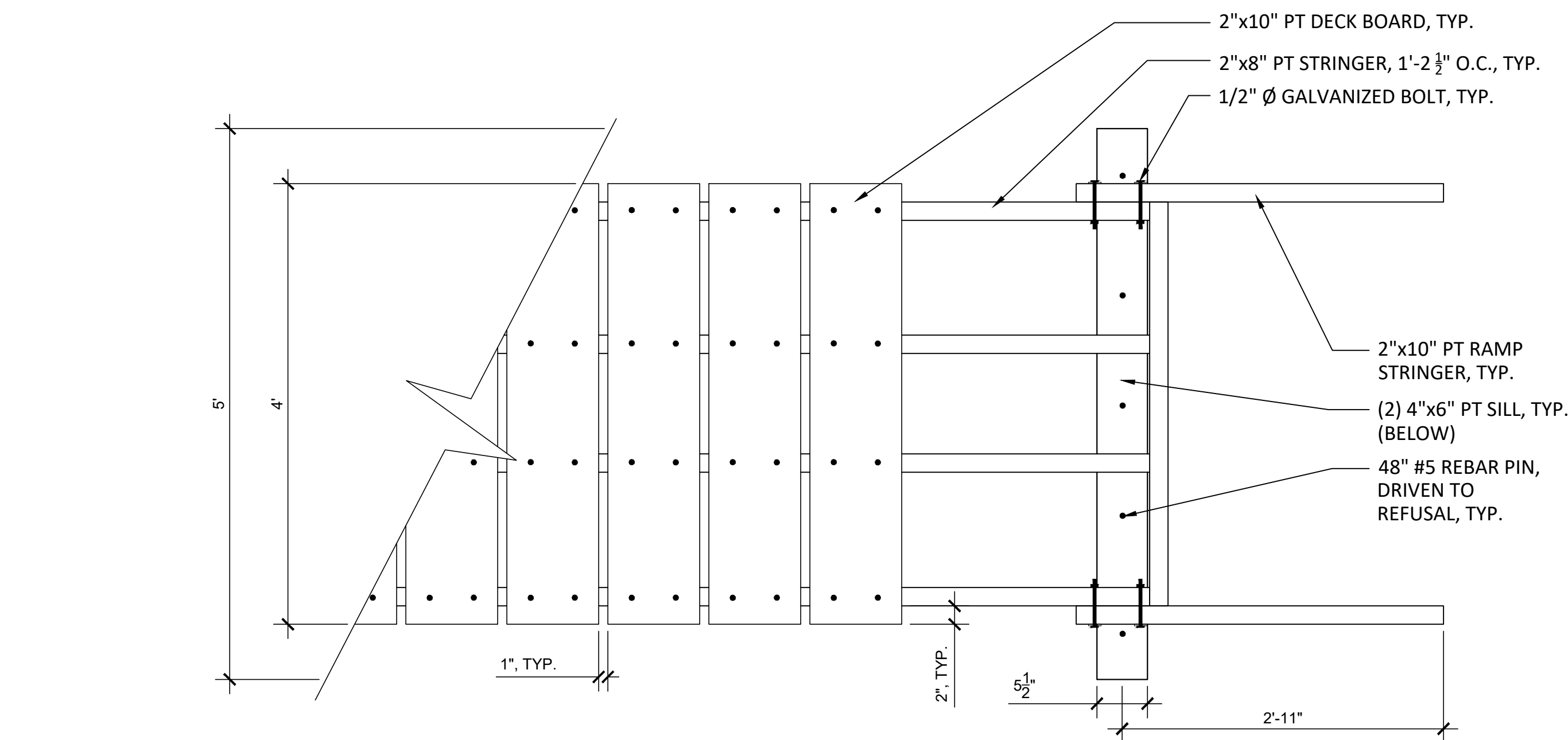
- NOTES:
1. LOCATION OF 3" POST TO BE DETERMINED IN THE FIELD.
  2. WIRE ROPE SHALL BE SECURELY FASTENED TO POST AND BOARDWALK.



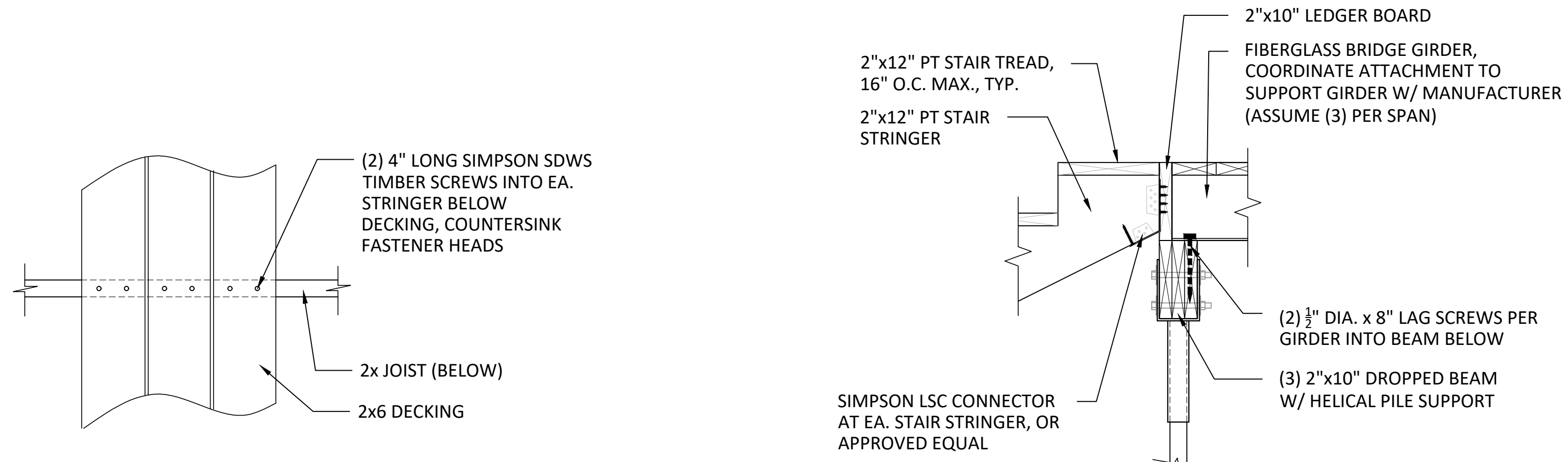
**3 BREAKAWAY BOARDWALK SECTION**  
SCALE: 1/2" = 1'-0"

**BREAKAWAY BOARDWALK & WOODEN STAIRS NOTES:**

1. EXACT NUMBER OF STAIR RISERS & TREADS TO BE DETERMINED IN FIELD BASED ON FINAL PEDESTRIAN BRIDGE ELEVATION.
2. CONTRACTOR SHALL VISIT THE SITE AND MAKE THEM SELF FAMILIAR WITH SITE CONDITIONS.
3. CONTRACTOR SHALL STAKE OUT BREAKAWAY BOARDWALK, STAIR, & FIBERGLASS PEDESTRIAN BRIDGE LAYOUT FOR OWNER'S APPROVAL (INCLUDING ELEVATIONS) FOR APPROVAL PRIOR TO CONSTRUCTION.
4. ALL LUMBER / TIMBER SHALL BE SOUTHERN YELLOW PINE NUMBER 2.
5. ALL LUMBER / TIMBER SHALL BE OF SOUND STOCK, DELIVERED DRY, AND SHALL BE FULLY PROTECTED AT ALL TIMES FROM INJURY. DAMP, SPLIT, BROKEN, OR OTHERWISE DAMAGED PIECES WILL NOT BE ALLOWED IN THE WORK.
6. LUMBER / TIMBER SHALL BE TREATED WITH PRESERVATIVES TO THE FOLLOWING REQUIREMENTS IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD U1 OR ICC-ES EVALUATION REPORTS:
  - 6.1. 4"x6" PT SILLS: UC4B, 0.60 PCF ACQ
  - 6.2. STRUCTURAL LUMBER FOR DECKING, RAMPS, AND STAIRS- 2"x6", 2"x8", 2"x10", & 2"x12": UC4A, 0.40 PCF ACQ
  - 6.3. HANDRAIL SYSTEM INCLUDING 6"x6" POSTS, 2"x6" TOP & BOTTOM RAILS, & 2"x8" CAP BOARD: UC4A, 0.40 PCF ACQ
7. ALL TREATMENT SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS OF THE AMERICAN WOOD PROTECTION ASSOCIATION FOR TREATING WOOD. APPLY A HEAVY COAT OF THE SAME PRESERVATIVE USED IN TREATING TO ALL SURFACES CUT AFTER TREATMENT.
8. ALL HARDWARE (BOLTS, SCREWS, NAILS, ETC.) SHALL BE GALVANIZED.
9. CONNECTING HARDWARE SHALL BE AS FOLLOWS:
  - 9.1. 2"x10" DECK BOARD TO 2"x10" STRINGERS: (2) 4" LONG SIMPSON SDWS TIMBER SCREWS INTO EA. STRINGER BELOW DECKING, COUNTERSINK FASTENER HEADS.
  - 9.2. 2"x10" STRINGER TO 2"x10" STRINGER: (4) 3/8" GALVANIZED HEX BOLT W/ 2" FLAT WASHER.
  - 9.3. 2"x12" STAIR STRINGER TO 2"x10" DECK BOARD: (2) SIMPSON ANGLE BRACKET AT EACH STRINGER.
  - 9.4. 2"x12" STAIR TREAD TO 2"x12" STAIR STRINGER: (3) 3" GALVANIZED #10 WOOD SCREW AT EACH CONNECTION.
  - 9.5. 6"x6" HANDRAIL POSTS TO 2"x12" STAIR STRINGER: (2) 5/8" GALVANIZED HEX BOLT W/ 2" FLAT WASHERS.
  - 9.6. 2"x6" TOP RAIL TO 6"x6" HANDRAIL POSTS: (2) 3" GALVANIZED #10 SCREWS AT EACH POST.
  - 9.7. 2"x8" CAP BOARD TO 6"x6" HANDRAIL POST: (2) 3" GALVANIZED #10 SCREWS AT EACH POST.
  - 9.8. 2"x8" CAP BOARDS TO 2"x6" TOP RAIL: 3" GALVANIZED #10 SCREWS AT 24" O.C.
10. ENDS OF RAMPS SHALL BE SLIGHTLY EMBEDDED INTO THE EXISTING GRADE FOR A SMOOTH TRANSITION.
11. GALVANIZED WIRE ROPE SHALL BE RATED FOR TWO TIMES THE ESTIMATED WEIGHT OF THE STRUCTURE.
12. AFTER WOOD HAS CURED FOR AT LEAST 3 WEEKS IN DRY WEATHER, TOP OF DECK BOARDS SHALL BE COMPLETELY COATED WITH A NON-SKID SURFACE (RATIO: 1 PINT NON-SKID MATERIAL WITH 1 GALLON FLAT LATEX DECK STAIN). SUBMIT MIXTURE COMPONENT INFORMATION TO LANDSCAPE ARCHITECT FOR APPROVAL PRIOR TO BEGINNING WORK.
13. 4"x6" PRESSURE TREATED SILLS SHALL BE RATED FOR GROUND CONTACT AND PINNED WITH FOUR SECTIONS OF #5 REBAR, 2' LENGTHS. CONTRACTOR MAY PROPOSE ALTERNATE MATERIAL FOR BRIDGE SILLS.
14. FOR BOLTED WORK, BORE HOLES OF SAME DIAMETER AS BOLTS AND DRIVE BOLTS INTO PLACE.
15. WASHERS SHALL BE PROVIDED UNDER ALL BOLT HEADS AND NUTS.
16. BOLTS SHALL BE STANDARD HEX HEADS.
17. DECKING SHALL BE INSTALLED WITH BARK SIDE UP TO PREVENT CUPPING. MAX SPACING BETWEEN DECK BOARDS IS 1/4", UNLESS OTHERWISE NOTED.
18. EDGES OF ALL HANDRAIL CAP BOARDS SHALL BE SANDED SMOOTH WITH NO SHARP EDGES OR CORNERS.
19. DESIGN IS BASED ON A LIVE LOAD OF 90 POUNDS PER SQUARE FOOT.
20. ALL DEMOLITION MATERIALS AND OFF-CUTS SHALL BE REMOVED FROM THE SITE AND PROPERLY DISPOSED OF.
21. SUBMITTALS ARE REQUIRED ON ALL MATERIALS TO BE INCORPORATED INTO THE WORK.



**2 BREAKAWAY BOARDWALK ENLARGEMENT PLAN**  
SCALE: 1" = 1'-0"



**4 TYP. DECKING ATTACHMENT DETAIL**  
SCALE: 1" = 1'-0"

**5 FIBERGLASS BRIDGE SUPPORT DETAIL**  
SCALE: 1" = 1'-0"



Seal:



Date: 03/18/2024

Project No: 2022-024

Drawn By: PS

Checked By: MS

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:

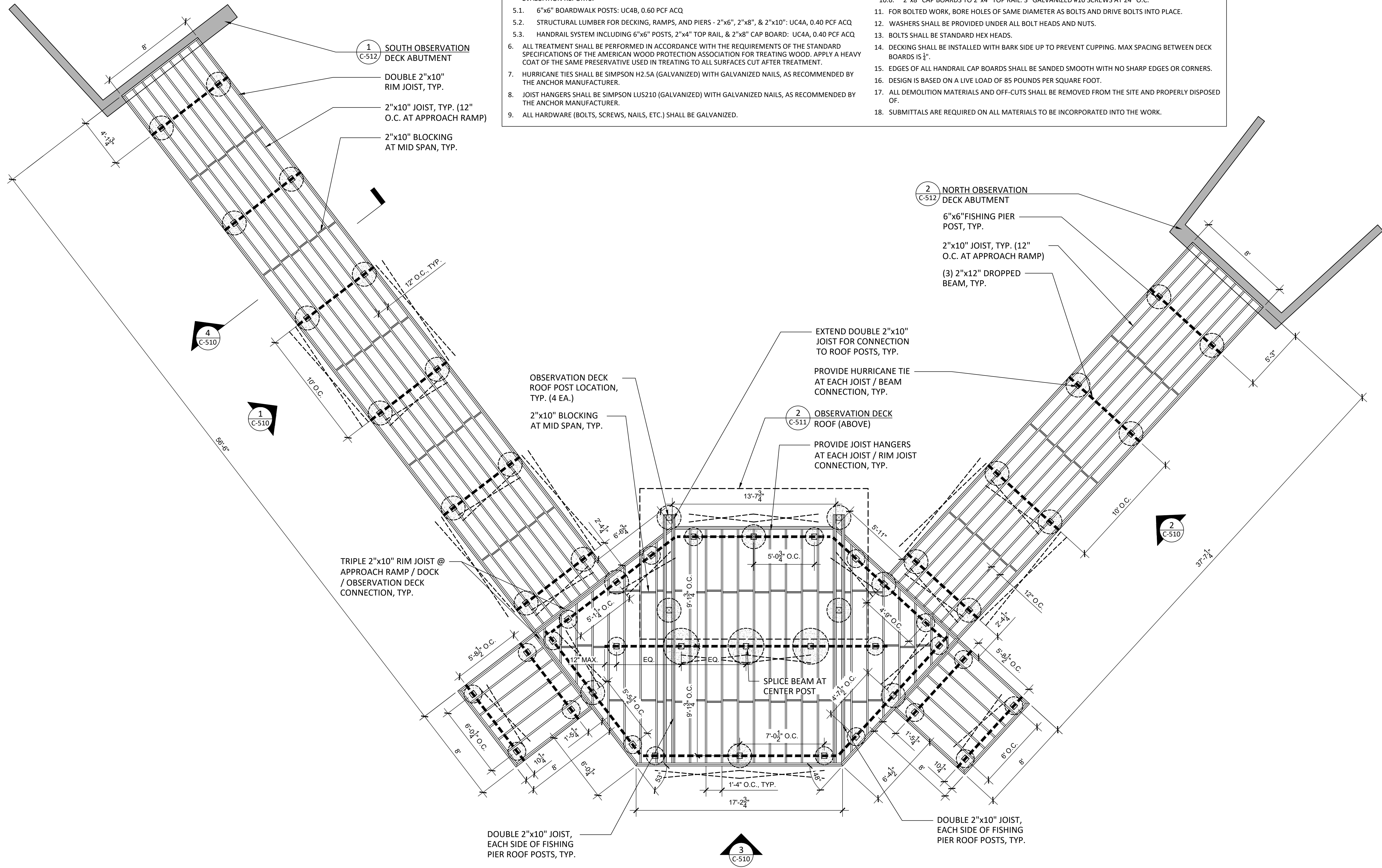
**Construction Details**

Sheet No:

**C-509A**

**FISHING PIER NOTES:**

- CONTRACTOR SHALL VISIT THE SITE AND MAKE THEM SELF FAMILIAR WITH SITE CONDITIONS.
- CONTRACTOR SHALL STAKE OUT FISHING PIER LAYOUT FOR OWNER'S APPROVAL (INCLUDING ELEVATIONS) FOR APPROVAL PRIOR TO CONSTRUCTION.
- ALL LUMBER / TIMBER SHALL BE SOUTHERN YELLOW PINE NUMBER 1.
- ALL LUMBER / TIMBER SHALL BE OF SOUND STOCK, DELIVERED DRY, AND SHALL BE FULLY PROTECTED AT ALL TIMES FROM INJURY, DAMP, SPLIT, BROKEN, OR OTHERWISE DAMAGED PIECES WILL NOT BE ALLOWED IN THE WORK.
- LUMBER / TIMBER SHALL BE TREATED WITH PRESERVATIVES TO THE FOLLOWING REQUIREMENTS IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD U1 OR ICC-ES EVALUATION REPORTS:
  - 6"x6" BOARDWALK POSTS: UC4B, 0.60 PCF ACQ
  - STRUCTURAL LUMBER FOR DECKING, RAMPS, AND PIERS - 2"x6", 2"x8", & 2"x10": UC4A, 0.40 PCF ACQ
  - HANDRAIL SYSTEM INCLUDING 6"x6" POSTS, 2"x4" TOP RAIL, & 2"x8" CAP BOARD: UC4A, 0.40 PCF ACQ
- ALL TREATMENT SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS OF THE AMERICAN WOOD PROTECTION ASSOCIATION FOR TREATING WOOD. APPLY A HEAVY COAT OF THE SAME PRESERVATIVE USED IN TREATING TO ALL SURFACES CUT AFTER TREATMENT.
- HURRICANE TIES SHALL BE SIMPSON H2.5A (GALVANIZED) WITH GALVANIZED NAILS, AS RECOMMENDED BY THE ANCHOR MANUFACTURER.
- JOIST HANGERS SHALL BE SIMPSON LUS210 (GALVANIZED) WITH GALVANIZED NAILS, AS RECOMMENDED BY THE ANCHOR MANUFACTURER.
- ALL HARDWARE (BOLTS, SCREWS, NAILS, ETC.) SHALL BE GALVANIZED.
- CONNECTING HARDWARE SHALL BE AS FOLLOWS:
  - 2"x10" SPLIT BEAM TO 6"x6" POSTS: (2) 3/8" Ø GALVANIZED HEX BOLT W/ 2" FLAT WASHERS.
  - 2"x10" JOISTS TO 2"x10" BEAMS: HURRICANE TIE
  - 2"x6" DECKING TO 2"x10" JOISTS: (3) 3" GALVANIZED #10 SCREWS AT EACH JOIST.
  - 2"x10" BLOCKING TO 2"x10" JOISTS: (3) 3" GALVANIZED #10 SCREWS AT EACH BLOCK.
  - 6"x6" HANDRAIL POSTS TO DOUBLE 2"x10" RIM JOISTS: (2) 3/8" Ø GALVANIZED HEX BOLT W/ 2" FLAT WASHERS.
  - 2"x4" TOP RAIL TO 6"x6" HANDRAIL POSTS: (2) 3" GALVANIZED #10 SCREWS AT EACH POST.
  - 2"x8" CAP BOARD TO 6"x6" HANDRAIL POST: (2) 3" GALVANIZED #10 SCREWS AT EACH POST.
  - 2"x8" CAP BOARDS TO 2"x4" TOP RAIL: 3" GALVANIZED #10 SCREWS AT 24" O.C.
  - FOR BOLTED WORK, BORE HOLES OF SAME DIAMETER AS BOLTS AND DRIVE BOLTS INTO PLACE.
  - WASHERS SHALL BE PROVIDED UNDER ALL BOLT HEADS AND NUTS.
  - BOLTS SHALL BE STANDARD HEX HEADS.
  - DECKING SHALL BE INSTALLED WITH BARK SIDE UP TO PREVENT CUPPING. MAX SPACING BETWEEN DECK BOARDS IS 1/4".
  - EDGES OF ALL HANDRAIL CAP BOARDS SHALL BE SANDED SMOOTH WITH NO SHARP EDGES OR CORNERS.
  - DESIGN IS BASED ON A LIVE LOAD OF 85 POUNDS PER SQUARE FOOT.
  - ALL DEMOLITION MATERIALS AND OFF-CUTS SHALL BE REMOVED FROM THE SITE AND PROPERLY DISPOSED OF.
  - SUBMITTALS ARE REQUIRED ON ALL MATERIALS TO BE INCORPORATED INTO THE WORK.



**1 OBSERVATION DECK FRAMING PLAN**  
SCALE: 1/4" = 1'-0"



Seal:



Date: 03/18/2024

Project No: 2022-024

Drawn By: PS

Checked By: MS

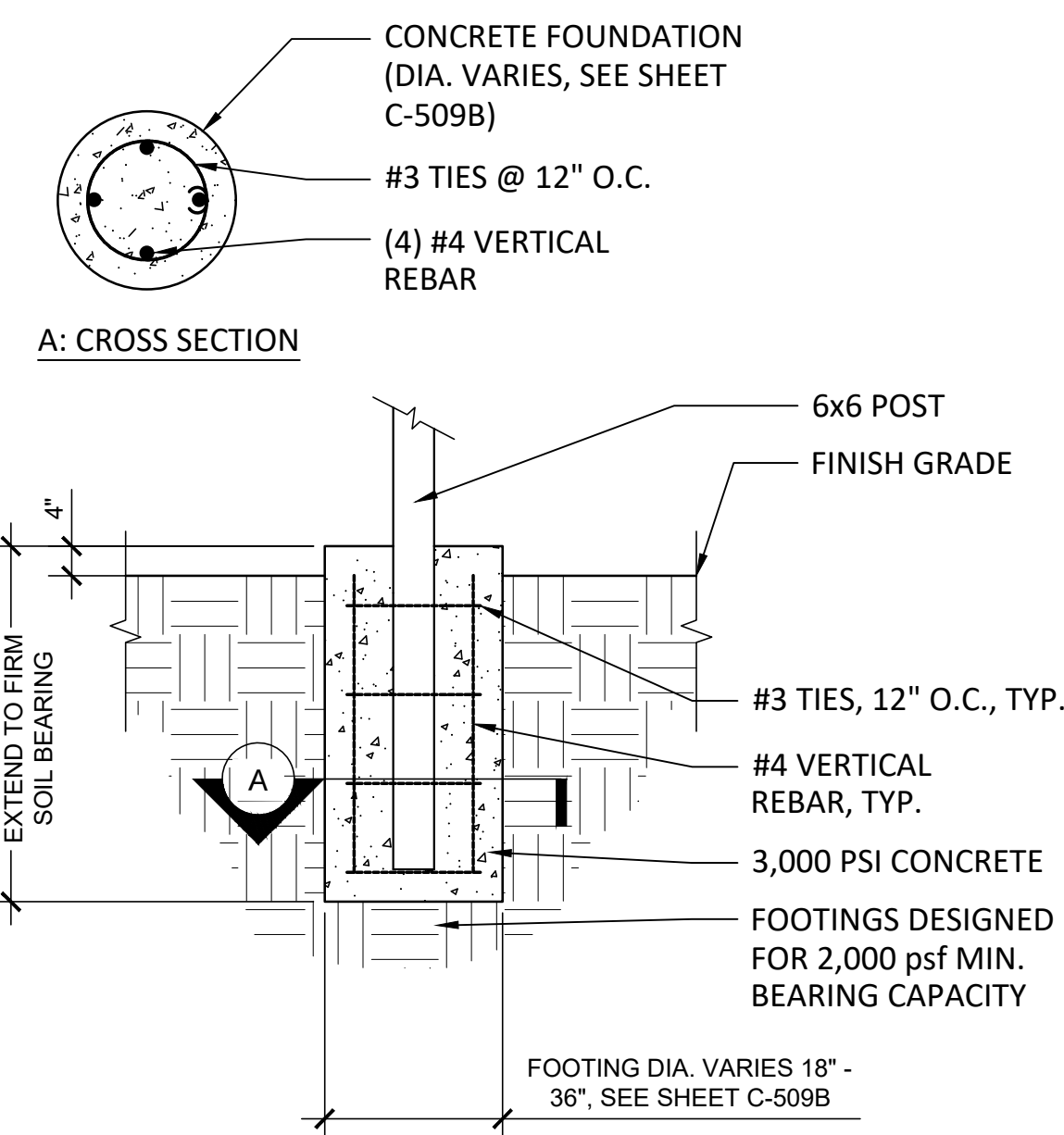
**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

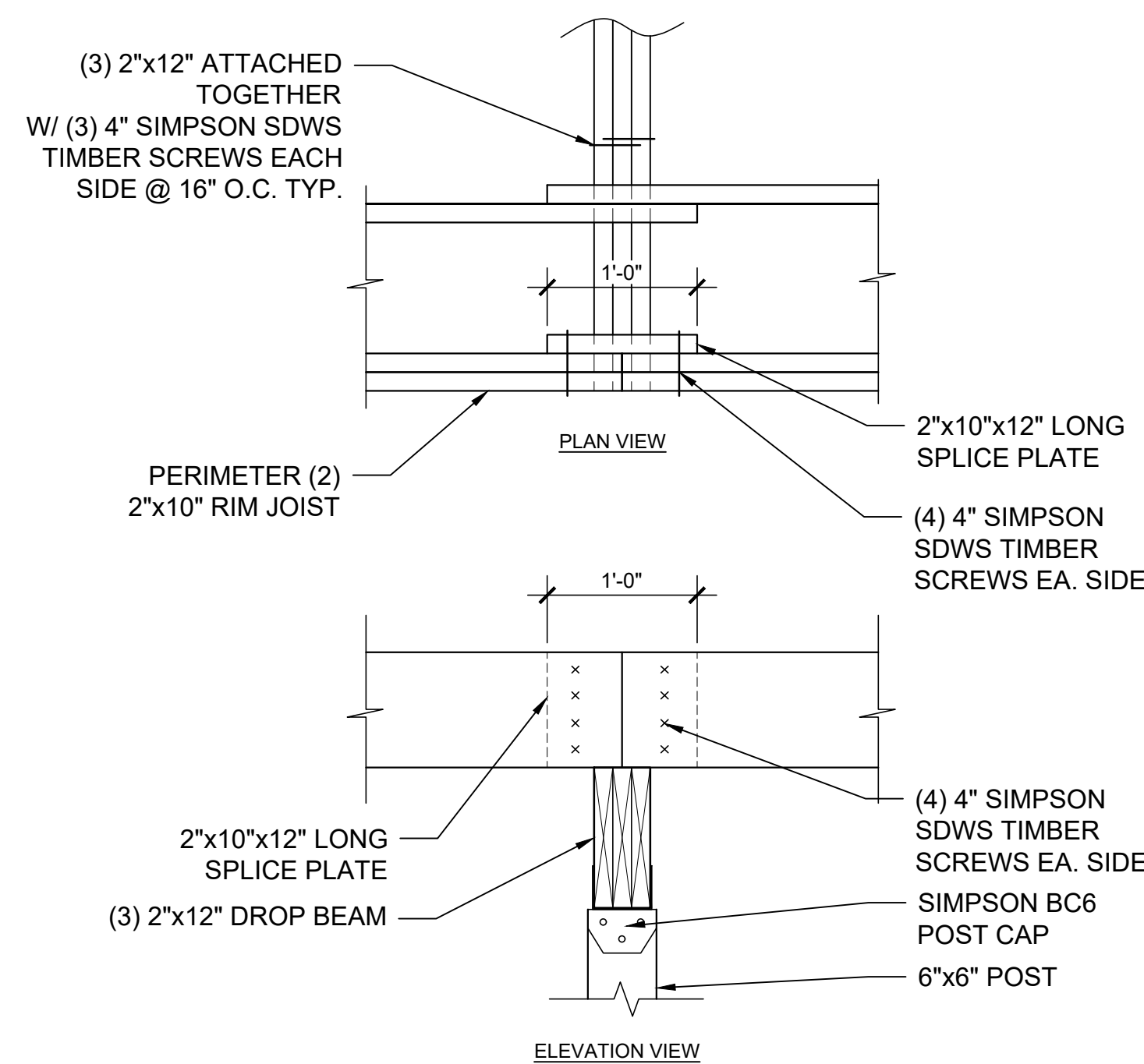
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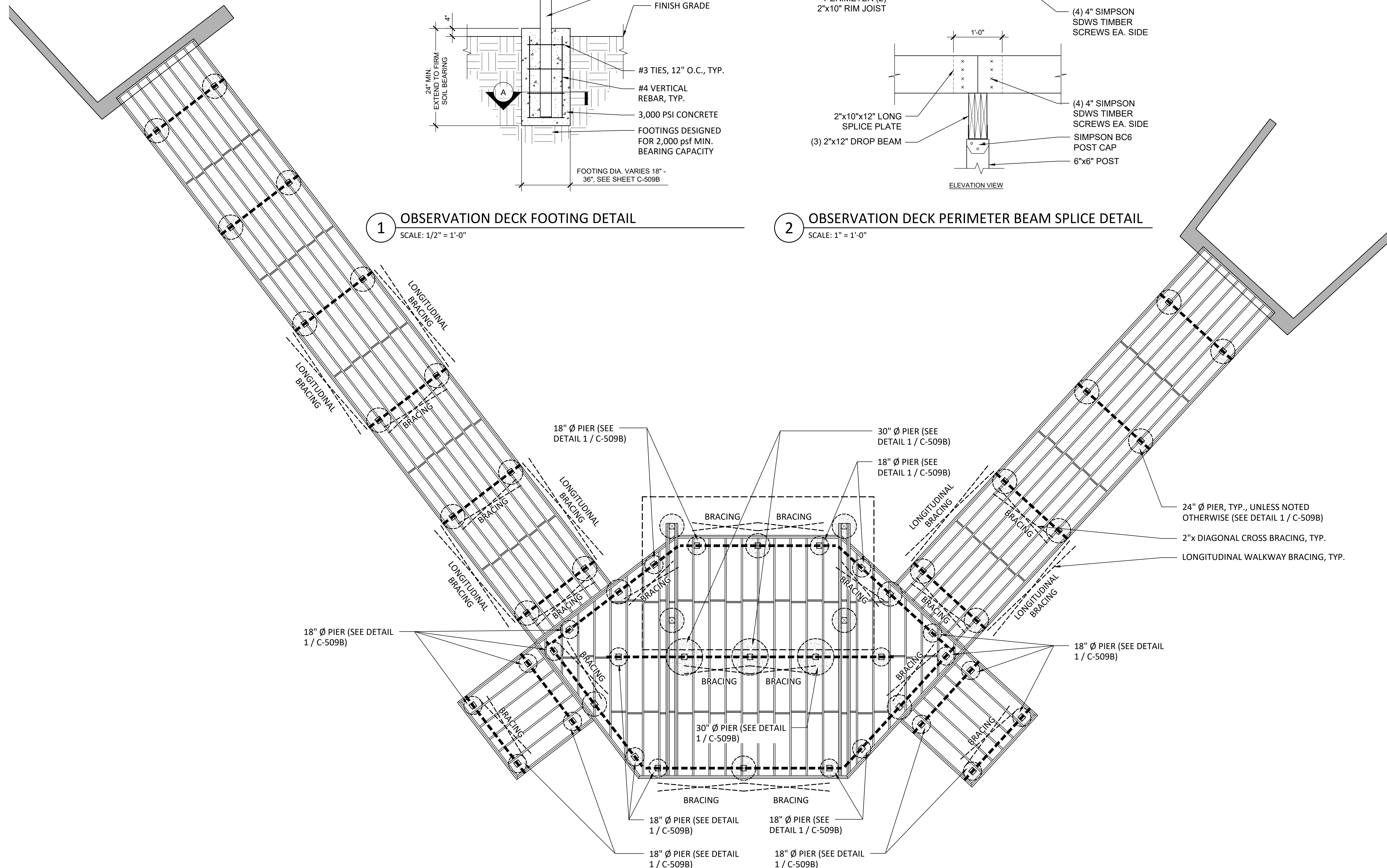
Sheet No:  
**C-509B**



**1** OBSERVATION DECK FOOTING DETAIL  
SCALE: 1/2" = 1'-0"



**2** OBSERVATION DECK PERIMETER BEAM SPLICE DETAIL  
SCALE: 1" = 1'-0"



**3** OBSERVATION DECK FOOTING & BRACING PLAN  
SCALE: 1/4" = 1'-0"

RELEASED FOR CONSTRUCTION



Seal:

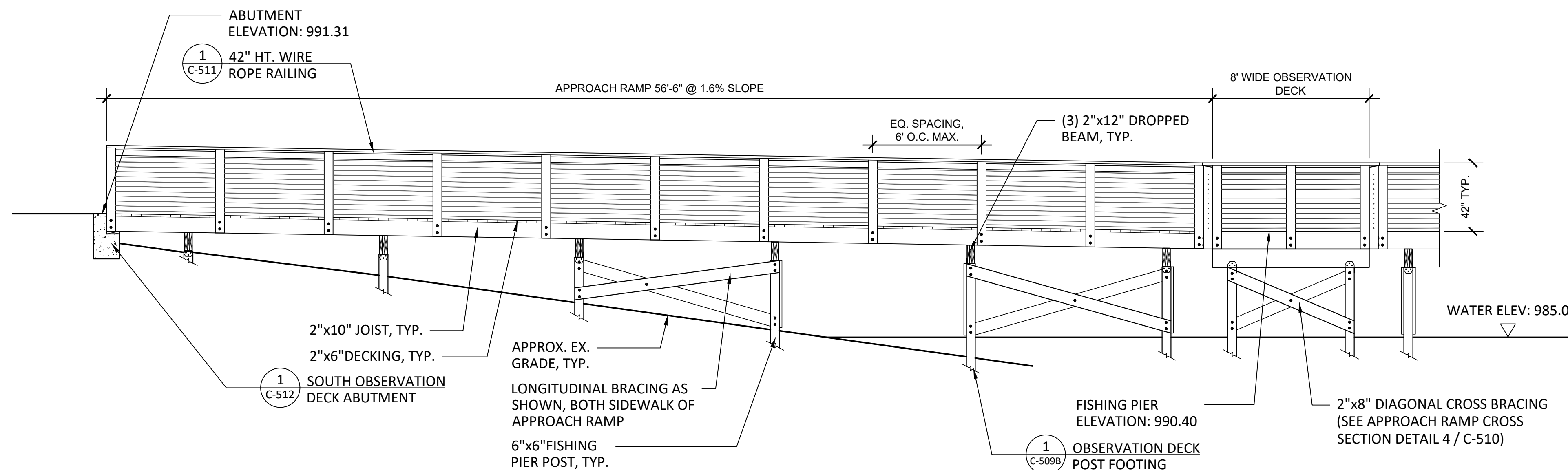


Date: 03/18/2024

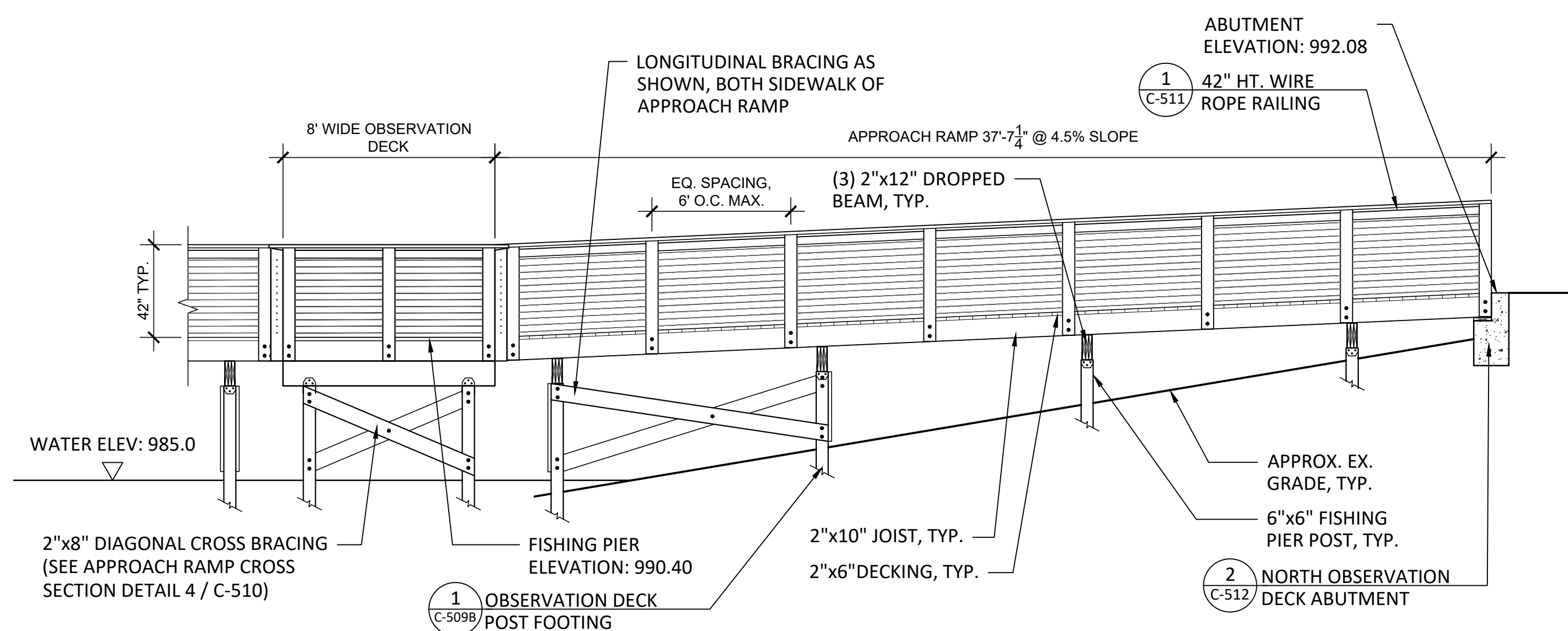
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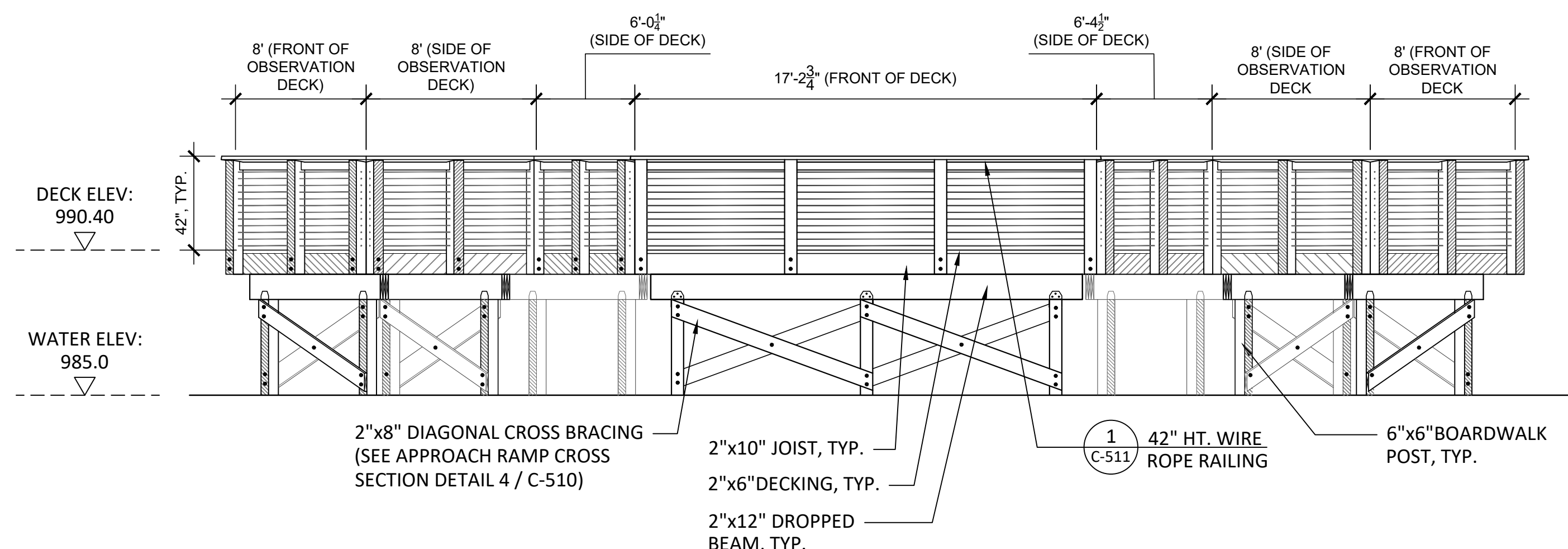
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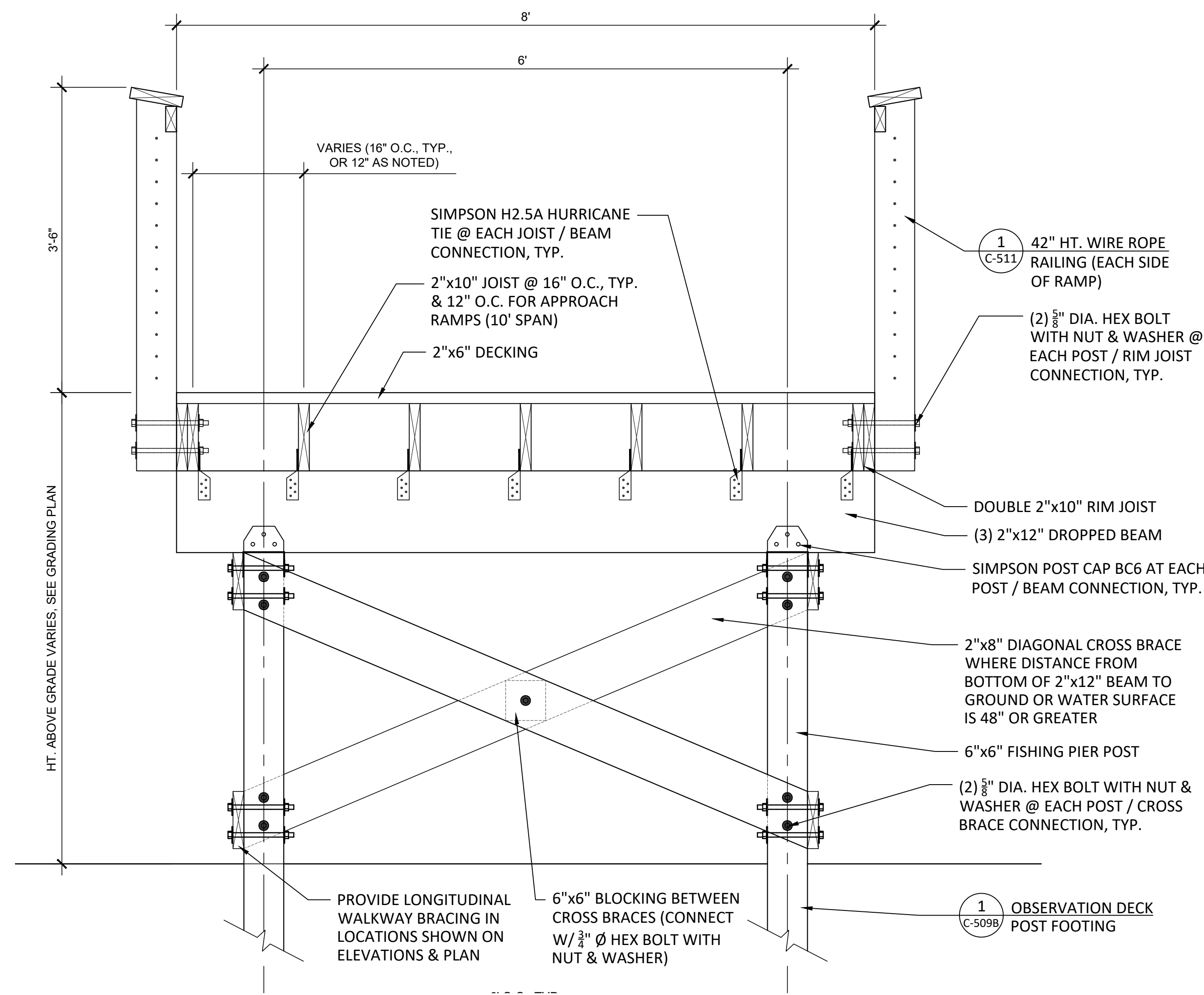
**1** OBSERVATION DECK SOUTH APPROACH RAMP ELEVATION  
SCALE: 1/4" = 1'-0"



**2** OBSERVATION DECK NORTH APPROACH RAMP ELEVATION  
SCALE: 1/4" = 1'-0"



**3** OBSERVATION DECK FRONT ELEVATION  
SCALE: 1/4" = 1'-0"



**4** OBSERVATION DECK APPROACH RAMP CROSS SECTION  
SCALE: 1" = 1'-0"

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

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**C-510**

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Date: 03/18/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MS

**Johns Homestead Park & Recreation Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

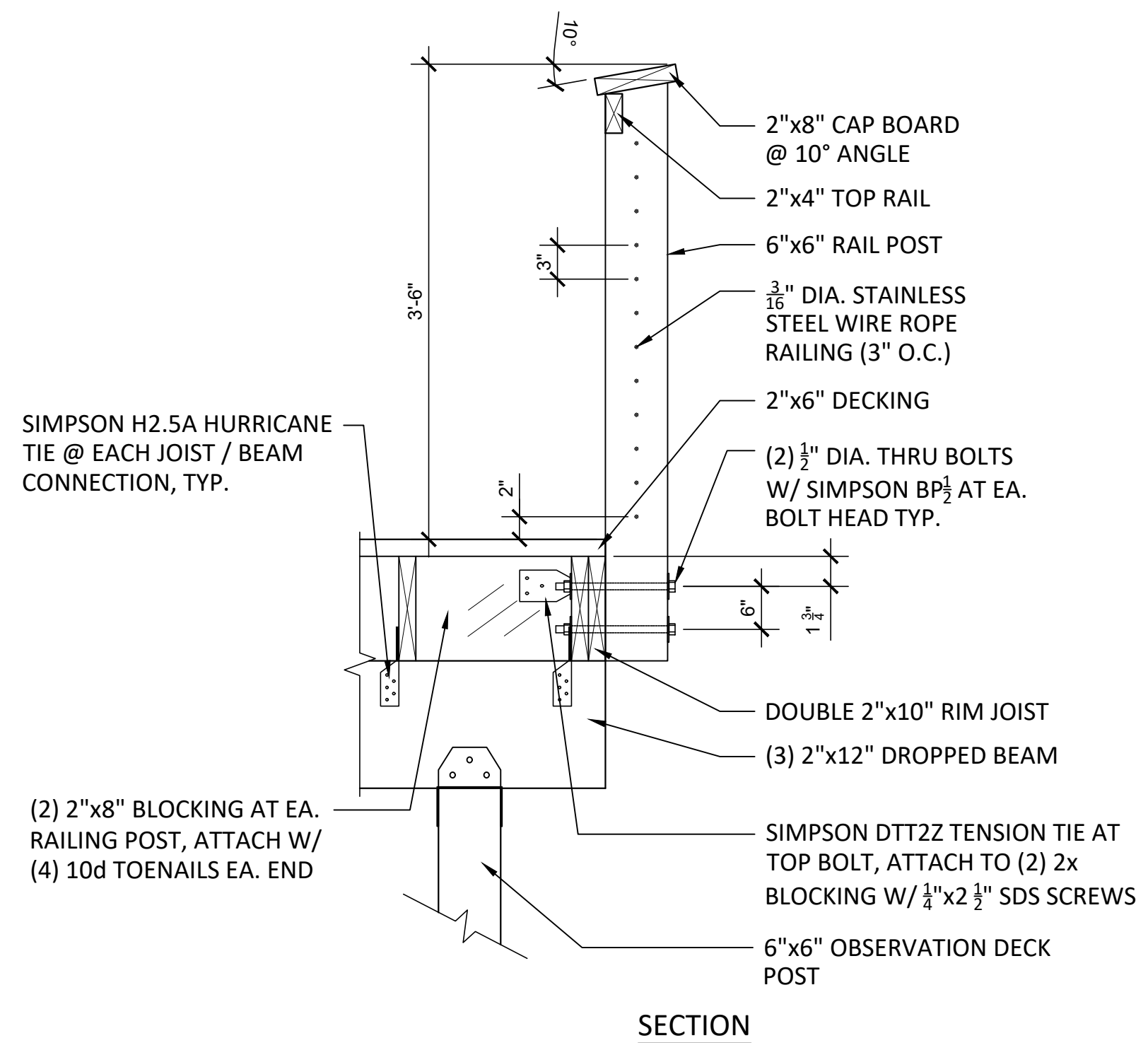
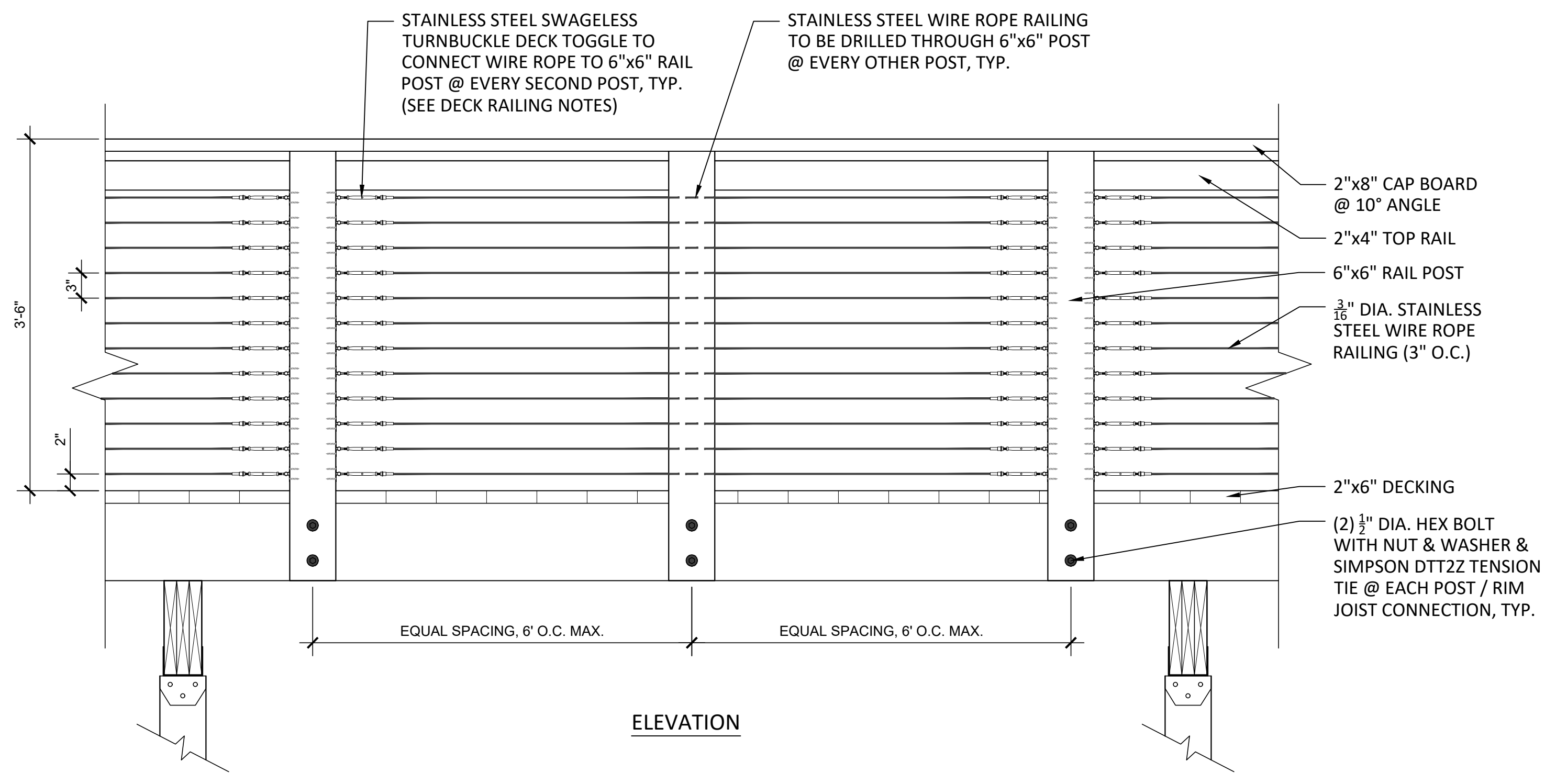
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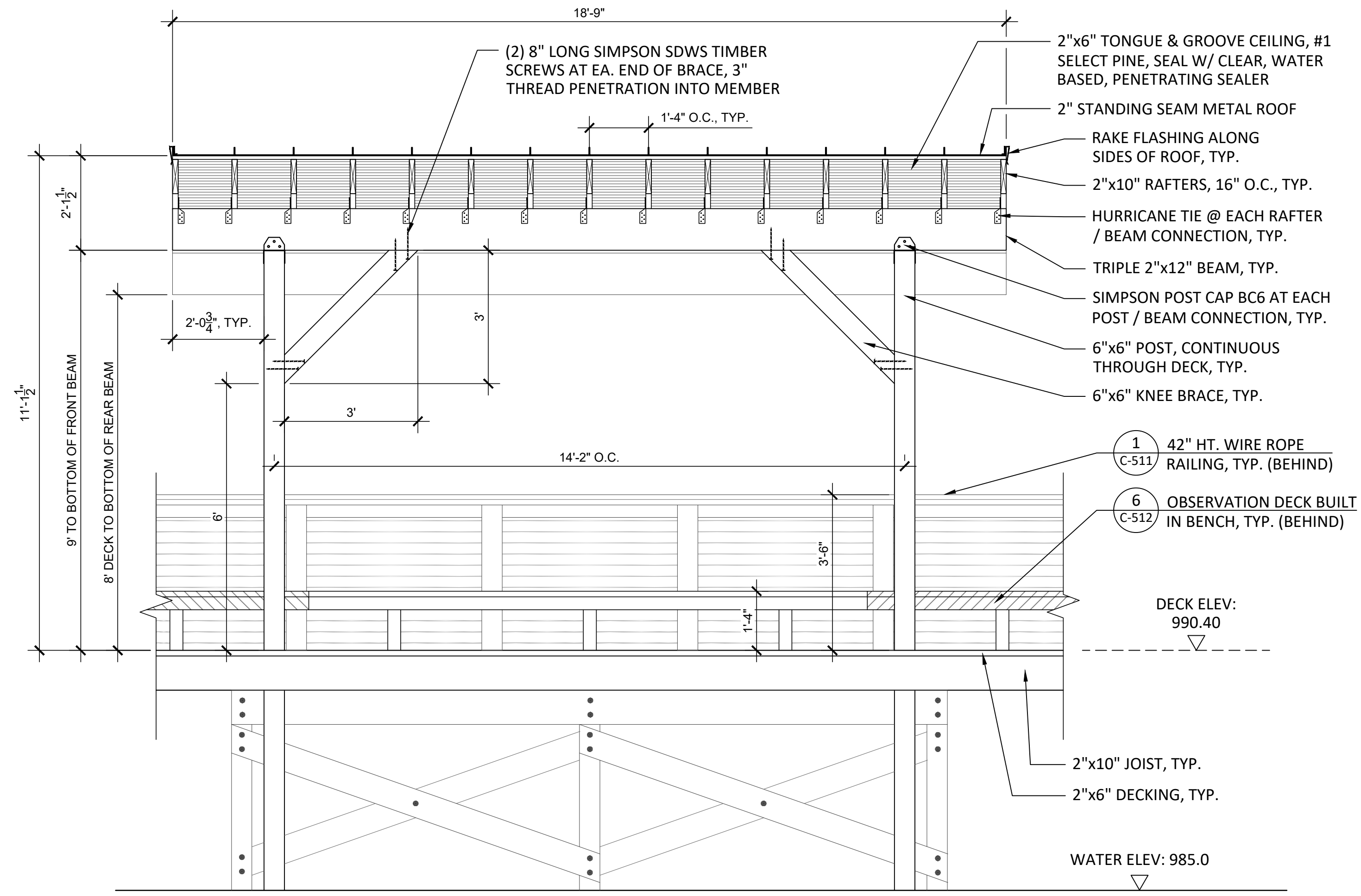
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Sheet No:  
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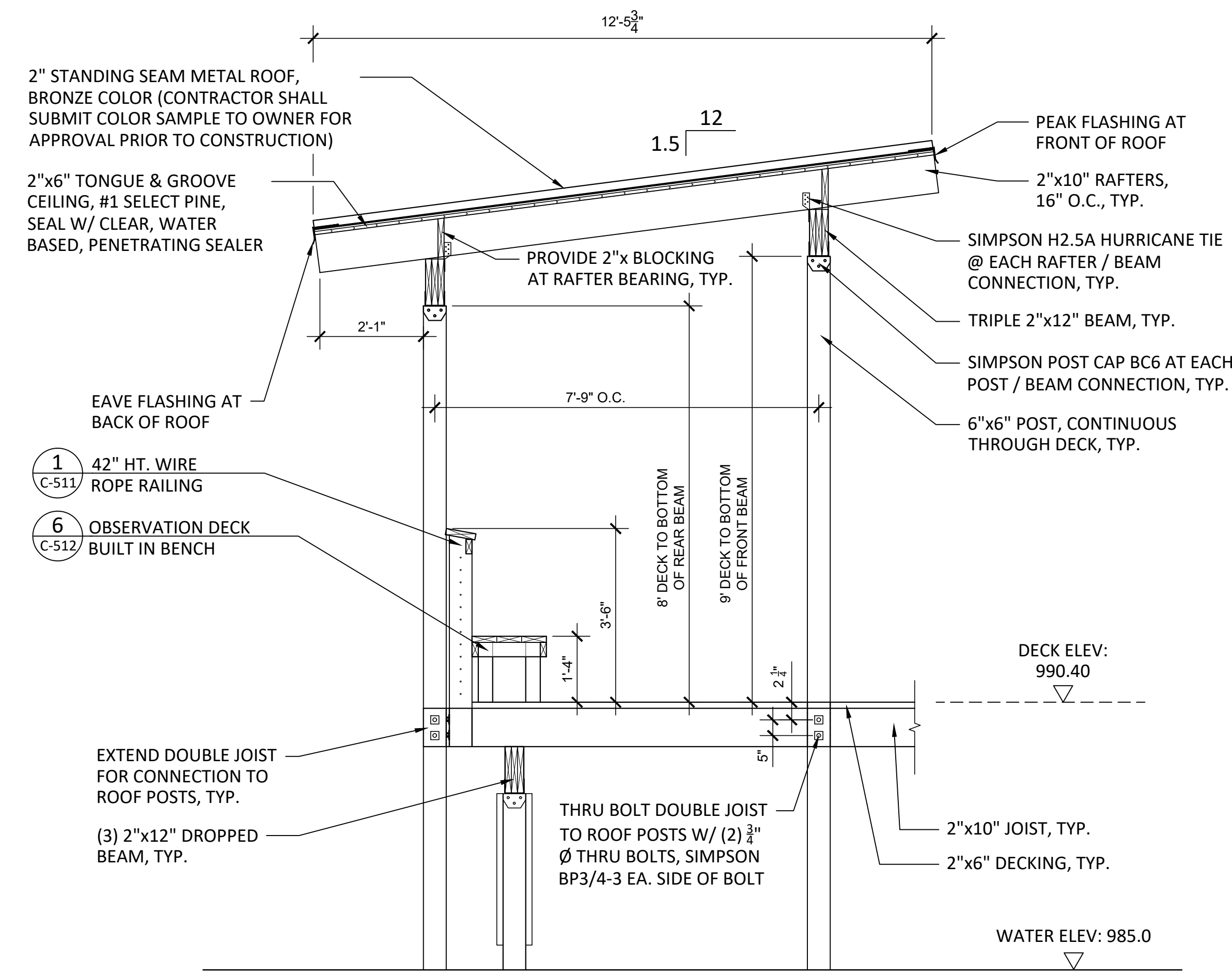
- DECK RAILING NOTES:**
1. STAINLESS STEEL WIRE ROPE RAILING SHALL BE CONNECTED DIRECTLY TO 6"x6" RAIL POST AT EVERY OTHER POST LOCATION (TO LIMIT SAG IN RAILING) WITH STAINLESS STEEL SWAGELESS TURNBUCKLE DECK TOGGLE.
  2. CONNECT SWAGELESS TURNBUCKLE TO RAIL POST W/ STAINLESS STEEL WOOD SCREWS.
  3. CONTRACTOR SHALL SUBMIT PRODUCT DATA FOR WIRE ROPE RAILING AND CONNECTING HARDWARE TO PROJECT LANDSCAPE ARCHITECT FOR APPROVAL PRIOR TO CONSTRUCTION.



**1 42" HT. WIRE ROPE RAILING DETAIL**  
SCALE: 1" = 1'-0"

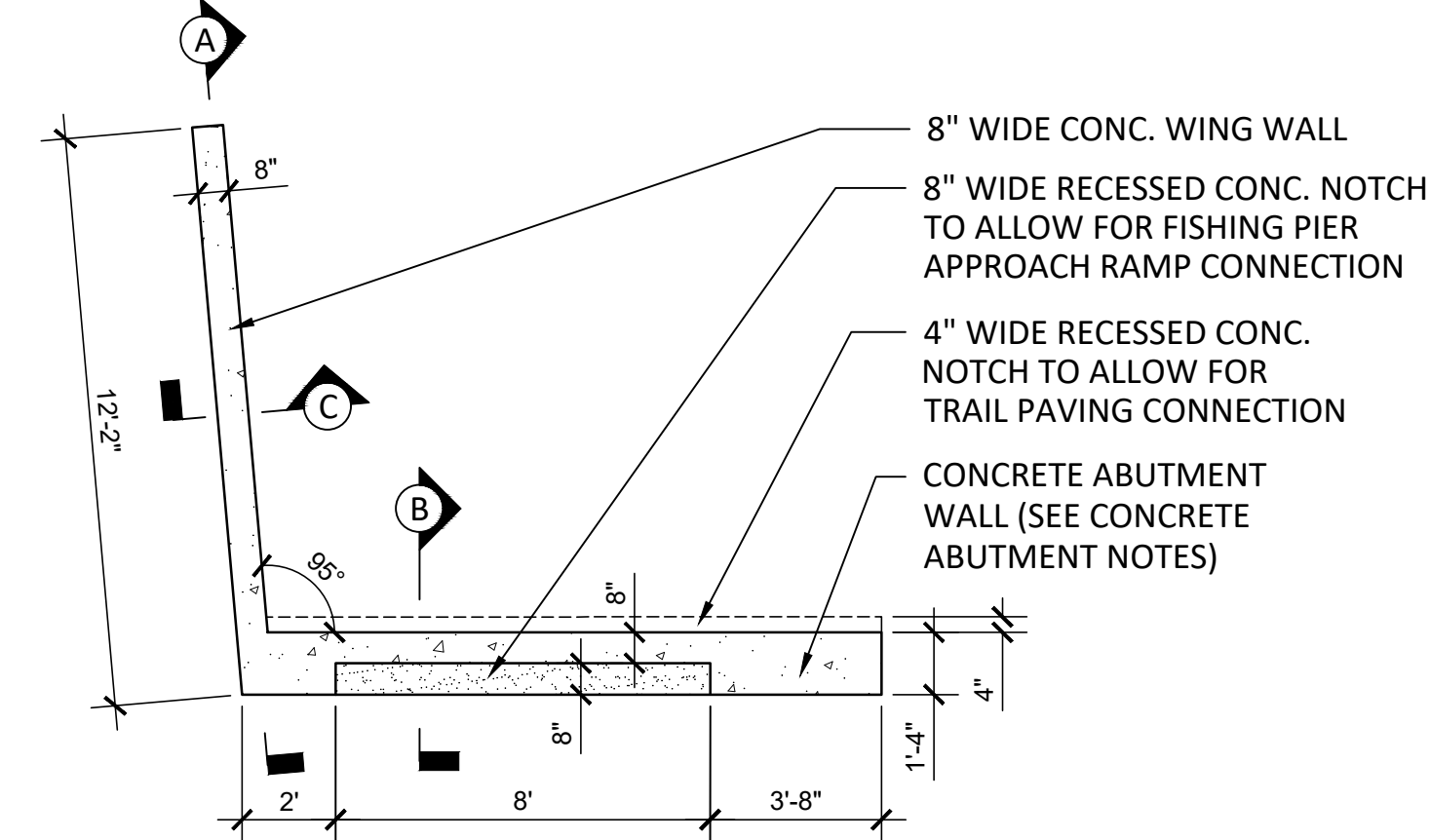


**2 OBSERVATION DECK ROOF - FRONT ELEVATION**  
SCALE: 1/2" = 1'-0"

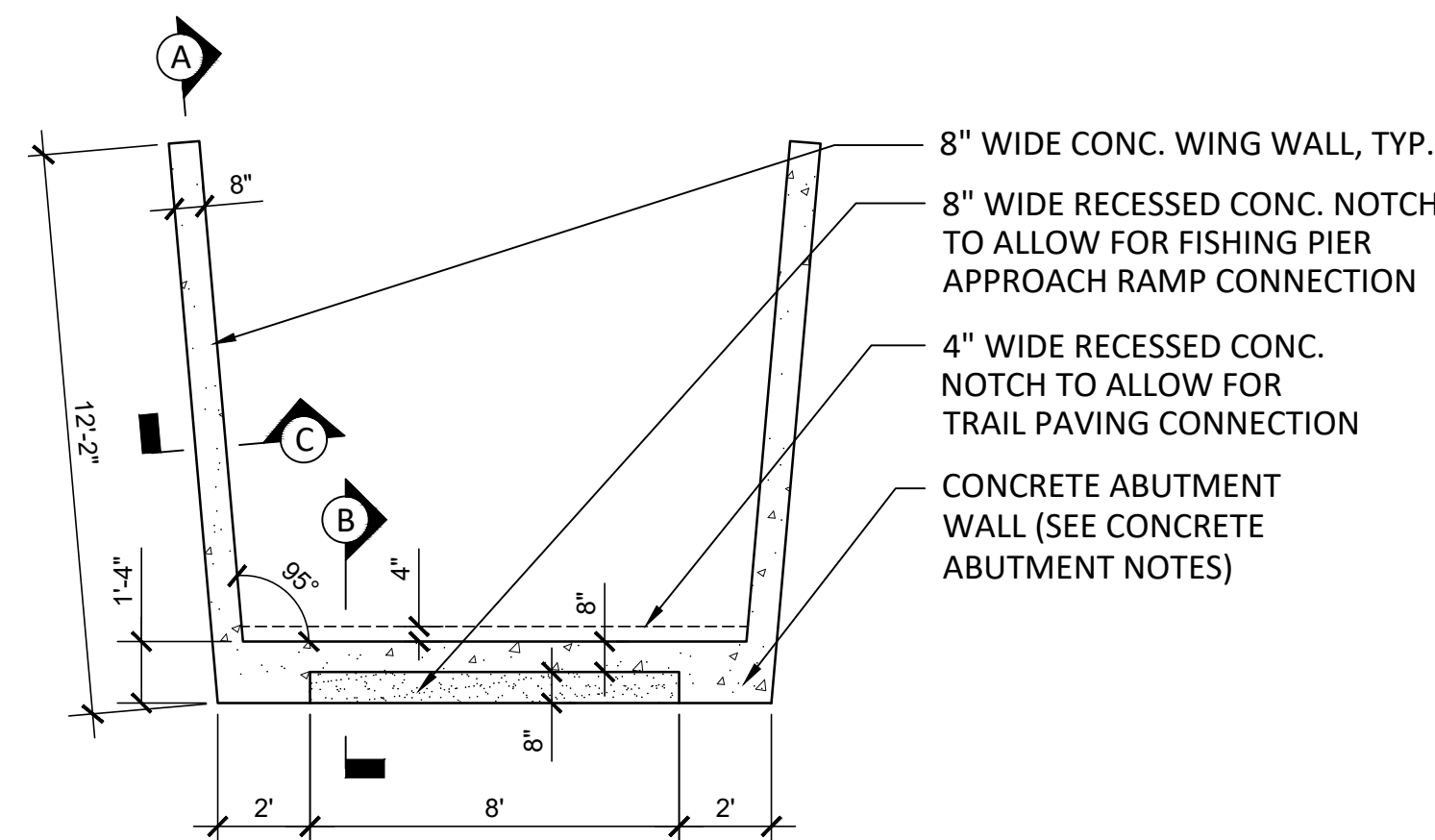


**3 OBSERVATION DECK ROOF - SIDE ELEVATION**  
SCALE: 1/2" = 1'-0"

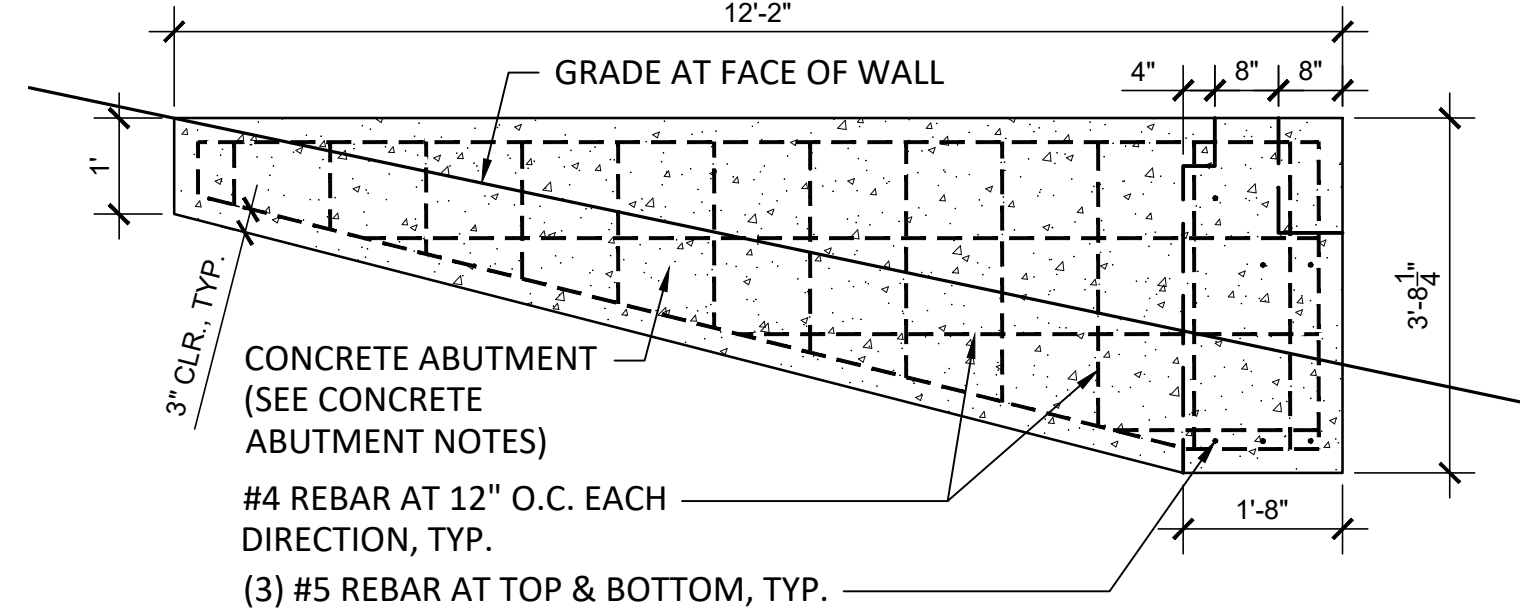




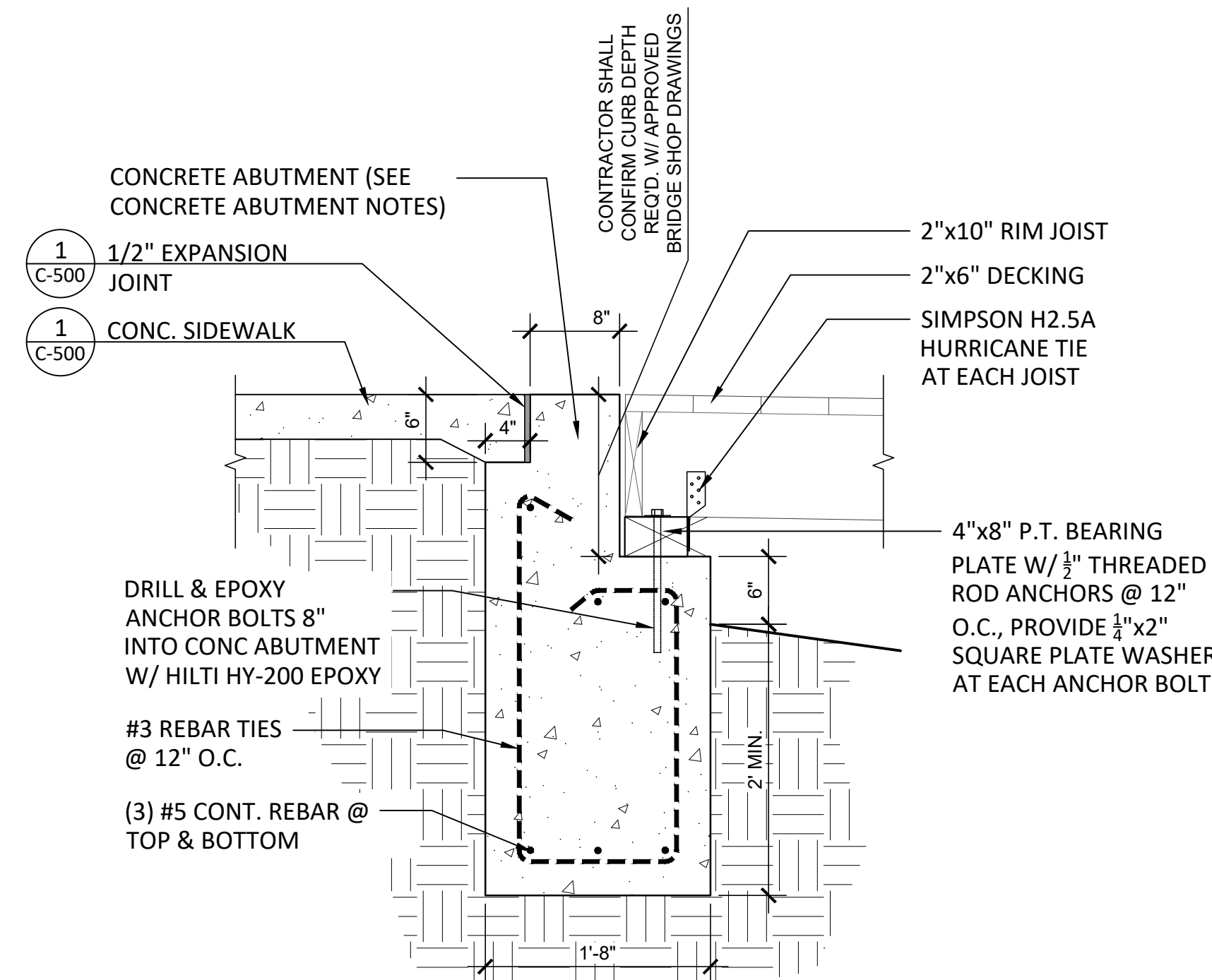
**1** OBSERVATION DECK SOUTH ABUTMENT ENLARGEMENT PLAN  
SCALE: 1/4" = 1'-0"



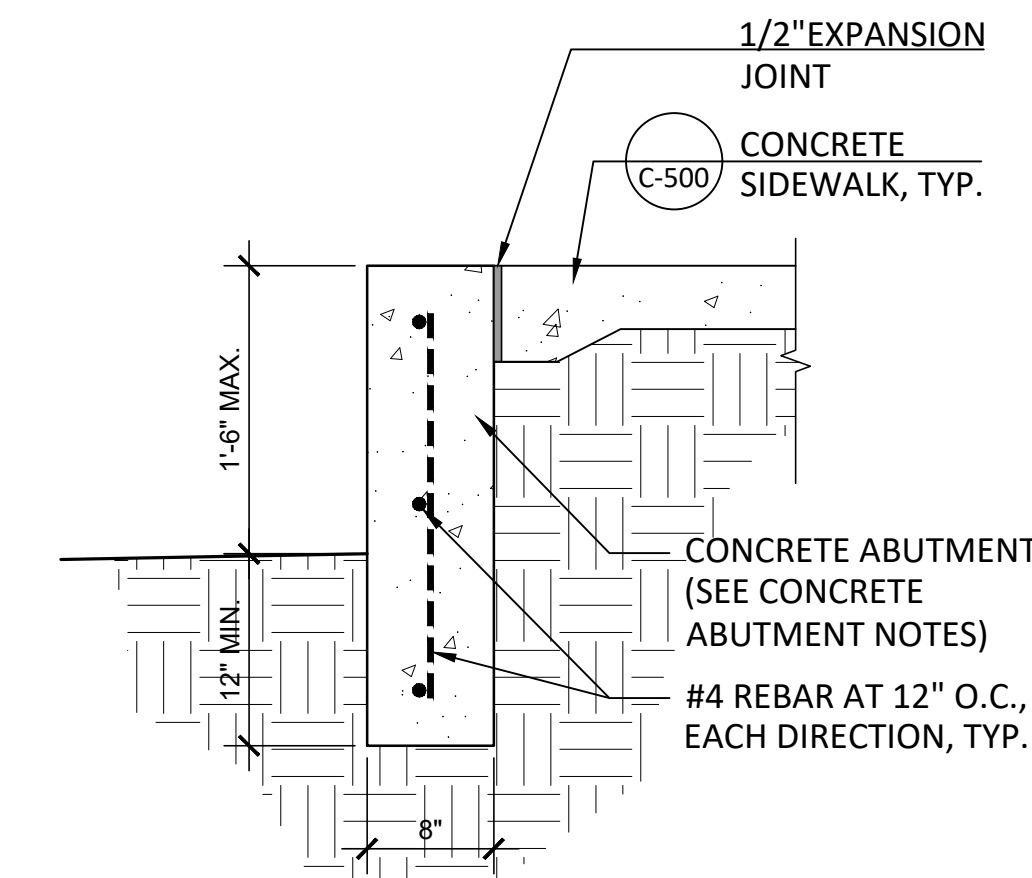
**2** OBSERVATION DECK ABUTMENT NORTH ENLARGEMENT PLAN  
SCALE: 1/4" = 1'-0"



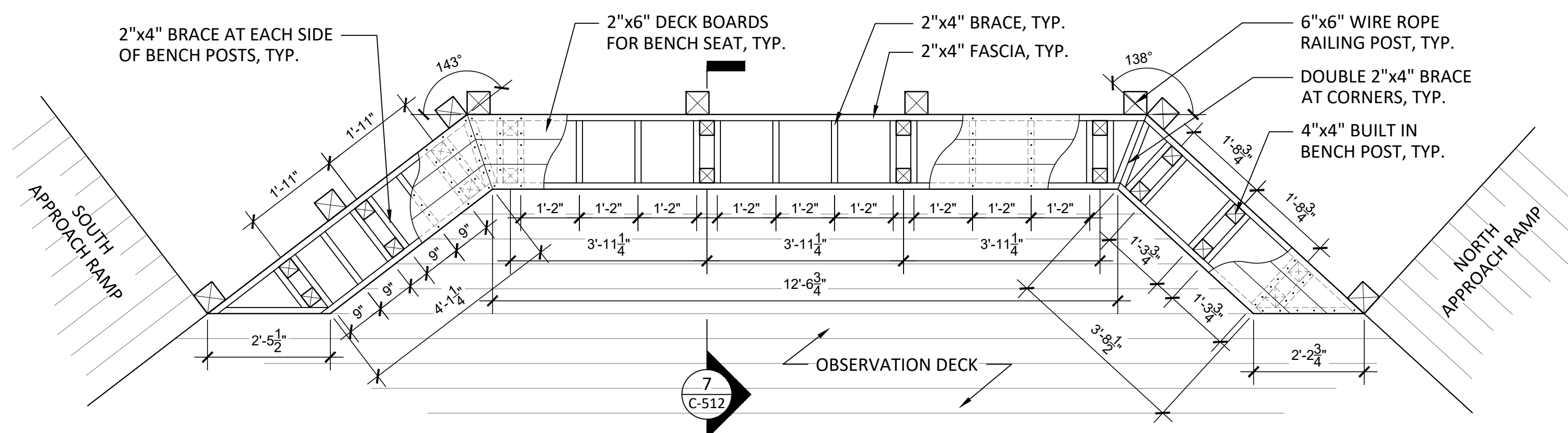
**3** OBSERVATION DECK ABUTMENT - SECTION A  
SCALE: 1/2" = 1'-0"



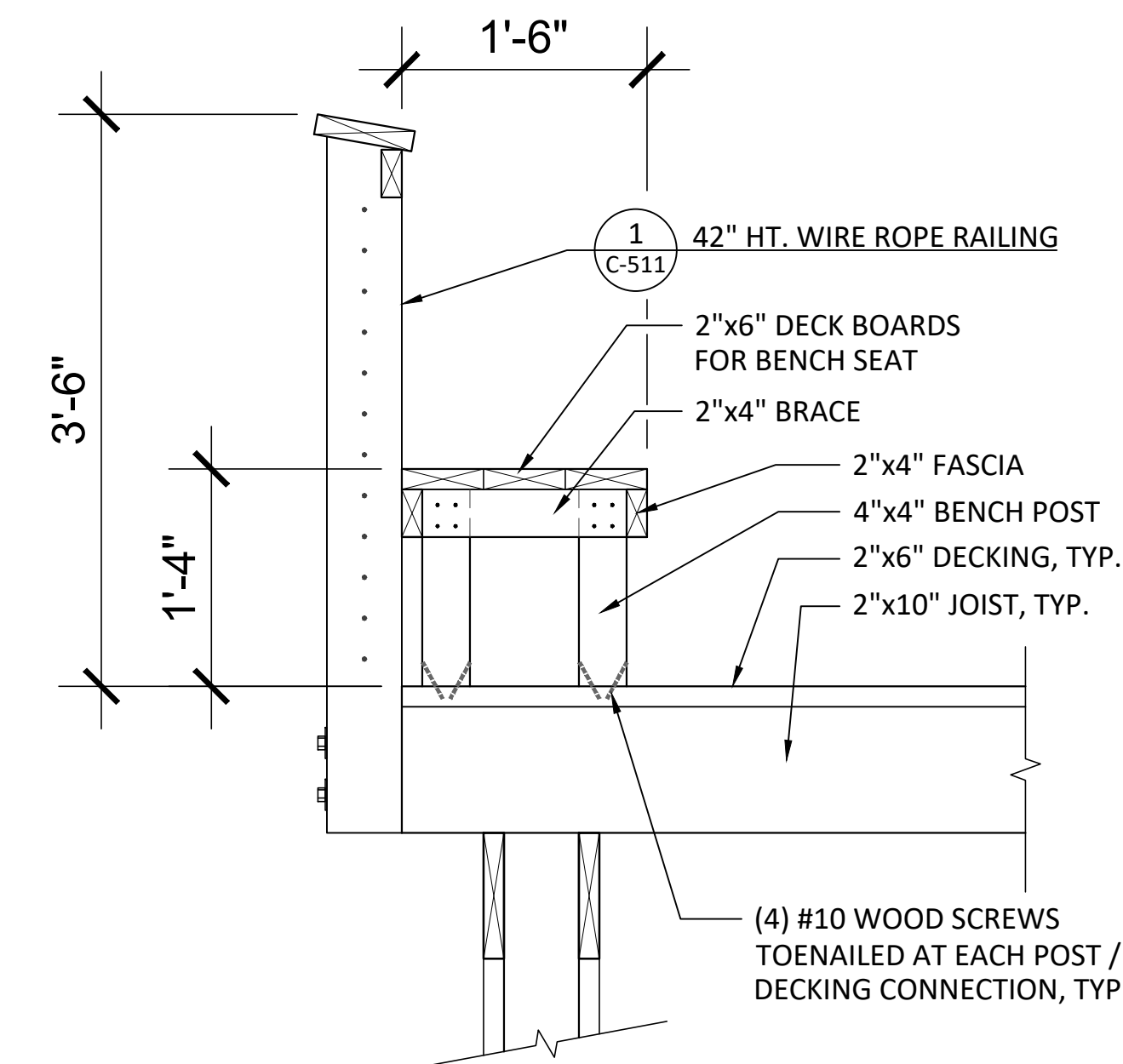
**4** OBSERVATION DECK ABUTMENT - SECTION B  
SCALE: 1" = 1'-0"



**5** OBSERVATION DECK ABUTMENT - SECTION C  
SCALE: 1" = 1'-0"



**6** OBSERVATION DECK BUILT IN BENCH FRAMING PLAN  
SCALE: 1/2" = 1'-0"



**7** OBSERVATION DECK BUILT IN BENCH SECTION  
SCALE: 1" = 1'-0"

**CONCRETE ABUTMENT NOTES:**

- FOOTINGS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. ALL FOOTINGS SHALL BEAR ON ORIGINAL EARTH OR COMPACTED FILL.
- THE BOTTOM OF ALL FOUNDATIONS SHALL EXTEND A MINIMUM OF TWENTY FOUR (24) INCHES BELOW FINAL FINISHED GRADE.
- FILL UNDER FOUNDATIONS SHALL BE OF A TYPE APPROVED BY A SOILS TESTING LABORATORY. COMPACTION SHALL BE TO 95% MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR TEXT (ASTM D968).
- CONCRETE SHALL DEVELOP A MINIMUM 28-DAY COMPRESSIVE STRENGTH F'c = 3000 PSI.
- UNLESS OTHERWISE NOTED, MINIMUM CONCRETE COVER FOR REINF. SHALL BE AS FOLLOWS:  
- CONCRETE CAST AGAINST EARTH: 3"  
- FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 2"
- CHAMFER ALL EXPOSED EDGES OF CONCRETE 3/4".
- BAR REINFORCEMENT SHALL CONFORM TO ASTM-615, GRADE 60.
- PRIOR TO PLACING CONCRETE, ALL REINFORCING STEEL SHALL BE FREE OF RUST, SCALE, OR ANY FOREIGN MATERIAL.
- ALL CONSTRUCTION SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE - 2012 EDITION.
- VERIFY ALL DIMENSIONS IN THE FIELD AND COORDINATE WITH SITE PLANS BEFORE ORDERING MATERIALS OR STARTING CONSTRUCTION.



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Date: 03/18/2024  
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Checked By: MS

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

**Revisions:**

NO.	DATE	DESCRIPTION

Sheet Title:

**Construction Details**

Sheet No:

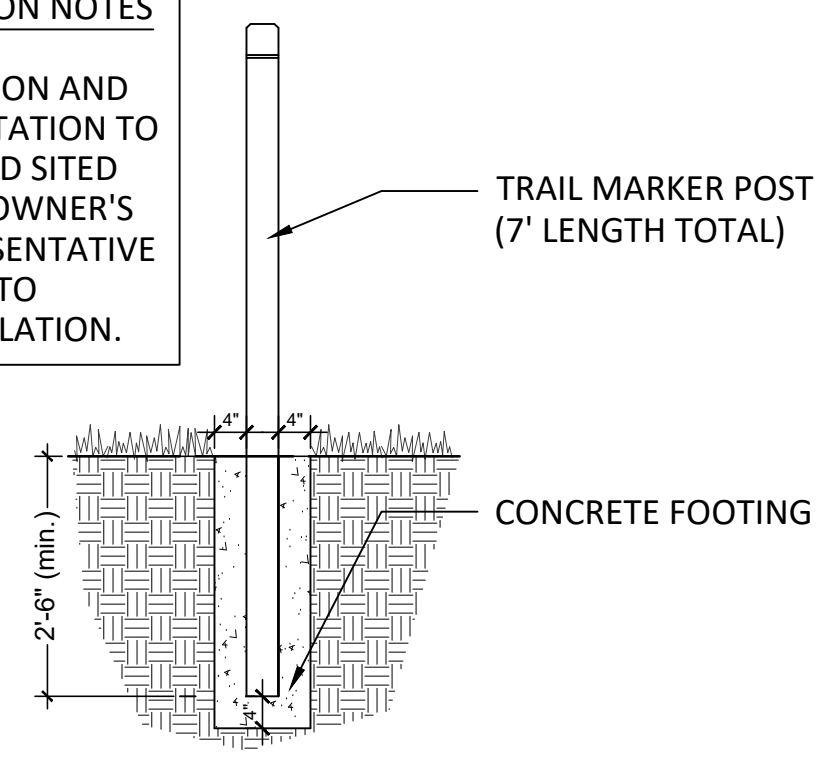
**C-512**



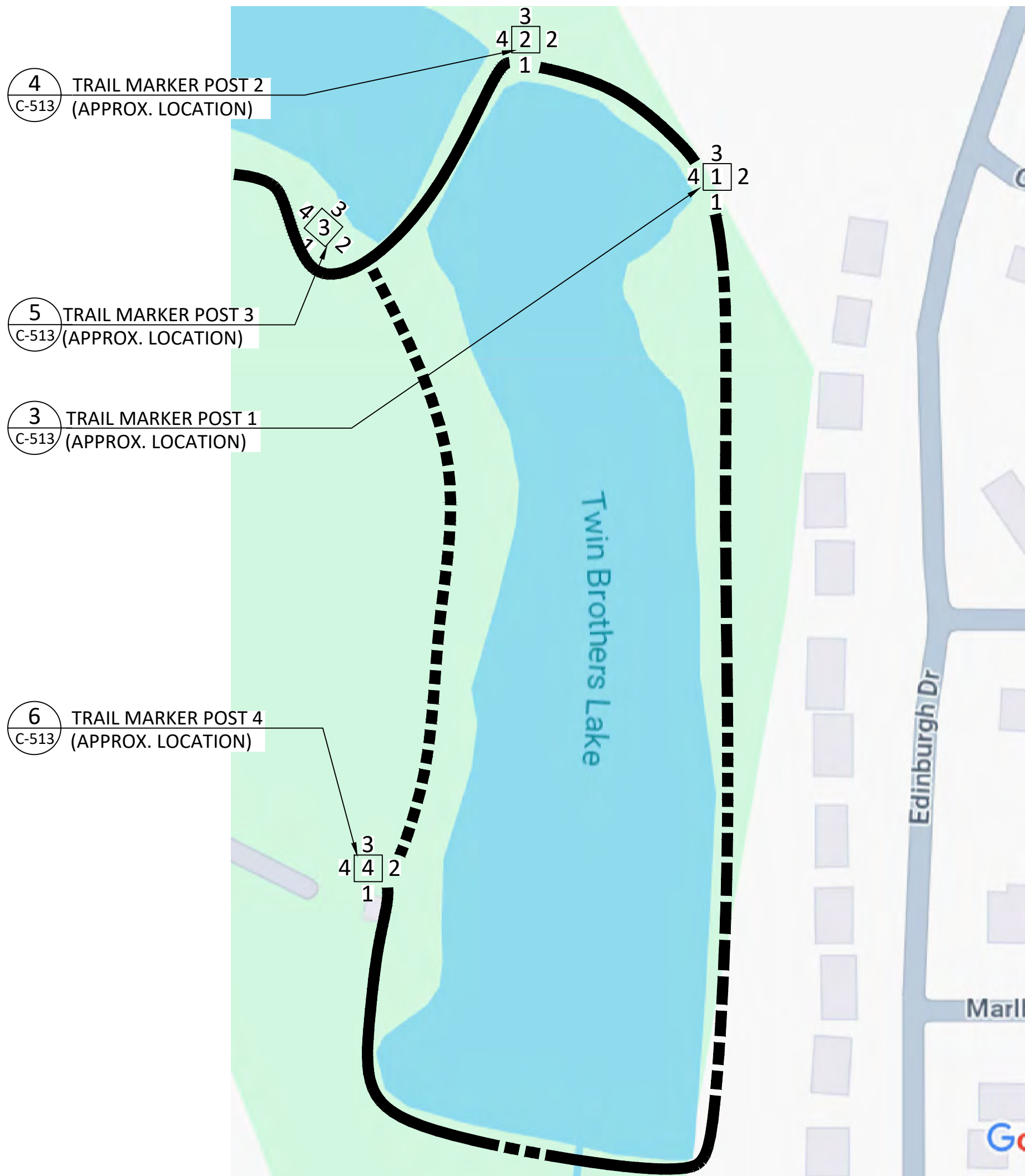
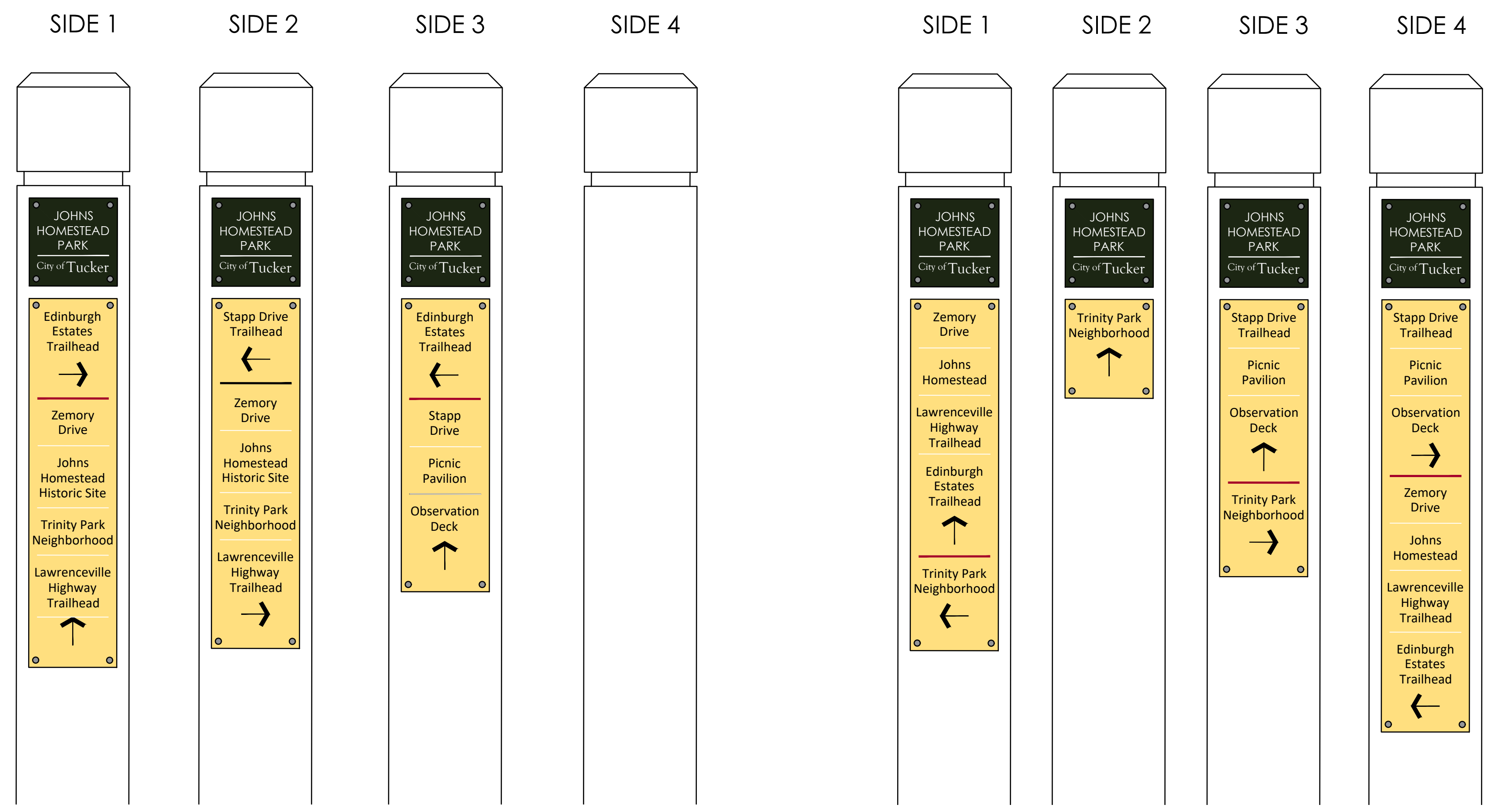
- TRAIL MARKER POSTS**
- MANUFACTURER: VACKER SIGN OR APPROVED EQUAL  
Vackersign.com  
Tel: (651) 487-3100  
Toll Free: (877) 487-3101  
948 Sherren Street W  
Roseville, MN 55113
  - SIZE: 4"x4" (TRUE) X 7' WITH TAPERED TOP, ROUTED DADO AND ROUTED POCKET TO ACCEPT A TUFF PANEL INSERT ON 3 OR FOUR POSTS SIDES.
  - POST SIGN MATERIALS AND COLOR: STRUCTURAL GRADE RECYCLED PLASTIC LUMBER POSTS WITH REINFORCED FIBERGLASS. COLOR: BI-COLOR, CEDAR ON THE OUTSIDE WITH CONTRASTING MEDIUM BROWN CORE.
  - QTY: POSTS: 4 EA WITH INSERT PANELS AS SHOWN, THIS DETAIL SHEET.
  - SIGN CONTENT: REFER TO INFORMATION THIS SHEET. PROJECT DESIGNER TO SUPPLY CONTRACTOR WITH PDF FILE OF SIGN INSERT LAYOUTS AND CONTENT.

**INSTALLATION NOTES**

- LOCATION AND ORIENTATION TO BE FIELD SITED WITH OWNER'S REPRESENTATIVE PRIOR TO INSTALLATION.

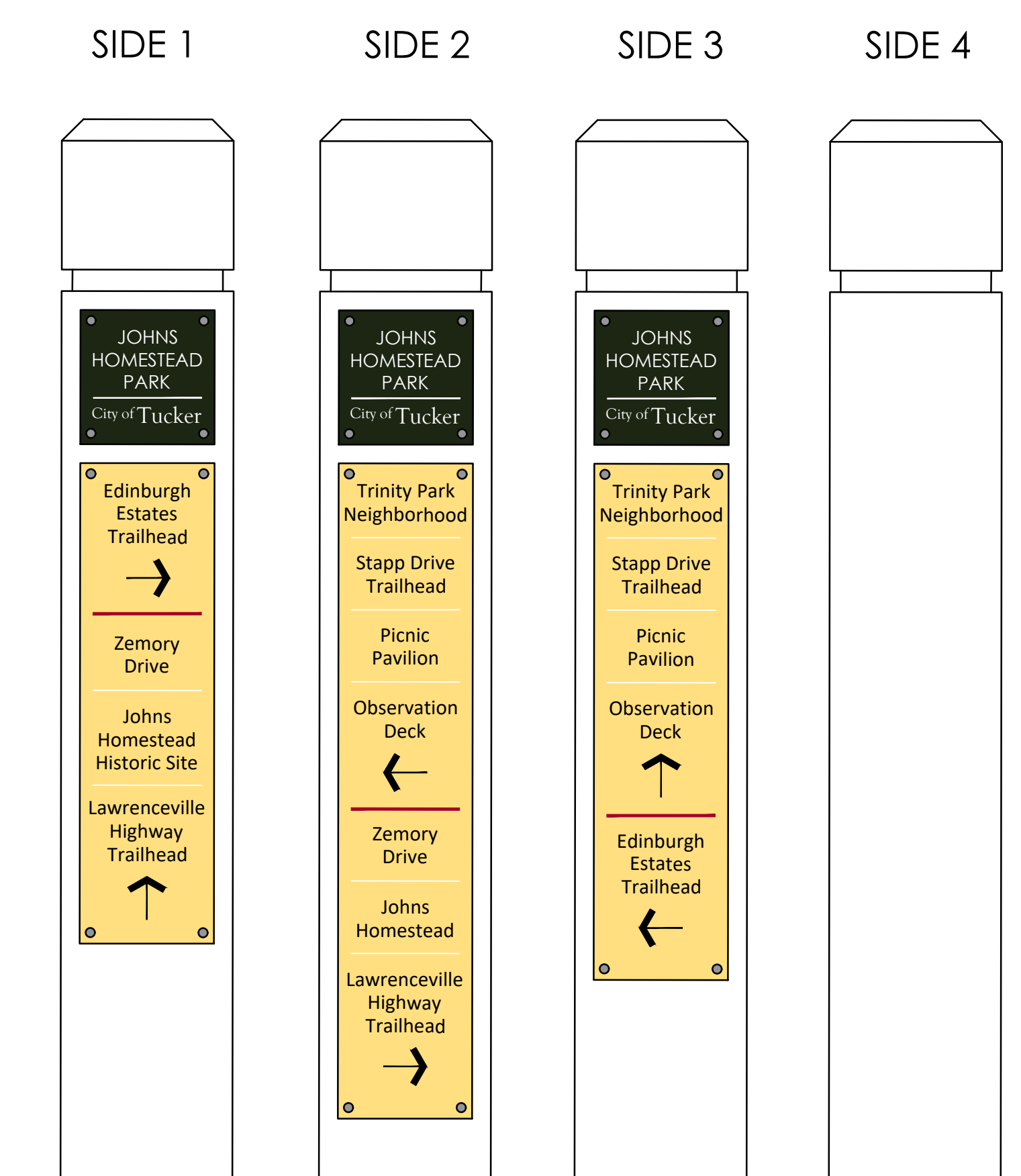


**1 TRAIL MARKER POST: NOTES AND INSTALLATION DETAIL**  
SCALE: 1"=1'-0"



**2 TRAIL MARKER POST LOCATION MAP**  
SCALE: NTS

**3 TRAIL MARKER POST 1 - EDINBURGH**  
SCALE: 4"=1'-0"

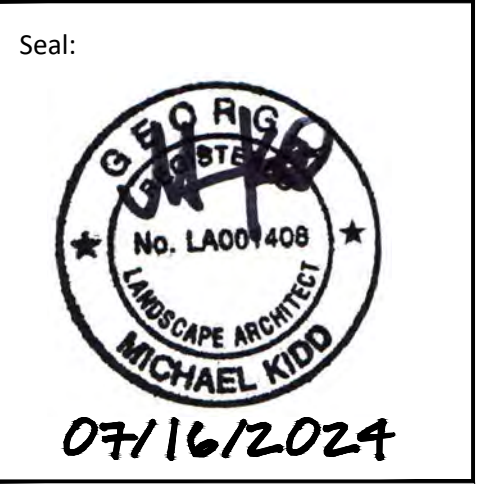


**4 TRAIL MARKER POST 2 - ZEMORY**  
SCALE: 4"=1'-0"

**5 TRAIL MARKER POST 3 - UPPER DAM**  
SCALE: 4"=1'-0"



**6 TRAIL MARKER POST 4 - WEST LOWER POND**  
SCALE: 4"=1'-0"



Date: 07/16/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MK

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Construction Details**

Sheet No:  
**C-513**

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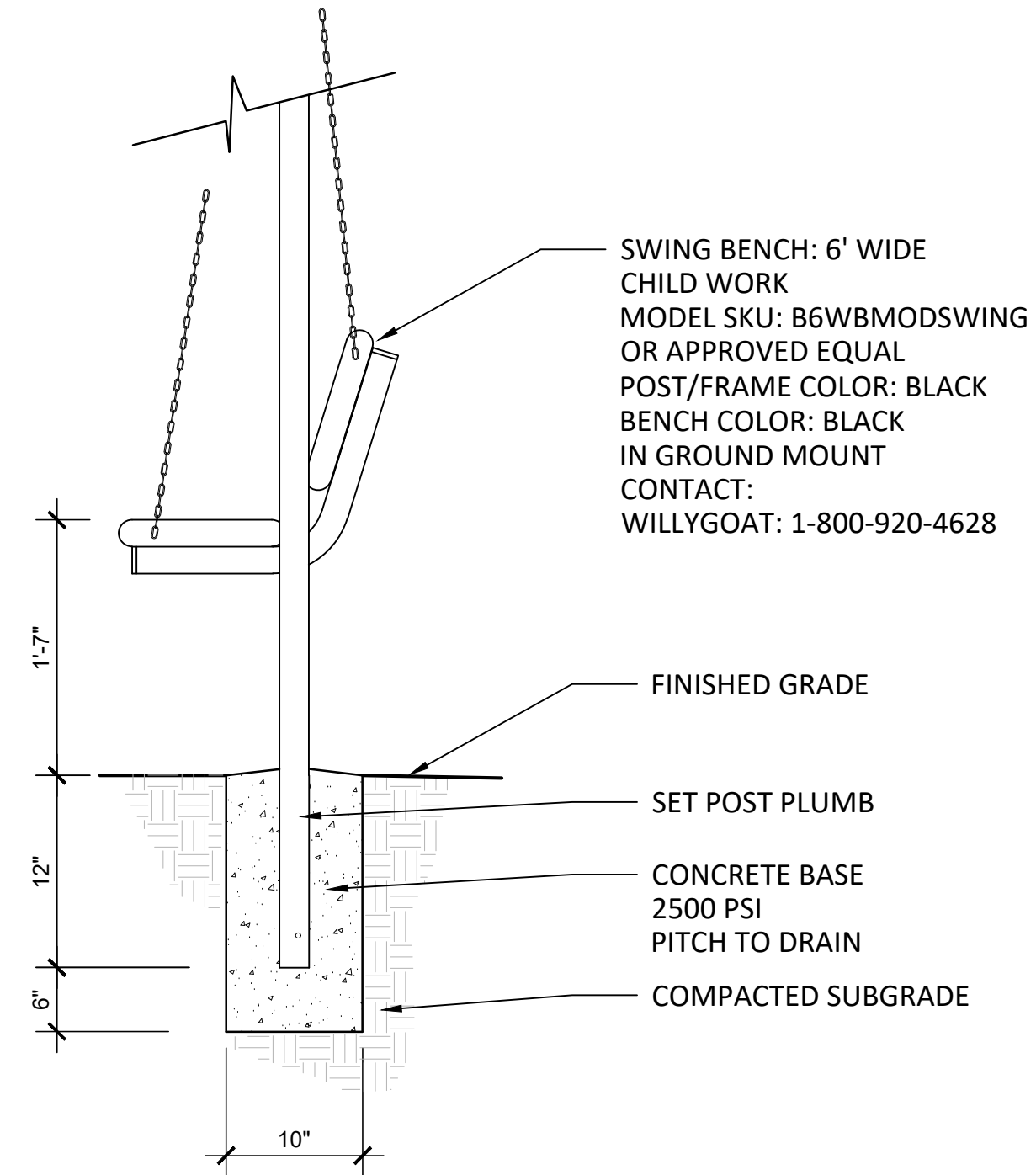


Date: 07/16/2024  
Project No: 2022-024  
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Checked By: MK

**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084



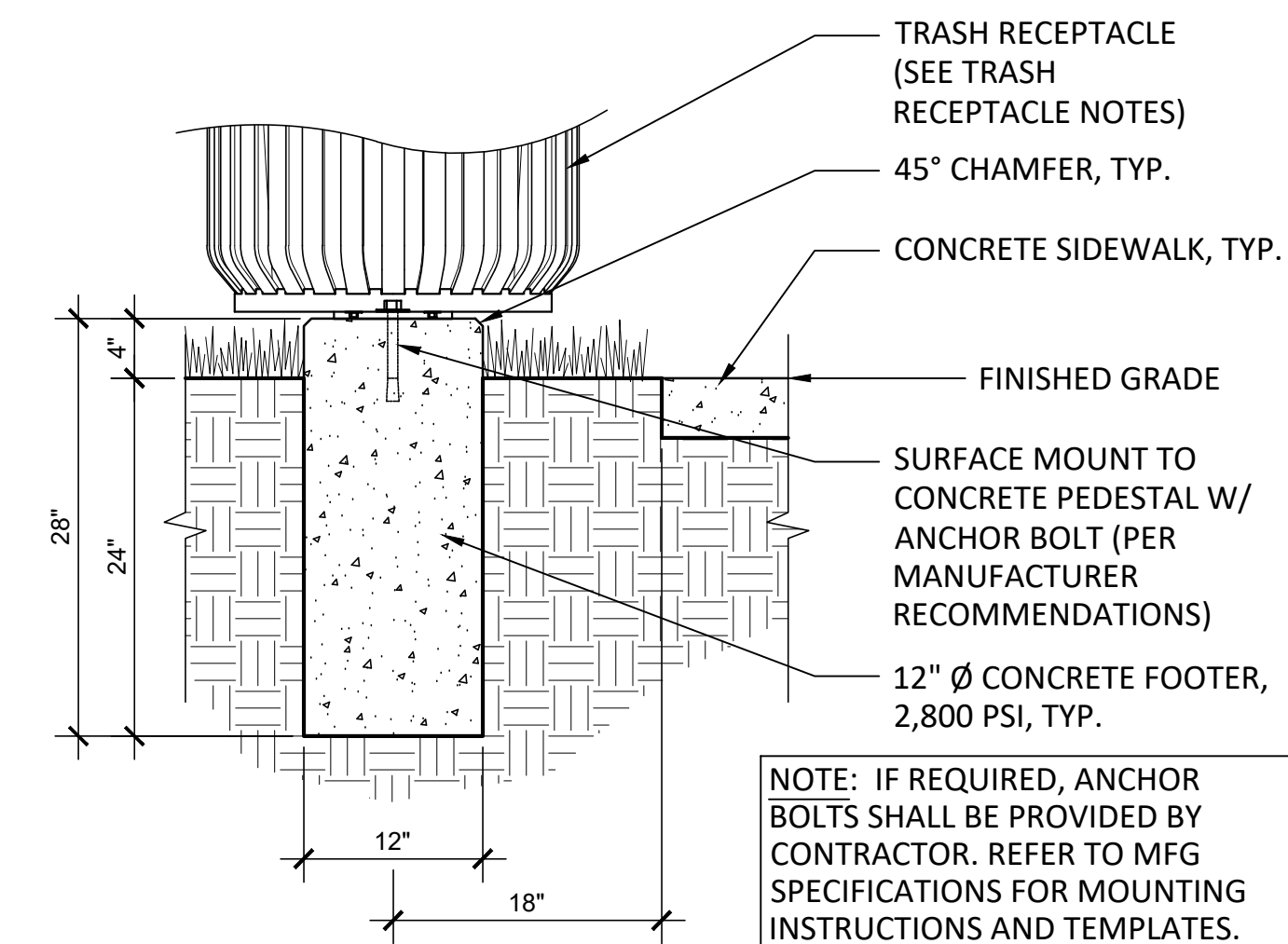
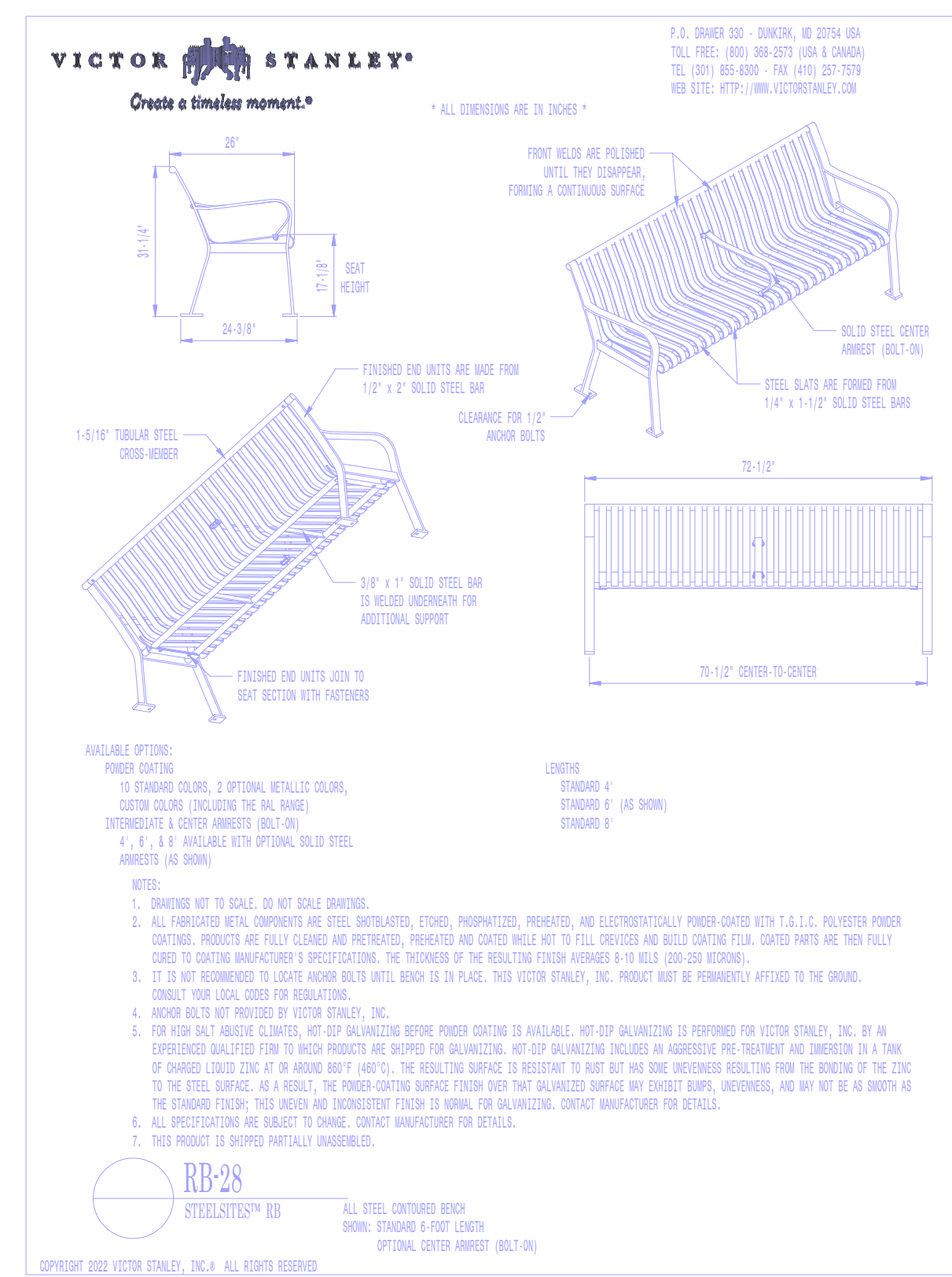
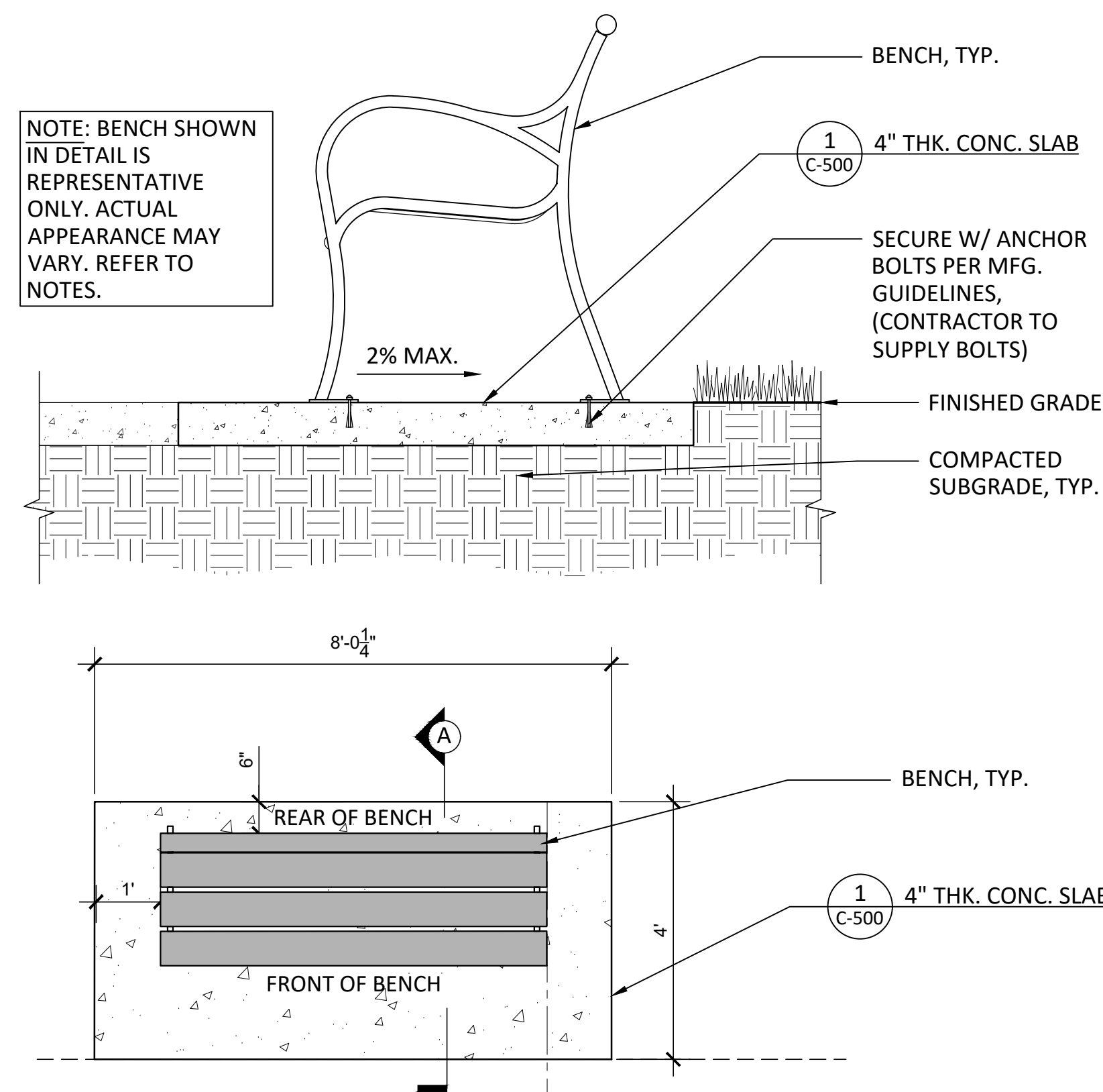
- DOG WASTE RECEPTACLE:**
- DOG WASTE RECEPTACLE BY DOGI POT MODEL 135-1003, AVAILABLE THROUGH THE PARK CATALOG (1-866-280-9894 THE PARKCATALOG.COM) OR APPROVED EQUAL.
  - CONTRACTOR TO MOUNT RECEPTACLE ON POST IN GROUND ACCORDING TO MANUFACTURER RECOMMENDATIONS.
  - CONTRACTOR SHALL SUBMIT PRODUCT INFORMATION FOR APPROVAL PRIOR TO INSTALLATION.



**1 DOG WASTE RECEPTACLE**  
SCALE: VARIES

**3 SWING BENCH**  
SCALE: VARIES

- BENCH NOTES:**
- BENCH SHALL BE 6' WIDE STEEL SITES SERIES RB28 (SURFACE MOUNT) BY VICTOR STANLEY (VICTORSTANLEY.COM 1-800-368-2573) OR APPROVED EQUAL.
  - SURFACE MOUNT BENCH TO SLAB W/CONCRETE ANCHOR PER MANUFACTURER'S RECOMMENDATIONS.
  - BIKE RACK SHALL BE BLACK IN COLOR POWDERCOATED FINISH.
  - BENCH PAD TO HAVE A CROSS SLOPE NOT TO EXCEED 2% IN ANY DIRECTION
  - CONTRACTOR SHALL SUBMIT PRODUCT INFORMATION FOR APPROVAL PRIOR TO INSTALLATION.



- TRASH RECEPTACLE NOTES:**
- TRASH RECEPTACLE TO BE MODEL # S-42 BY VICTOR STANLEY (victorst Stanley.com , 1-800-368-2573) OR APPROVED EQUAL.
  - TRASH RECEPTACLE SHALL HAVE RAIN BONNET LID.
  - TRASH RECEPTACLE & RAIN BONNET LID TO BE POWDERCOATED BLACK.
  - TRASH RECEPTACLE SHALL BE MOUNTED TO CONCRETE PEDESTAL PER MANUFACTURER RECOMMENDATIONS, AS SHOWN ABOVE.
  - CONTRACTOR SHALL SUBMIT PRODUCT INFORMATION FOR APPROVAL PRIOR TO INSTALLATION.

**2 BENCH AND BENCH PAD**  
SCALE: VARIES

**4 TRASH RECEPTACLE**  
SCALE: 1" = 1'-0"

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Construction Details**

Sheet No:  
**C-514**





Date: 03/18/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MS

**Johns Homestead Park & Recreation Dam Improvements**  
City of Tucker Parks and Recreation  
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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Construction Details**

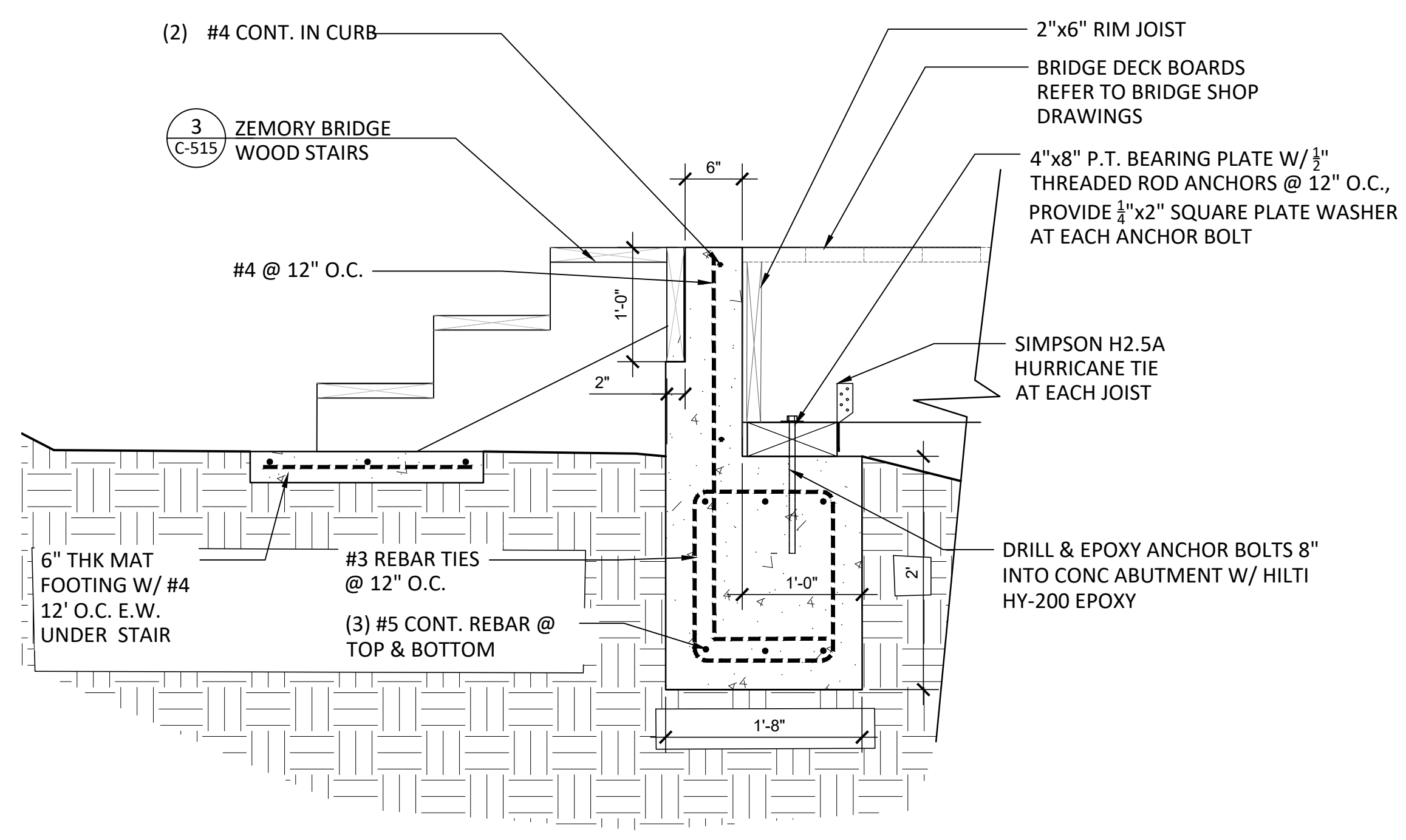
Sheet No:  
**C-515**

**BRIDGE NOTES**

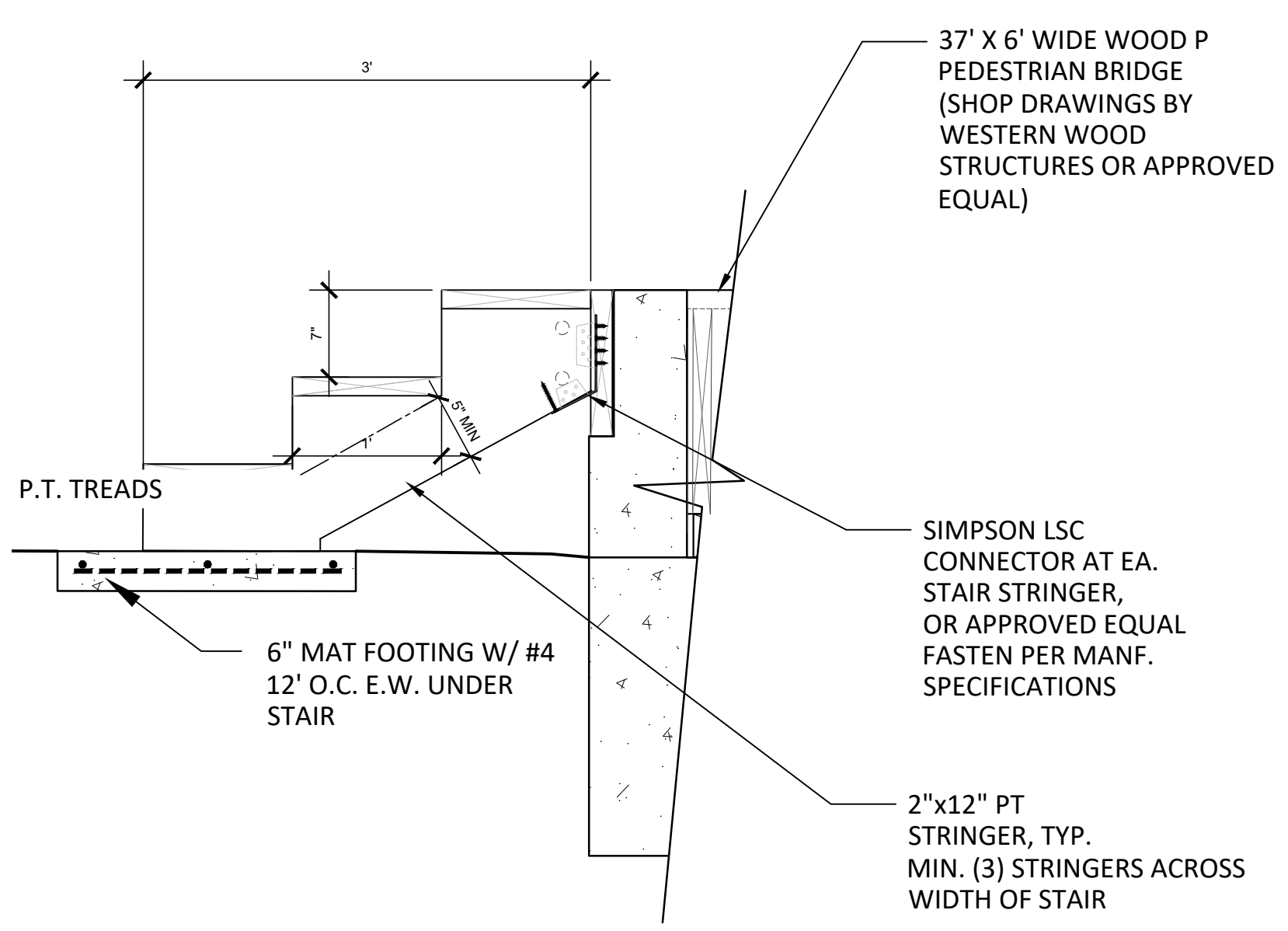
- BRIDGE TYPE: "SIDE GIRDER STYLE" PEDESTRIAN BRIDGE BY WESTERN WOOD STRUCTURES, OR EQUIVALENT
- BRIDGE TO BE 37' IN LENGTH.
- CONTACT JAMIE AGIDIUS, 503-692-6900, JAMIE@WWSI.COM
- WOODEN PEDESTRIAN RAILS: 42" HT, PER AASHTO, 6' CLEAR WIDTH BETWEEN RAILS.
- BEAMS: GLULAM, 100% WATERPROOF GLUE
- SAWN LUMBER: PRESSURE TREATED DOUGLAS FIR NO. 1
- GALVANIZED HARDWARE AND STEEL, AS REQUIRED
- PRESSURE TREATMENT: GLULAMS PRESSURE TREATED WITH .075 CUNAP AND HI-CLEAR II PER AWPA SPECIFICATIONS.
- SHOP DRAWINGS: FURNISHED BY CONTRACTOR, STAMPED BY REGISTERED P.E. IN THE STATE OF GEORGIA.
- ABUTMENT CONNECTION TO BE FINALIZED AFTER SHOP DRAWINGS FOR BRIDGE RECEIVED.

**CONCRETE ABUTMENT NOTES:**

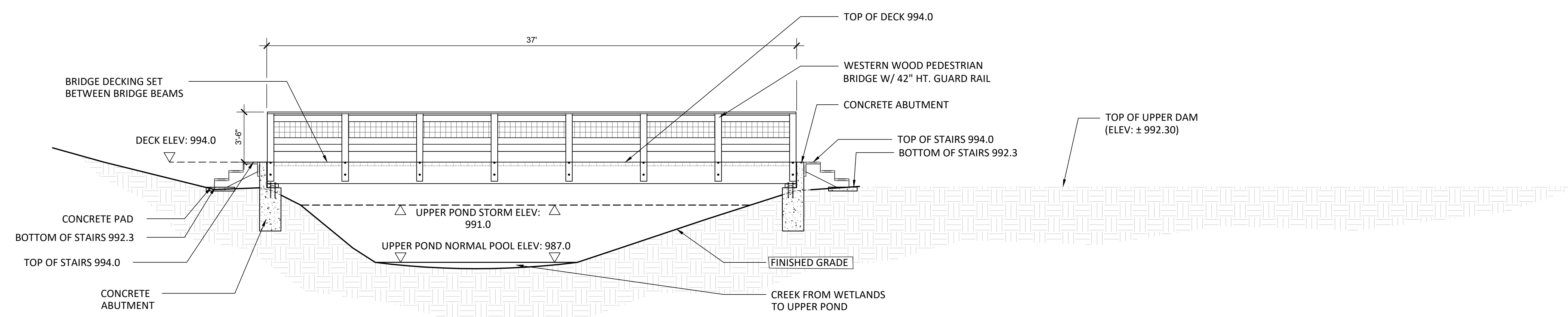
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- UNLESS OTHERWISE NOTED, MINIMUM CONCRETE COVER FOR REINF. SHALL BE AS FOLLOWS:
  - CONCRETE CAST AGAINST EARTH: 3"
  - FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 2"
- CHAMFER ALL EXPOSED EDGES OF CONCRETE 3/4".
- BAR REINFORCEMENT SHALL CONFORM TO ASTM-615, GRADE 60.
- PRIOR TO PLACING CONCRETE, ALL REINFORCING STEEL SHALL BE FREE OF RUST, SCALE, OR ANY FOREIGN MATERIAL.
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**1 ZEMORY BRIDGE ABUTMENT**  
SCALE: 1" = 1'-0"



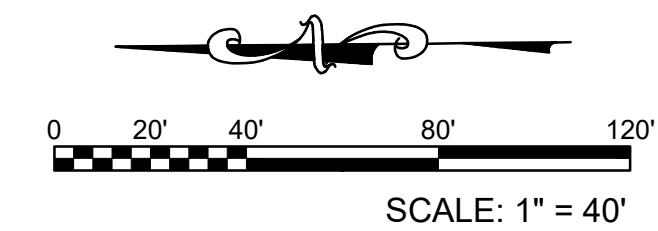
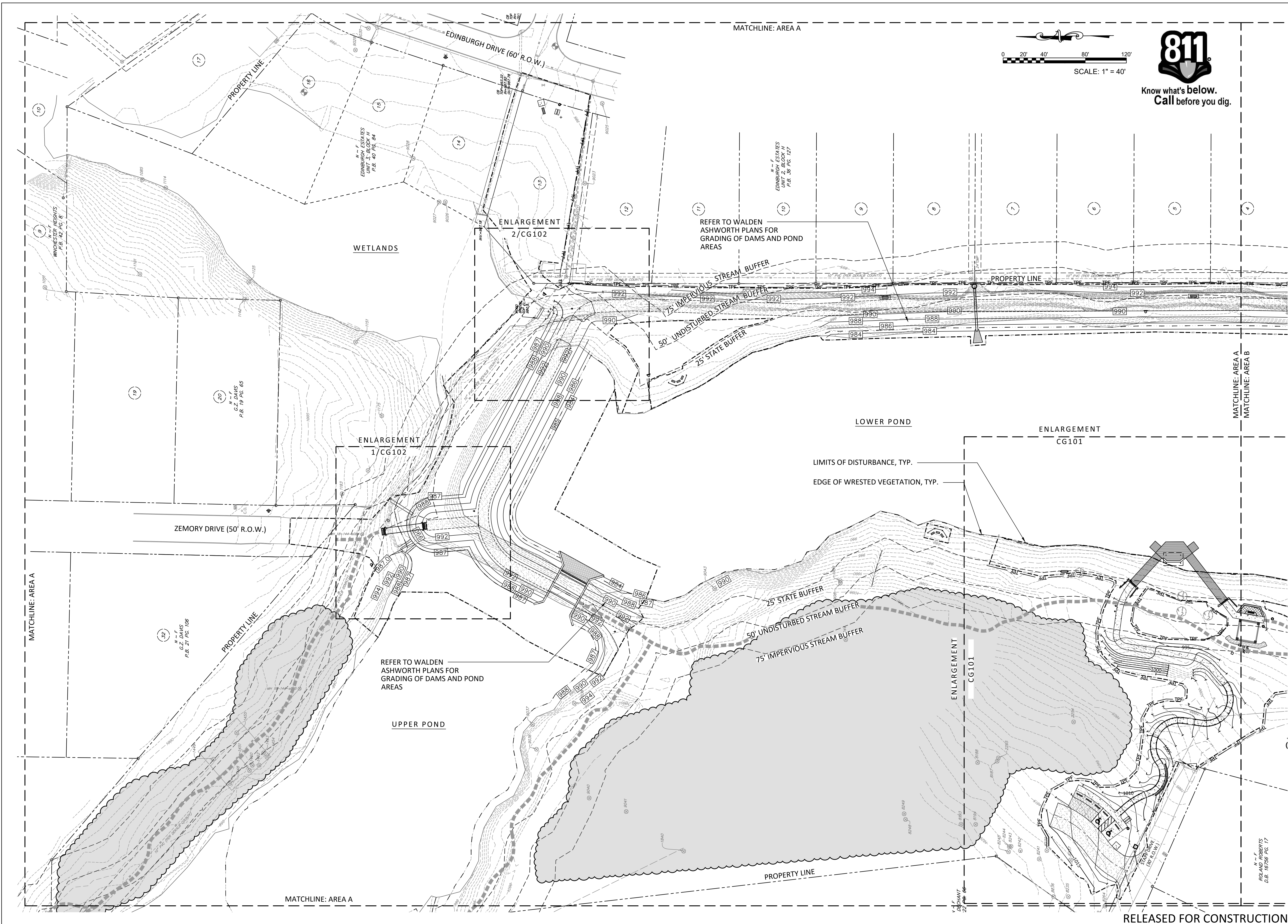
**3 ZEMORY BRIDGE WOOD STAIRS**  
SCALE: 1/2" = 1'-0"



**2 ZEMORY DRIVE PEDESTRIAN BRIDGE SECTION ELEVATION**  
SCALE: 1/4" = 1'-0"

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Date: 07/16/2024  
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**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

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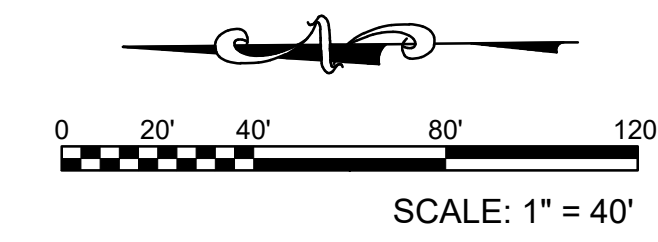
Sheet Title:  
**Grading & Drainage Plan:  
Area A**

Sheet No:  
**CG100**

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MATCHLINE: AREA B



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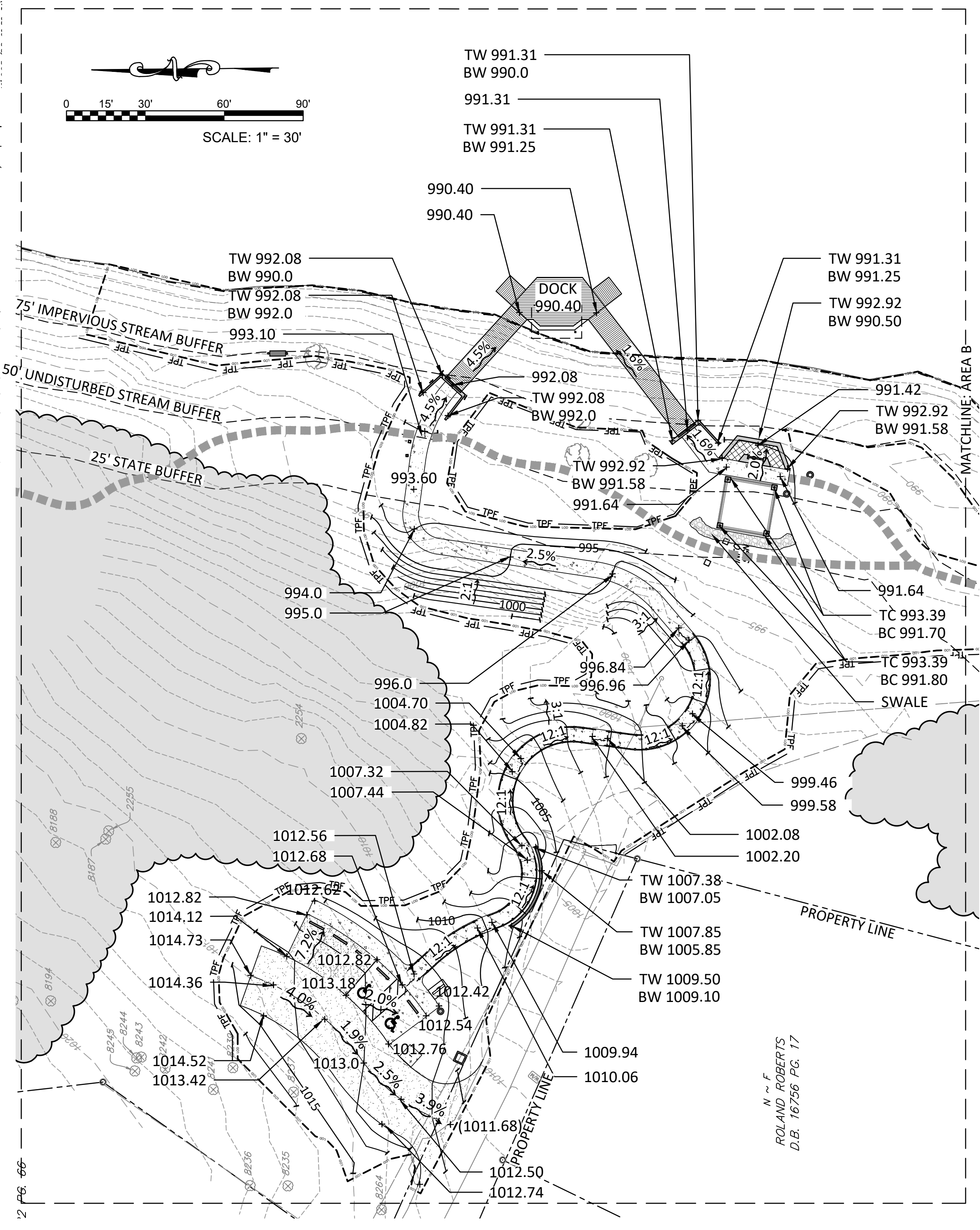
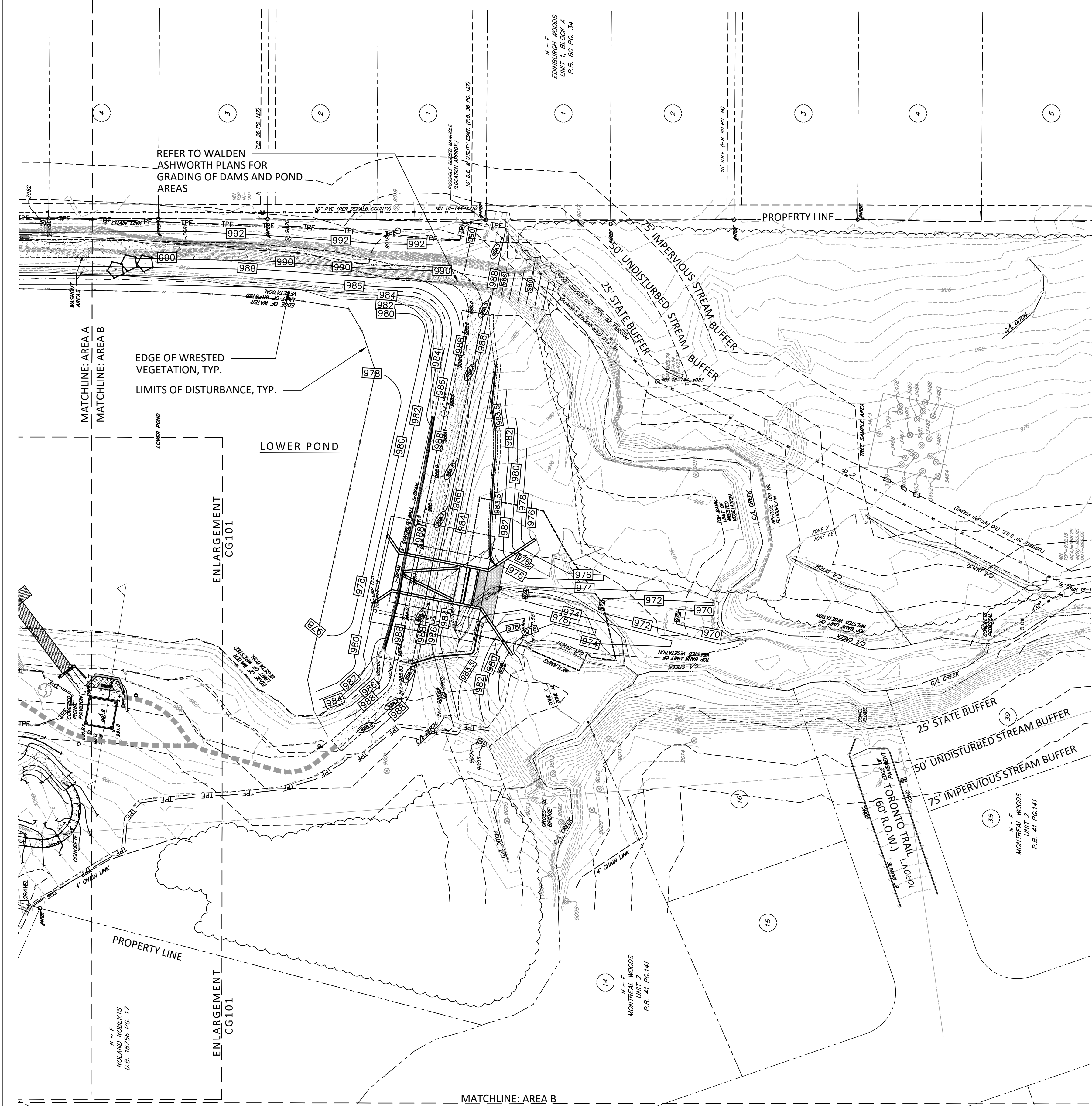
**Johns Homestead Park & Dam Improvements**  
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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Fine Grading Plan: Area B**

Sheet No:  
**CG101**



**1 GRADING ENLARGEMENT - STAPP DRIVE AREA**  
 SCALE: 1" = 30'-0"

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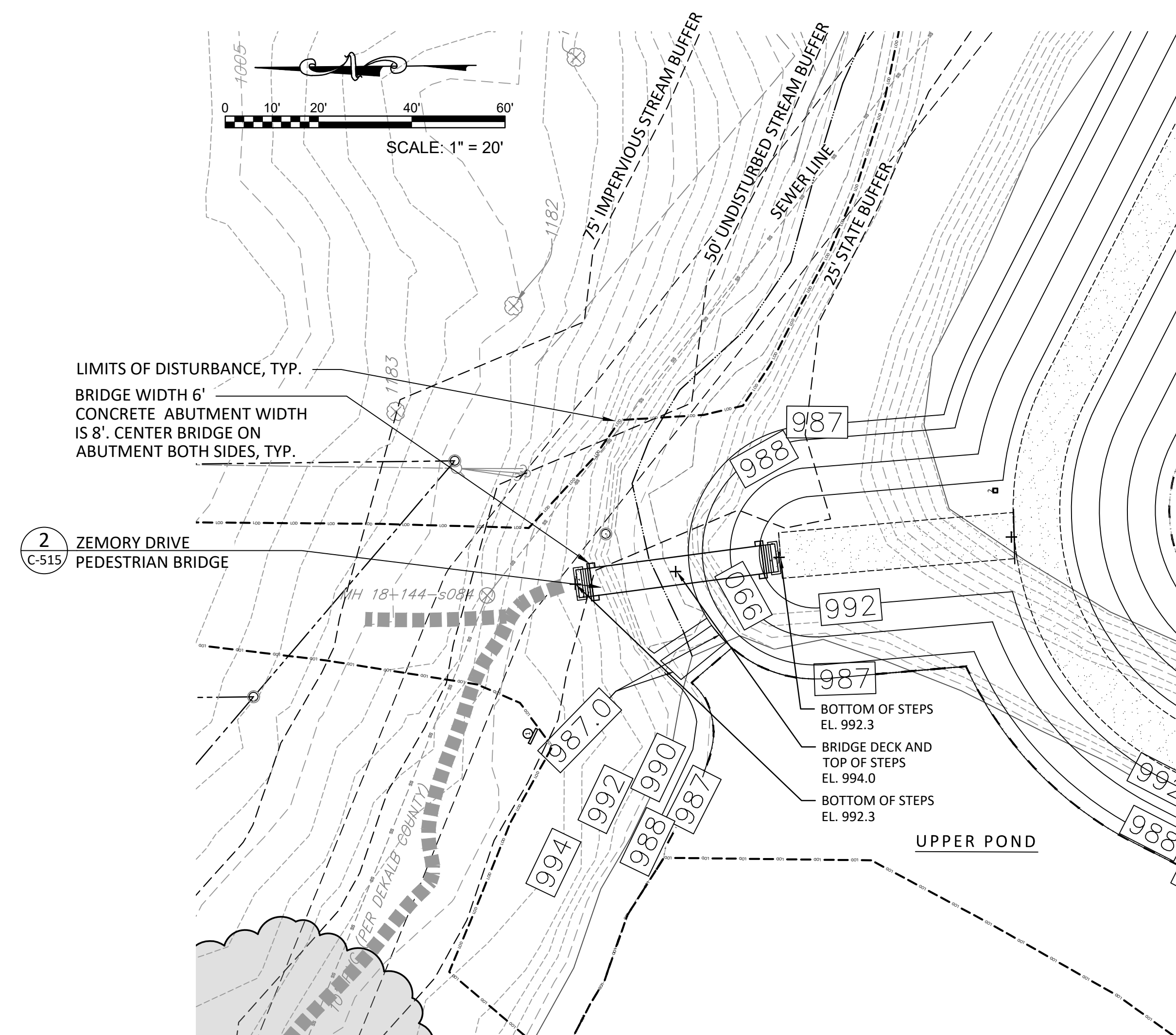
07/16/2024

Date: 07/16/2024

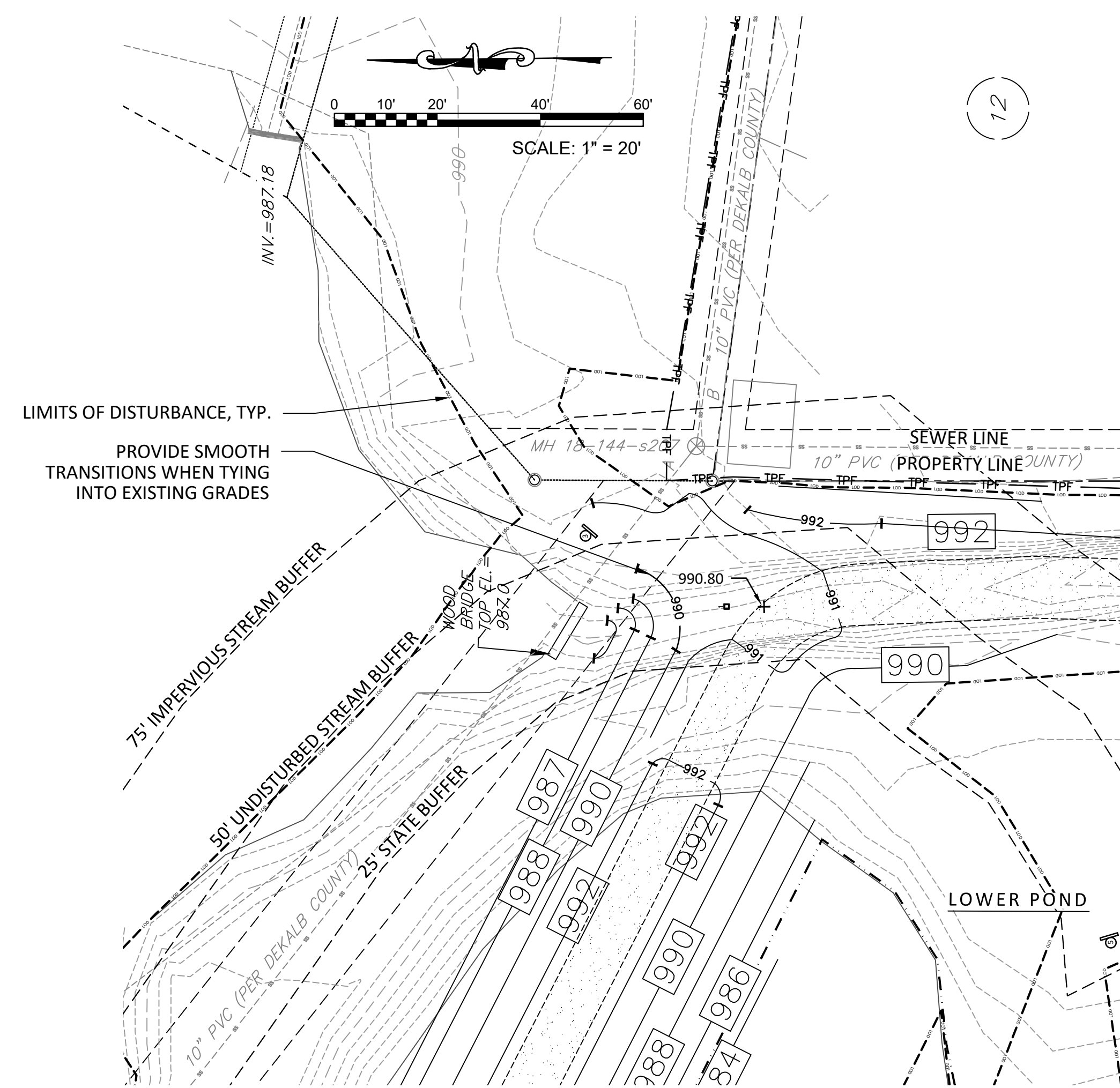
Project No: 2022-024

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1 GRADING ENLARGEMENT - ZEMORY BRIDGE  
SCALE: 1" = 20'-0"



2 GRADING ENLARGEMENT - EDINBURGH CONNECTION  
SCALE: 1" = 20'-0"

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City of Tucker Parks and Recreation  
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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
Fine Grading:  
Enlargements

Sheet No:  
CG102

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**BUFFER MITIGATION PLAN INDEX:**

- L-000 BUFFER MITIGATION PLANTING NOTES
- L-100 BUFFER MITIGATION KEY PLAN
- L-101 BUFFER MITIGATION PLANTING PLAN
- L-500 BUFFER MITIGATION PLANTING DETAILS

**GENERAL NOTES:**

1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, CODES AND REGULATIONS.
2. THE CONTRACTOR SHALL SECURE ALL NECESSARY PERMITS PRIOR TO BEGINNING ANY WORK.
3. CONTRACTOR MUST POSSESS CURRENT LICENSES AND/OR CERTIFICATIONS AS MAY BE REQUIRED BY LAW TO APPLY GENERAL OR RESTRICTED USE PESTICIDES AND CHEMICALS.
4. USER OF THESE DRAWINGS IS CAUTIONED THAT EXISTING UNDERGROUND UTILITIES AND FOUNDATIONS AS SHOWN ARE NOT GUARANTEED, NOR IS THERE ANY GUARANTEE THAT ALL EXISTING UTILITIES AND FOUNDATIONS, WHETHER ABANDONED OR FUNCTIONAL, ARE SHOWN ON DRAWINGS. IF UNDERGROUND FOUNDATION OR UTILITY WHICH IS NOT SHOWN ON DRAWINGS IS ENCOUNTERED OR DAMAGED BY CONSTRUCTION WORK, NOTIFY THE LANDSCAPE ARCHITECT IMMEDIATELY.
5. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CHECK AND VERIFY PROPOSED GRADES, DIMENSIONS, AND EXISTING CONDITIONS. REPORT ANY DISCREPANCIES TO THE LANDSCAPE ARCHITECT FOR DIRECTION BEFORE PROCEEDING WITH WORK. WORK STARTED WITHOUT DIRECTION FROM THE LANDSCAPE ARCHITECT WILL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND WILL BE CORRECTED IF NECESSARY AT HIS/HER EXPENSE.
6. THE CONTRACTOR SHALL CONTACT THE UTILITY PROTECTION SERVICE (811) TO LOCATE ALL ABOVE AND BELOW GROUND UTILITIES PRIOR TO BEGINNING WORK.
7. THE CONTRACTOR SHALL PROVIDE TRAFFIC CONTROL MEASURES AS NEEDED TO ENSURE THE SAFETY OF VEHICULAR AND PEDESTRIAN TRAFFIC WITHIN CONSTRUCTION ZONES.
8. THE CONTRACTOR SHALL MAINTAIN A CLEAN AND COURTEOUS WORK SITE FOR THE DURATION OF THE PROJECT. TRASH AND DEBRIS SHALL BE PICKED UP AND PROPERLY DISPOSED OF EACH DAY. VEHICLES AND EQUIPMENT SHALL BE CLEANED AND MAINTAINED REGULARLY SO AS NOT TO DRIP ANY FLUIDS OR TRACK AND SEDIMENT WITHIN THE PROPERTY.
9. THE CONTRACTOR SHALL ENSURE THAT NO SEDIMENT LEAVES THE WORK SITE DURING CONSTRUCTION. ANY SEDIMENT THAT ACCUMULATES ALONG THE CURB OR ON THE ROADWAY SHALL BE SWEEP UP AT THE END OF EACH WORK DAY AND PRIOR TO EACH RAIN EVENT.
10. ALL CONSTRUCTION DEBRIS SHALL BE HAULED OFF SITE AND PROPERLY DISPOSED OF IN ACCORDANCE WITH APPLICABLE LAWS.
11. ANY PROPERTY THAT IS DAMAGED AS A RESULT OF CONSTRUCTION ACTIVITIES FROM THIS PROJECT SHALL BE REPLACED OR REPAIRED TO ITS ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE.
12. THE CONTRACTOR SHALL LEAVE THE PROJECT SITE IN A PRISTINE CONDITION UPON COMPLETION OF THE WORK. REMOVE AND PROPERLY DISPOSE OF ALL TRASH, DEBRIS, EXCESS SOIL, ETC. CLEAN ALL VISIBLE SURFACES SO THEY ARE FREE FROM DIRT, SMUDGES, STAINS, ETC.

**TREE PLANTING NOTES:**

1. ALL TREES SHALL BE TEN (10) FEET MINIMUM FROM GAS LINES AND SANITARY SEWER, AND FIVE (5) FEET MINIMUM FROM FIRE HYDRANTS AND ALL OTHER UNDERGROUND UTILITIES.
2. TREES SHALL BE SET PLUMB AND LEVEL.
3. CAREFULLY LIFT AND SET TREES BY THE ROOT BALL ONLY. DO NOT LIFT USING THE TRUNK OR STEM. IF TREES ARE TOO LOW OR NOT PLUMB AFTER SETTING IN THE HOLE, RE-SET BY ADJUSTING THE ROOT BALL AS NEEDED.

**TREE STAKING NOTES:**

1. ONLY STAKE TREES ON SLOPES 5:1 OR GREATER.

**MULCHING NOTES:**

1. PROVIDE 100% PERMANENT MULCH COVER OF ALL EXPOSED SOIL MULCH TO BE MINIMUM OF 3" THICK.
2. MULCH TO BE AGED SHREDDED HARDWOOD.
3. MULCH SHALL NOT BE PLACED IN AREAS WITHIN THE 25' BUFFER THAT ARE SUBJECT TO FLOODING.

**LANDSCAPE NOTES:**

1. LANDSCAPE PLANS ARE FOR THE LOCATION AND IDENTIFICATION OF PLANT MATERIAL ONLY. NO OTHER WORK IS TO BE PERFORMED BASED ON THESE PLANS.
  2. QUANTITIES ON THE PLANT SCHEDULE ARE PROVIDED FOR CONVENIENCE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HIS/HER OWN QUANTITY CALCULATIONS. IN THE EVENT OF A DISCREPANCY BETWEEN THE LANDSCAPE PLANS AND THE PLANT SCHEDULE, THE LANDSCAPE PLAN WILL TAKE PRECEDENCE. THE CONTRACTOR SHALL INFORM THE LANDSCAPE ARCHITECT IMMEDIATELY UPON DISCOVERING ANY QUANTITY DISCREPANCIES.
  3. THE CONTRACTOR SHALL NOT CHANGE OR SUBSTITUTE PLANT VARIETIES OR SPECIES WITHOUT PRIOR WRITTEN APPROVAL FROM THE LANDSCAPE ARCHITECT. PLANT MATERIAL SHALL BE PLACED AS SHOWN ON THE LANDSCAPE PLANS.
  4. CONTRACTOR SHALL ENSURE POSITIVE DRAINAGE OF ALL PLANTING HOLES AND PLANT BEDS PRIOR TO INSTALLATION.
  5. NEW SHRUB AND GROUND COVER PLANTING SHALL BE A MINIMUM OF 36" AWAY FROM EXISTING TREES.
  6. QUALITY OF PLANT MATERIAL: ALL PLANTS SHALL CONFORM TO THE CURRENT VERSION OF THE AMERICAN STANDARD FOR NURSERY STOCK (ANSI Z60.1). PLANT MATERIAL SHALL BE FREE OF INSECTS, DISEASE AND/OR INJURY, AND SHALL HAVE A HEALTHY ROOT SYSTEM WITH NO CIRCLING OR KINKED ROOTS. CONTAINER PLANTS SHALL NOT BE ROOT BOUND. TREES SHALL HAVE STRAIGHT TRUNKS, A STRONG DOMINANT CENTRAL LEADER (AS REQUIRED BY SPECIES), DENSE CANOPIES AND STRONG BRANCHING WITH GOOD CROTCH ANGLES.
  7. INSPECTION AND APPROVAL OF PLANT MATERIAL: ALL PLANT MATERIAL SHALL BE INSPECTED AND APPROVED BY THE OWNER'S REPRESENTATIVE UPON DELIVERY TO THE SITE AND PRIOR TO INSTALLATION. CONTRACTOR SHALL PROVIDE AT LEAST ONE WEEK NOTICE PRIOR TO PLANT DELIVERY.
  8. PLANT MATERIAL SHALL BE SUFFICIENTLY WATERED TO WET THE ENTIRE ROOT BALL WITHIN TWO HOURS OF PLANTING.
  9. MAINTENANCE: CONTRACTOR SHALL MAINTAIN ALL PLANT MATERIAL FROM THE TIME IT IS INSTALLED UNTIL FINAL ACCEPTANCE OR WHEN THE OWNER TAKES OVER MAINTENANCE, WHICHEVER OCCURS FIRST. MAINTENANCE SHALL INCLUDE BUT NOT BE LIMITED TO MOWING, EDGING, WEEDING, WATERING, PRUNING, FERTILIZING, ETC.
  10. WARRANTY: CONTRACTOR SHALL PROVIDE A WARRANTY ON ALL PLANT MATERIAL AND LABOR FOR A PERIOD OF TWO YEARS OR PER JURISDICTIONAL REQUIREMENTS, WHICHEVER IS LONGER. WARRANTY PERIOD SHALL BEGIN UPON FINAL COMPLETION OR WHEN THE OWNER TAKES OVER MAINTENANCE, WHICHEVER OCCURS FIRST.
- THE CONTRACTOR SHALL MAKE PERIODIC INSPECTIONS OF THE PROJECT DURING THE WARRANTY PERIOD TO ENSURE THAT THE ESTABLISHMENT RATE OF GROWTH IS ADEQUATE. ANY METHODS OR PRODUCTS DEEMED NOT NORMAL OR DETRIMENTAL TO GOOD PLANT GROWTH SHALL BE REPORTED TO THE OWNER IN WRITING. FAILURE TO INSPECT AND REPORT WILL BE INTERPRETED AS APPROVAL, AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL REPLACEMENTS.

**PLANT BED PREPARATION NOTES:**

1. FLAG OR STAKE PROPOSED TREE LOCATIONS AND MARK ALL BEDLINES WITH ORANGE OR WHITE PAINT. NOTIFY THE OWNER'S REPRESENTATIVE FOR REVIEW AND FIELD ADJUSTMENTS, AS NEEDED.
2. ADD ONE (1) INCH OF TOP SOIL TO THE SURFACE OF EACH BED. (SEE "LANDSCAPE NOTES" FOR MORE INFORMATION.)
3. ADD ONE (1) INCH OF AGED COMPOST TO THE SURFACE OF EACH BED. COMPOST MUST BE AGED APPROXIMATELY THREE (3) TO SIX (6) MONTHS. SOURCES MAY INCLUDE: CHICKEN MANURE, HORSE MANURE, ZOO ATLANTA MANURE, OR APPROVED EQUAL. SUBMIT PRODUCT INFORMATION TO OWNER'S REPRESENTATIVE FOR APPROVAL.
4. ONCE TREES ARE INSTALLED, LAYOUT REMAINING PLANT MATERIAL IN EACH BED FOR REVIEW AND APPROVAL BY OWNER'S REPRESENTATIVE.
5. INSTALL REMAINING PLANT MATERIAL PER PLANTING DETAILS.
6. REMOVE ALL PLANT TAGS AFTER APPROVAL OF PLANT INSTALLATION BY LANDSCAPE ARCHITECT.
7. MULCH ALL PLANT BEDS AND TREE RINGS WITH FRESH, CLEAN PINESTRAW TO A MINIMUM DEPTH OF THREE (3) INCHES (UNLESS OTHERWISE NOTED ON PLANS). DO NOT PILE MULCH AROUND THE BASE OF PLANTS OR TREE TRUNKS. ALL MULCH EDGES SHALL BE NEATLY TUCKED. ALL STRING AND/OR BAILING WIRE SHALL BE REMOVED. "DUST" SHRUBS AND GROUND COVER AFTER MULCHING TO REMOVE LOOSE PINESTRAW FROM THE PLANTS.
8. WATER PLANT BEDS IMMEDIATELY AFTER INSTALLING MULCH. ALL PLANTS SHALL BE WATERED THE SAME DAY THEY ARE INSTALLED. APPLY AT LEAST ONE (1) INCH OF WATER TO EACH BED.



Date: 07/16/2024  
 Project No: 2022-024  
 Drawn By: PS  
 Checked By: MK

**Johns Homestead Park & Dam Improvements**  
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Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Buffer Mitigation Planting Notes**

Sheet No:  
**L-000**

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Tucker, GA 30084

Revisions:

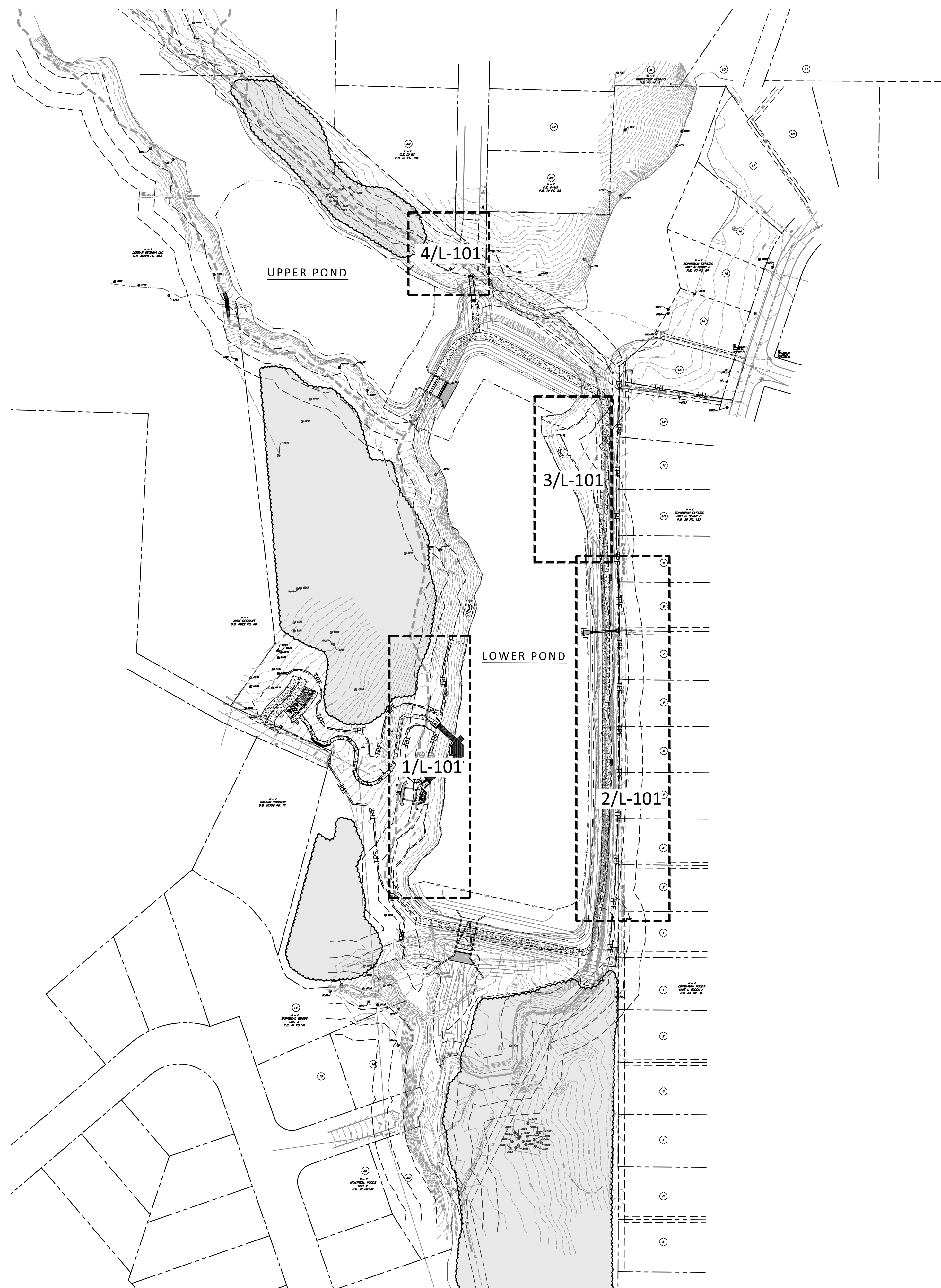
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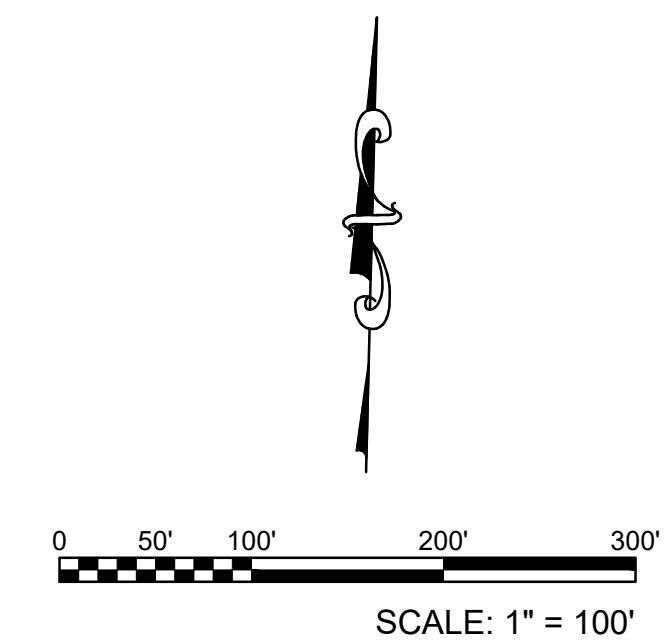
**Buffer Mitigation  
Key Plan**

Sheet No:

**L-100**



Know what's below.  
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07/16/2024

Date: 07/16/2024

Project No: 2022-024

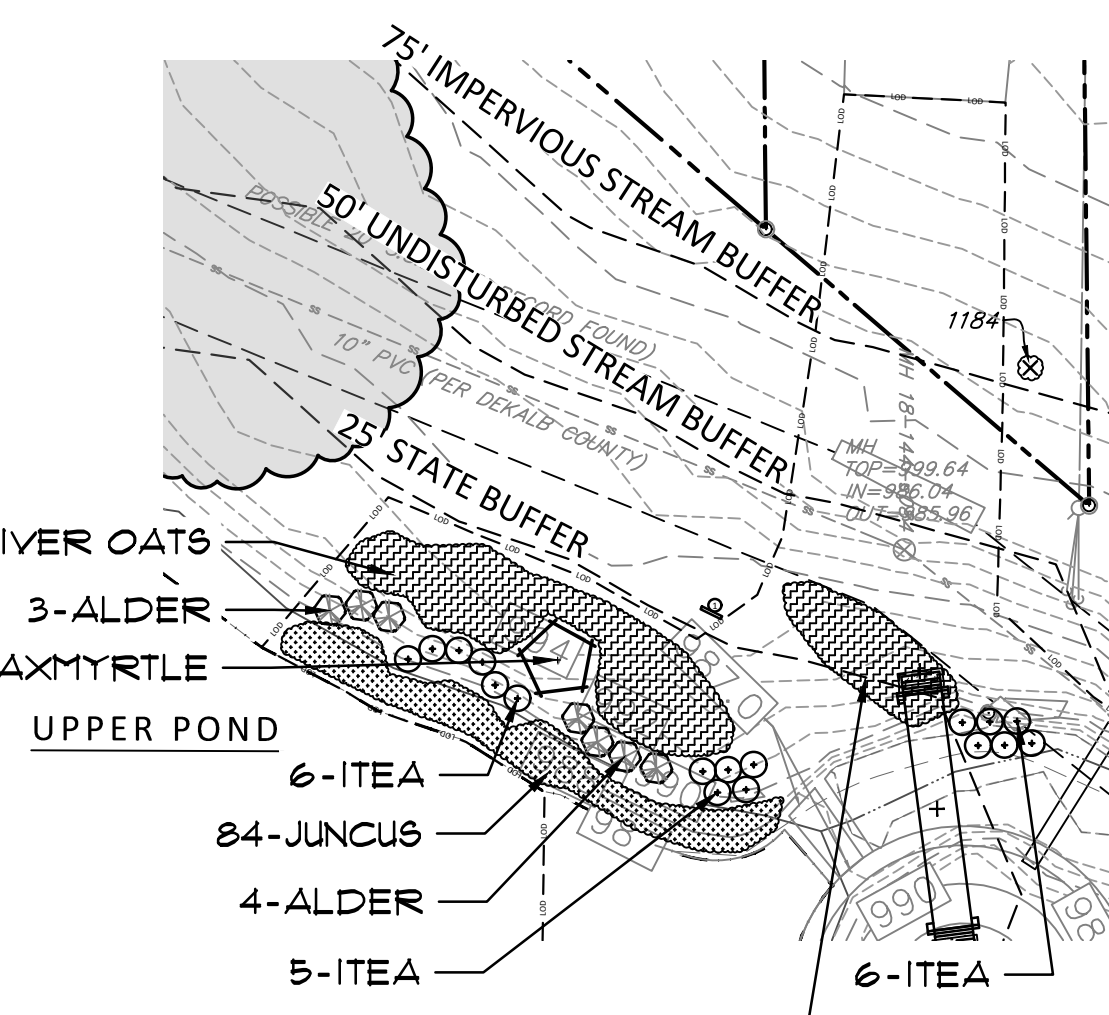
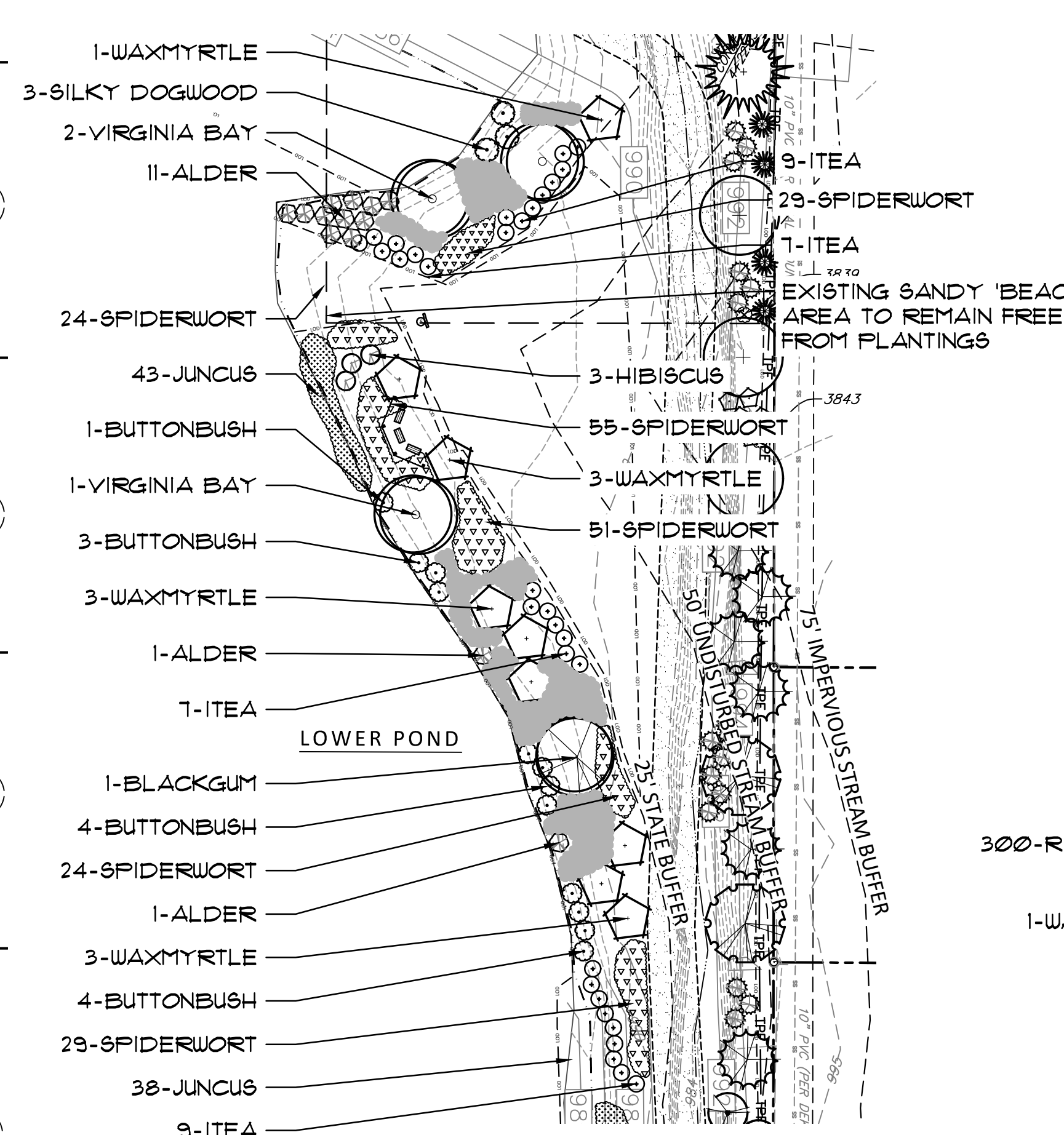
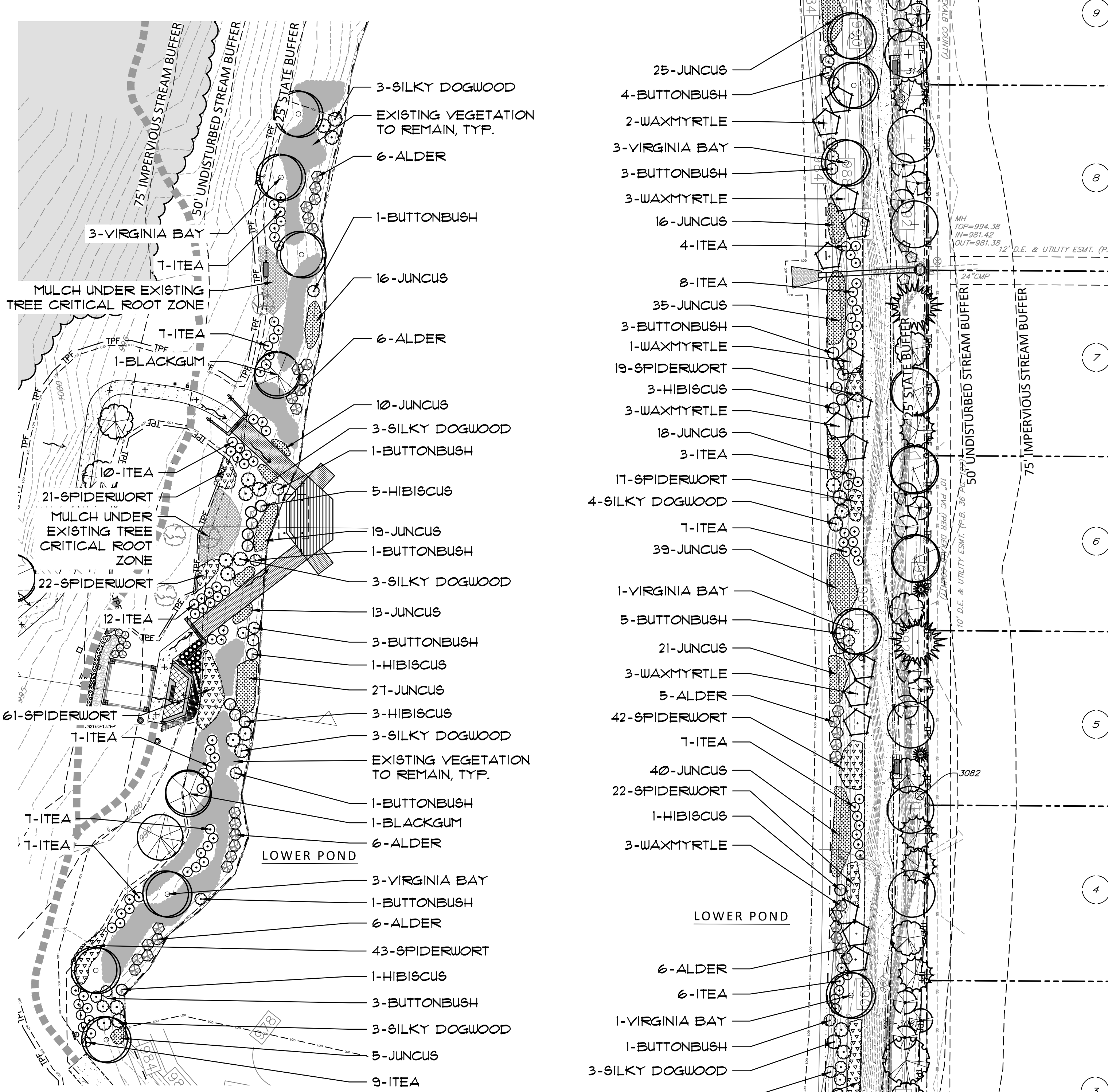
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Checked By: MK

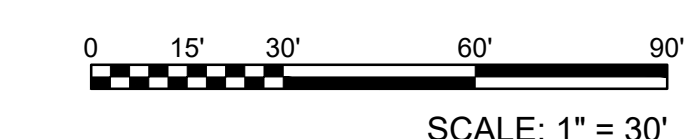
**Buffer Mitigation Plant Schedule**

BOTANICAL NAME	COMMON NAME	Quantity	Size	Spacing	Remarks
<b>CANOPY TREES</b>					
<i>Nyssa sylvatica</i>	Blackgum	3	3/4"-1" Cal.	Per Plan	1, 3, 4, 5, 6, 10, 11
<b>UNDERSTORY TREES</b>					
<i>Magnolia virginiana</i>	Virginia Bay Magnolia	18	15 Gal.	Per Plan	2, 3, 4, 6, 8, 9, 12
<b>SHRUBS</b>					
<i>Itea virginica</i>	Virginia Sweetpire	149	3 Gal.	4' O.C.	5, 6, 10, 12
<i>Cornus amomum</i>	Silky dogwood	25	5 Gal.	6' O.C.	5, 6, 10, 12
<i>Myrica cerifera</i>	Wax Myrtle	25	5 Gal.	Per Plan	5, 6, 10, 12
<b>GRASSES, PERENNIALS, GROUNDCOVERS</b>					
<i>Tradescantia ohioensis</i>	Ohio Spiderwort	540	1 Gal.	24" O.C.	5, 6, 10, 12
<i>Chasmanthium latifolium</i>	River oats	434	4" Pot	18" O.C.	5, 6, 10, 12
<i>Alnus serrulata</i>	Smooth Alder	60	1 Gal.	5' O.C.	5, 6, 10, 12
<i>Hibiscus coccineus</i>	Scarlet Rosemarrow	21	1 Gal.	5' O.C.	5, 6, 10, 12
<i>Cephalanthus occidentalis</i>	Buttongum	36	3 Gal.	5' O.C.	5, 6, 10, 12
<i>Juncus effusus</i>	Common Rush	496	1 Gal.	30" O.C.	5, 6, 10, 12
<b>MULCH</b>					
Hardwood Much		590	Sq. Ft.		

- REMARKS:
1. Single Trunk
  2. Multi-Trunk, 3-5 canes, 1" min./cane
  3. Straight Trunk
  4. Central Leader
  5. Natural Branching
  6. Full Canopy / Head
  7. Full to Ground
  8. Tree Form
  9. Specimen
  10. Matched Set
  11. Balled and Burlapped (B&B)
  12. Container



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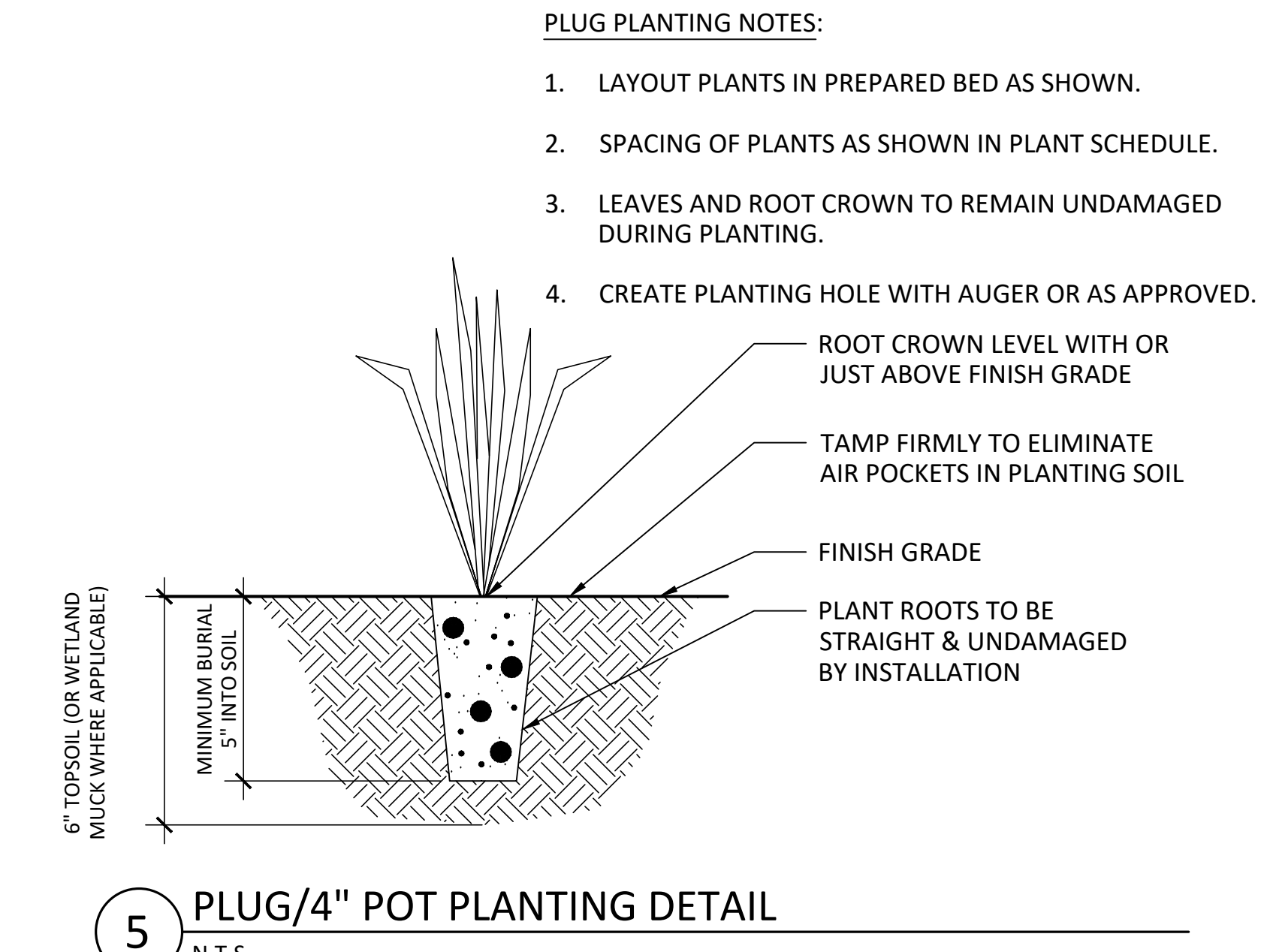
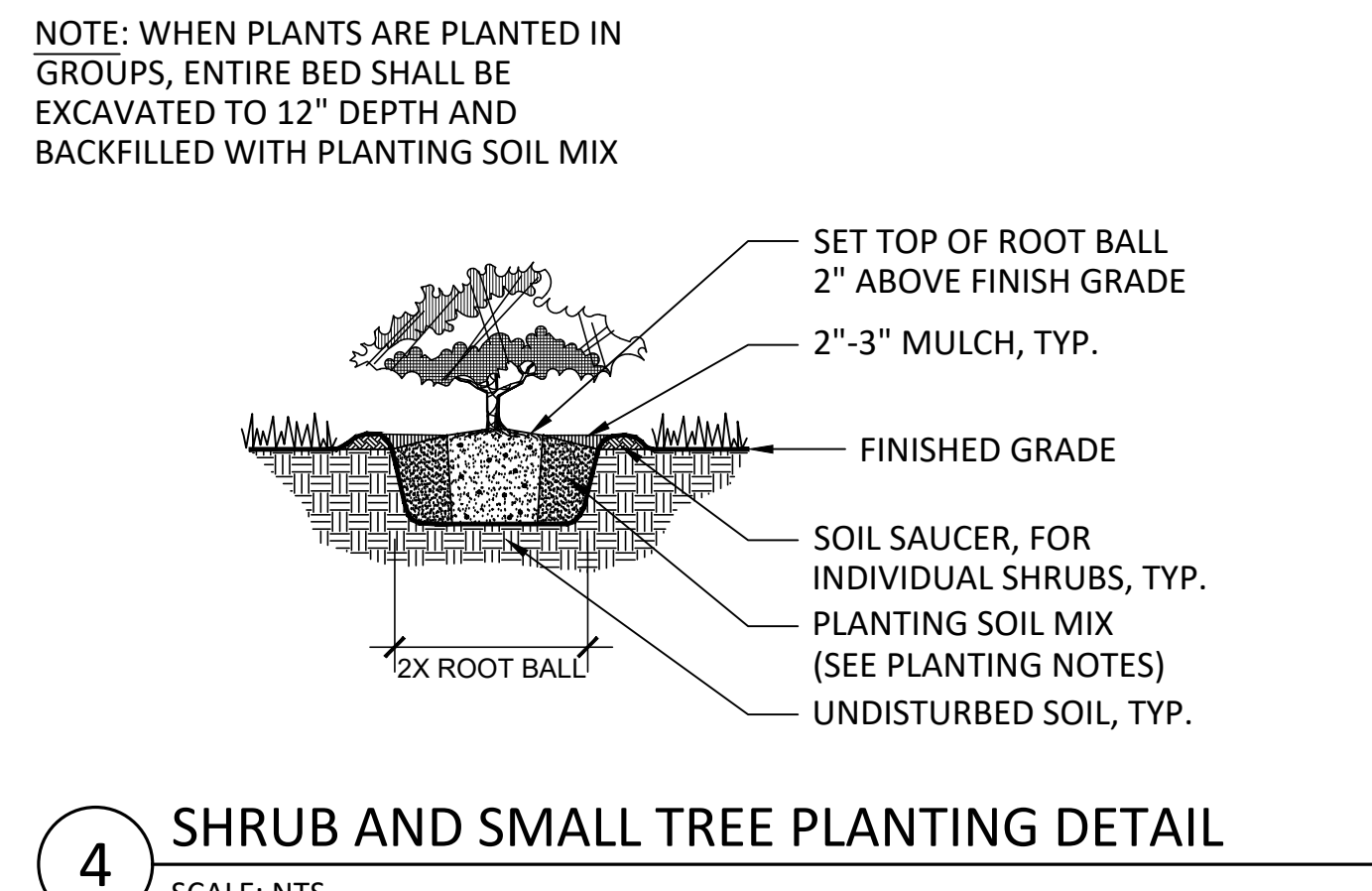
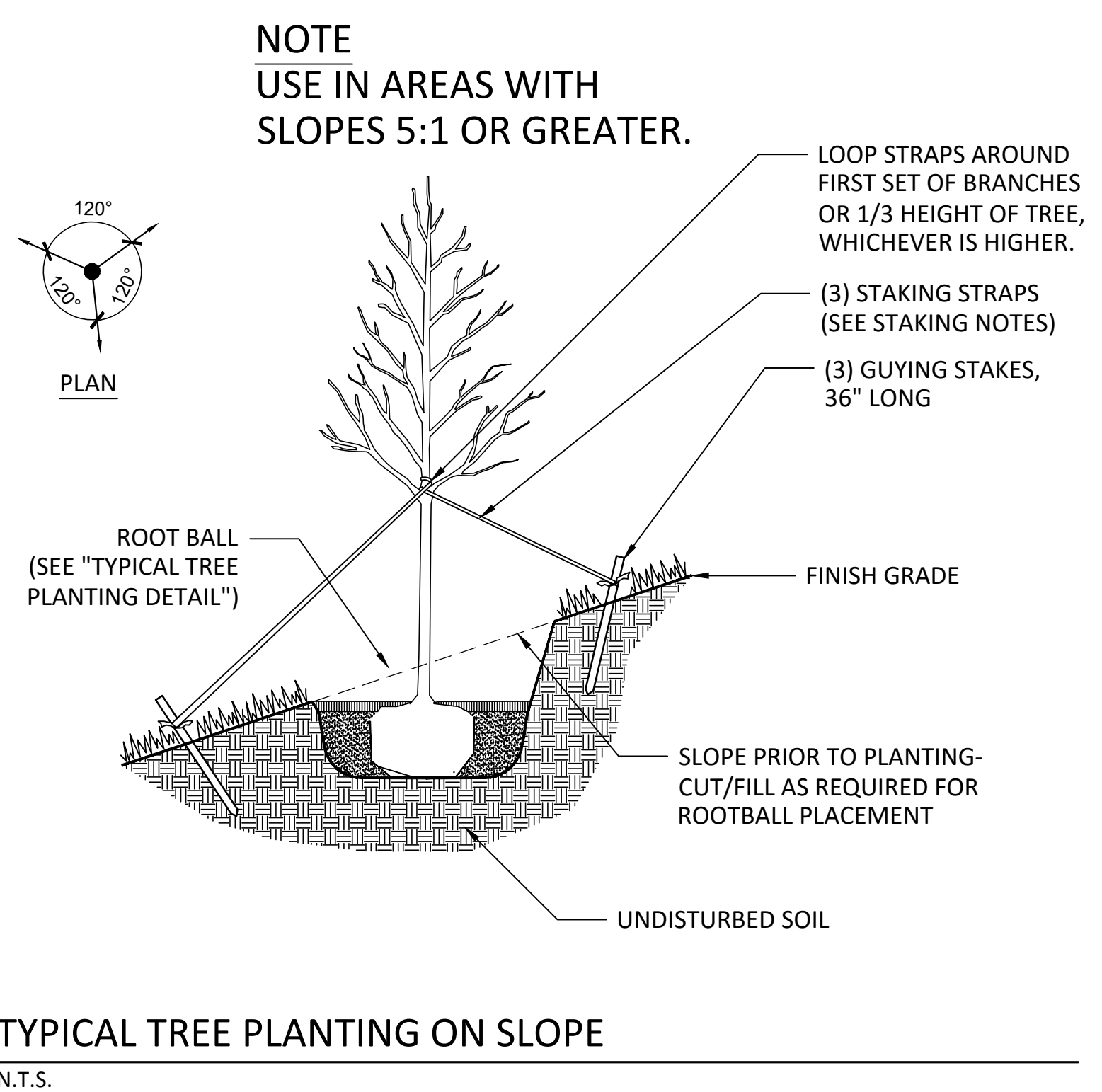
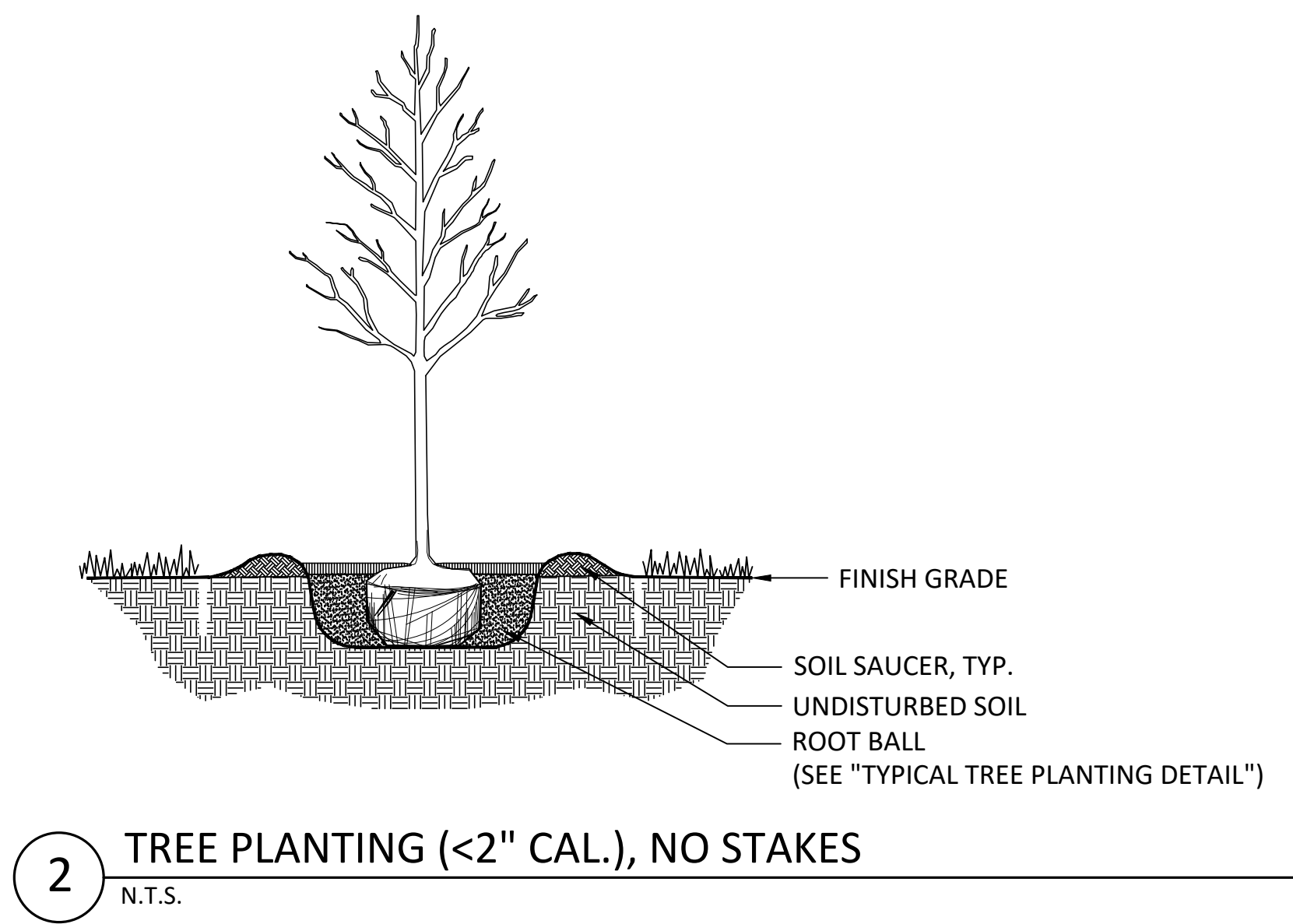
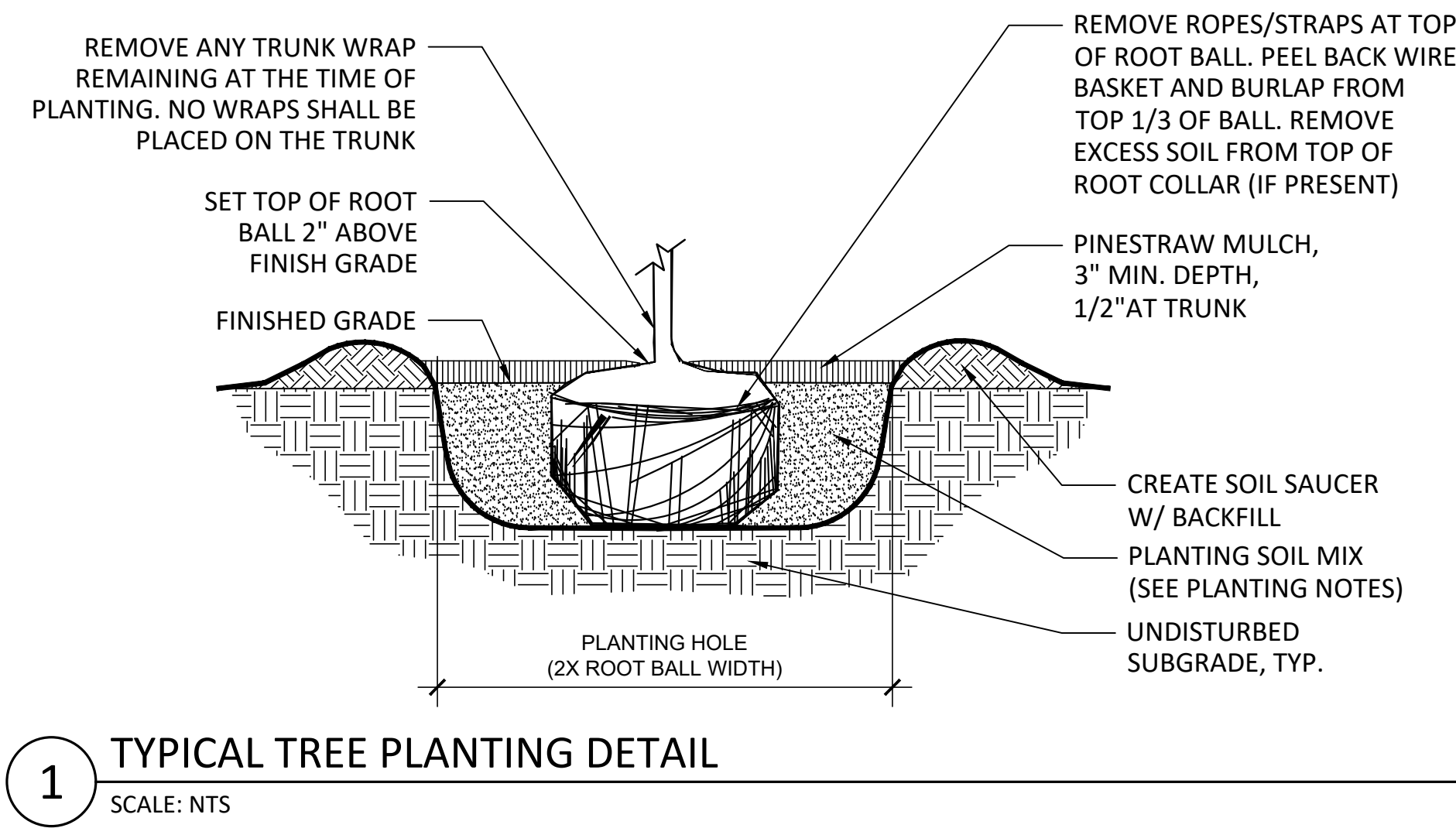
Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Buffer Mitigation Planting Plan**

Sheet No:  
**L-101**





Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Buffer Mitigation Planting Details**

Sheet No:  
**L-500**





**TREE PLAN INDEX:**

LT000 TREE PLAN NOTES AND CALCULATIONS  
 LT001 TREE AND LANDSCAPE KEY PLAN  
 LT100 TREE PROTECTION AND REMOVAL PLAN: AREA A  
 LT101 TREE PROTECTION AND REMOVAL PLAN: AREA B  
 LT102 TREE AND LANDSCAPE PLAN: AREA A  
 LT103 TREE AND LANDSCAPE PLAN: AREA B  
 LT400 LANDSCAPE ENLARGEMENT PLANS  
 LT500 TREE PLAN DETAILS

**GENERAL NOTES:**

- THE WORK SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, CODES AND REGULATIONS. IF ANY OF THESE NOTES ARE FOUND TO BE IN CONFLICT WITH LOCAL JURISDICTION NOTES AND SPECIFICATIONS, THEN LOCAL JURISDICTION NOTES AND SPECIFICATIONS TAKE PRECEDENCE.
- THE CONTRACTOR SHALL SECURE NECESSARY PERMITS PRIOR TO BEGINNING ANY WORK.
- CONTRACTOR MUST POSSESS CURRENT LICENSES AND/OR CERTIFICATIONS AS MAY BE REQUIRED BY LAW TO APPLY GENERAL OR RESTRICTED USE PESTICIDES AND CHEMICALS.
- USER OF THESE DRAWINGS IS CAUTIONED THAT EXISTING UNDERGROUND UTILITIES AND FOUNDATIONS AS SHOWN ARE NOT GUARANTEED, NOR IS THERE ANY GUARANTEE THAT ALL EXISTING UTILITIES AND FOUNDATIONS, WHETHER ABANDONED OR FUNCTIONAL, ARE SHOWN ON DRAWINGS. IF UNDERGROUND FOUNDATION OR UTILITY WHICH IS NOT SHOWN ON DRAWINGS IS ENCOUNTERED OR DAMAGED BY CONSTRUCTION WORK, NOTIFY THE LANDSCAPE ARCHITECT IMMEDIATELY.
- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CHECK AND VERIFY PROPOSED GRADES, DIMENSIONS, AND EXISTING CONDITIONS. REPORT DISCREPANCIES TO THE LANDSCAPE ARCHITECT FOR DIRECTION BEFORE PROCEEDING WITH WORK. WORK STARTED WITHOUT DIRECTION FROM THE LANDSCAPE ARCHITECT WILL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND WILL BE CORRECTED IF NECESSARY AT HIS/HER EXPENSE.
- THE CONTRACTOR SHALL CONTACT THE UTILITY PROTECTION SERVICE (811) TO LOCATE ALL ABOVE AND BELOW GROUND UTILITIES PRIOR TO BEGINNING WORK.
- THE CONTRACTOR SHALL PROVIDE TRAFFIC CONTROL MEASURES AS NEEDED TO ENSURE THE SAFETY OF VEHICULAR AND PEDESTRIAN TRAFFIC WITHIN CONSTRUCTION ZONES.
- THE CONTRACTOR SHALL MAINTAIN A CLEAN AND COURTEOUS WORK SITE FOR THE DURATION OF THE PROJECT. TRASH AND DEBRIS SHALL BE PICKED UP AND PROPERLY DISPOSED OF EACH DAY. VEHICLES AND EQUIPMENT SHALL BE CLEANED AND MAINTAINED REGULARLY SO AS NOT TO DRIP ANY FLUIDS OR TRACK AND SEDIMENT WITHIN THE PROPERTY.
- THE CONTRACTOR SHALL ENSURE THAT NO SEDIMENT LEAVES THE WORK SITE DURING CONSTRUCTION. SEDIMENT THAT ACCUMULATES ALONG THE CURB OR ON THE ROADWAY SHALL BE SWEEPED UP AT THE END OF EACH WORK DAY AND PRIOR TO EACH RAIN EVENT.
- ALL CONSTRUCTION DEBRIS SHALL BE HAULED OFF SITE AND PROPERLY DISPOSED OF IN ACCORDANCE WITH APPLICABLE LAWS.
- EXISTING PROPERTY THAT IS DAMAGED AS A RESULT OF CONSTRUCTION ACTIVITIES FROM THIS PROJECT SHALL BE REPLACED OR REPAIRED TO ITS ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE.
- THE CONTRACTOR SHALL LEAVE THE PROJECT SITE IN A PRISTINE CONDITION UPON COMPLETION OF THE WORK. REMOVE AND PROPERLY DISPOSE OF ALL TRASH, DEBRIS, EXCESS SOIL, ETC. CLEAN ALL VISIBLE SURFACES SO THEY ARE FREE FROM DIRT, SMUDGES, STAINS, ETC.

**TREE PROTECTION NOTES:**

- THE "CRITICAL ROOT ZONE" (CRZ) IS DETERMINED BY CONVERTING THE TREE TRUNK DIAMETER IN INCHES MEASURED AT BREAST HEIGHT (DBH) TO CRZ RADIUS IN FEET. (REFER TO THE MOST CURRENT VERSION OF THE LOCAL ORDINANCE FOR CONVERSION FACTOR.)
- ALL TREE PROTECTION FENCING SHALL BE INSTALLED PRIOR TO AND MAINTAINED THROUGHOUT LAND DISTURBING AND CONSTRUCTION ACTIVITIES, AND SHALL NOT BE REMOVED UNTIL THE FINAL LANDSCAPING IS INSTALLED.
- TREE PROTECTION AREA SHALL INCLUDE NO LESS THAN THE TOTAL AREA BENEATH THE TREE CANOPY AS DEFINED BY THE CRZ OF THE TREE OR GROUP OF TREES COLLECTIVELY, UNLESS OTHERWISE INDICATED ON THE PLANS.
- TREE PROTECTION FENCES MUST HAVE SIGNAGE IN BOTH ENGLISH AND SPANISH LANGUAGES THAT READS "STAY OUT", "NO ENTRADA", AND "TREE SAVE", "SALVE UN ARBOL", POSTED ON THE FENCE EVERY 20 FEET WITH A MINIMUM OF 4 SIGNS. SIGNS REQUESTING SUBCONTRACTOR COOPERATION AND COMPLIANCE WITH THE TREE PROTECTION STANDARDS SHALL ALSO BE POSTED AT THE JOB SITE ENTRANCES.
- ANY ACTIVITY THAT INVOLVES DISTURBING SOIL WITHIN THE CRZ OF TREES TO REMAIN (I.E. CUT, FILL, OR COMPACTION) OR CLOSE PHYSICAL CONTACT BETWEEN EQUIPMENT AND TREES TO REMAIN MUST BE REVIEWED AND APPROVED IN ADVANCE BY THE LANDSCAPE ARCHITECT AND THE LOCAL JURISDICTIONAL AUTHORITY.
- TREE SAVE AREAS SHALL NOT BE USED FOR EMPLOYEE PARKING OR LOITERING, LOCATION OF TEMPORARY SANITATION FACILITIES, ACCESS, STAGING, MATERIALS STORAGE, TRENCHING, GRADING, OR STORAGE OF DEMOLITION EQUIPMENT.
- NO DEMOLITION, EQUIPMENT FUELING, LUBRICATION, OR MAINTENANCE SHALL BE ALLOWED WITHIN THE TREE SAVE AREAS OR WITHIN THE CRZ OF TREES TO REMAIN.
- STORAGE OF ROLL OFF DUMPSTERS, FUEL, LUBRICANTS, CHEMICALS, ETC. WILL NOT BE PERMITTED ADJACENT TO THE TREE SAVE AREAS OR THE CRZ OF TREES TO REMAIN.

**LANDSCAPE NOTES:**

- LANDSCAPE PLANS ARE FOR THE LOCATION AND IDENTIFICATION OF PLANT MATERIAL ONLY. NO OTHER WORK IS TO BE PERFORMED BASED ON THESE PLANS.
- QUANTITIES ON THE PLANT SCHEDULE ARE PROVIDED FOR CONVENIENCE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HIS/HER OWN QUANTITY CALCULATIONS. IN THE EVENT OF A DISCREPANCY BETWEEN THE LANDSCAPE PLANS AND THE PLANT SCHEDULE, THE LANDSCAPE PLAN WILL TAKE PRECEDENCE. THE CONTRACTOR SHALL INFORM THE LANDSCAPE ARCHITECT IMMEDIATELY UPON DISCOVERING QUANTITY DISCREPANCIES.
- THE CONTRACTOR SHALL NOT CHANGE OR SUBSTITUTE PLANT VARIETIES OR SPECIES WITHOUT PRIOR WRITTEN APPROVAL FROM THE LANDSCAPE ARCHITECT. PLANT MATERIAL SHALL BE PLACED AS SHOWN ON THE LANDSCAPE PLANS.
- CONTRACTOR SHALL ENSURE POSITIVE DRAINAGE OF ALL PLANTING HOLES AND PLANT BEDS PRIOR TO INSTALLATION.
- NEW SHRUB AND GROUND COVER PLANTING SHALL BE A MINIMUM OF 36" AWAY FROM EXISTING TREES.
- TOPSOIL: ANY AVAILABLE TOP SOIL WITHIN THE LIMITS OF DISTURBANCE SHALL BE STOCKPILED ON SITE FOR RE-USE IN LANDSCAPE WORK. IF NO TOPSOIL IS STOCKPILED, THE CONTRACTOR SHALL IMPORT TOPSOIL, AS REQUIRED, TO COMPLETE LANDSCAPE WORK.  
  
IMPORTED TOP SOIL SHALL BE FERTILE, FRIABLE, NATURAL LOAM, SURFACE SOIL, REASONABLY FREE OF ROOTS, STUMPS AND LARGE STONES AND FREE OF BRUSH, WEEDS, LITTER, AND OTHER EXTRANEOUS MATTER HARMFUL TO PLANT GROWTH.  
  
OBTAIN FROM LOCAL SOURCES OR AREAS HAVING SIMILAR SOIL CHARACTERISTICS TO THAT FOUND AT PROJECT SITE. OBTAIN TOPSOIL FROM NATURALLY, WELL DRAINED SITES WHERE TOPSOIL OCCURS IN A DEPTH OF NOT LESS THAN FOUR (4) INCHES. DO NOT OBTAIN FROM BOGS OR MARSHES.
- COMPOST: SHALL BE DARK BROWN IN COLOR. PARENT MATERIAL SHALL NO LONGER BE VISIBLE. STRUCTURE SHALL BE FINE AND MEDIUM SIZE PARTICLES. SMALL SHALL BE RICH HUMUS LIKE THE FOREST FLOOR WITH NO AMMONIA OR ANAEROBIC ODORS. COMPOST SHALL MEET THE FOLLOWING MINIMUM REQUIREMENTS:  
 7.1. COMPOST SHALL BE "US COMPOSTING COUNCIL-STA" CERTIFIED.  
 7.2. ORGANIC CONTENT: 35%-65%  
 7.3. MATURITY TEST: "STABLE" OR "VERY STABLE" RATING BASE ON CO2 RESPIRATION.  
 7.4. CARBON/NITROGEN RATIO: 14-20  
 7.5. ELECTRICAL CONDUCTIVITY (CEC): LESS THAN 5.0 dS/m  
 7.6. PATHOGENS AND METALS: PER USEPA STANDARDS FOR CLASS A BIOSOLIDS.  
 7.7. CONTAMINANTS: LESS THAN 1% BY WT. FOR METAL, GLASS, PLASTIC AND OTHER "INERTS". LESS THAN 0.5% BY WT. FOR PLASTIC FILM.

**LANDSCAPE NOTES (continued):**

- PLANTING SOIL MIX REFERS TO THE PLANTING MEDIUM USED TO BACKFILL INDIVIDUAL PLANTING HOLES AND SHALL CONSIST OF THE FOLLOWING:  
 80% (BY VOLUME): TOPSOIL (SEE NOTE #6)  
 20% (BY VOLUME): AGED COMPOST (SEE NOTE #7)  
*CONTRACTOR MAY SUBMIT ALTERNATIVE MIX FOR REVIEW.*
- MYCORRHIZAL INOCULATE: EACH TREE AND SHRUB SHALL BE INOCULATED WITH MYCORRHIZAE. MICORRHIZAE SHALL BE A GRANULAR PRODUCT CONTAINING BOTH ENDO AND ECTO-MICORRHIZAL FUNGI, SUCH AS MYCOR TREE SAVER™ TRANSPLANT, AS MANUFACTURED BY PLANT HEALTH CARE, INC. (800-421-9051) OR APPROVED EQUAL. PROVIDE SAMPLE OR PRODUCT DATA SHEET FOR APPROVAL PRIOR TO WORK.**  
  
**INOCULANT SHALL BE ADDED AFTER TREES AND SHRUBS HAVE BEEN PLACED IN THE PLANTING HOLE. INCORPORATE INTO THE TOP SIX (6) TO EIGHT (8) INCHES OF BACKFILL. APPLY PER MFG. RECOMMENDATIONS.**
- QUALITY OF PLANT MATERIAL: ALL PLANTS SHALL CONFORM TO THE CURRENT VERSION OF THE AMERICAN STANDARD FOR NURSERY STOCK (ANSI Z60.1). PLANT MATERIAL SHALL BE FREE OF INSECTS, DISEASE AND/OR INJURY, AND SHALL HAVE A HEALTHY ROOT SYSTEM WITH NO CIRCLING OR KINKED ROOTS. CONTAINER PLANTS SHALL NOT BE ROOT BOUND. TREES SHALL HAVE STRAIGHT TRUNKS, A STRONG DOMINANT CENTRAL LEADER (AS REQUIRED BY SPECIES), DENSE CANOPIES AND STRONG BRANCHING WITH GOOD CROTCH ANGLES.
- INSPECTION AND APPROVAL OF PLANT MATERIAL: ALL PLANT MATERIAL SHALL BE INSPECTED AND APPROVED BY THE OWNER'S REPRESENTATIVE UPON DELIVERY TO THE SITE AND PRIOR TO INSTALLATION. CONTRACTOR SHALL PROVIDE AT LEAST ONE WEEK NOTICE PRIOR TO PLANT DELIVERY.
- PLANT MATERIAL SHALL BE SUFFICIENTLY WATERED TO WET THE ENTIRE ROOT BALL WITHIN TWO HOURS OF PLANTING.
- MAINTENANCE: CONTRACTOR SHALL MAINTAIN ALL PLANT MATERIAL FROM THE TIME IT IS INSTALLED UNTIL FINAL ACCEPTANCE OR WHEN THE OWNER TAKES OVER MAINTENANCE, WHICHEVER OCCURS FIRST. MAINTENANCE SHALL INCLUDE BUT NOT BE LIMITED TO MOWING, EDGING, WEEDING, WATERING, PRUNING, FERTILIZING, ETC.
- WARRANTY: CONTRACTOR SHALL PROVIDE A WARRANTY ON ALL PLANT MATERIAL AND LABOR FOR A PERIOD OF ONE YEAR OR PER JURISDICTIONAL REQUIREMENTS, WHICHEVER IS LONGER. WARRANTY PERIOD SHALL BEGIN UPON FINAL COMPLETION OR WHEN THE OWNER TAKES OVER MAINTENANCE, WHICHEVER OCCURS FIRST.  
  
THE CONTRACTOR SHALL MAKE PERIODIC INSPECTIONS OF THE PROJECT DURING THE WARRANTY PERIOD TO ENSURE THAT THE ESTABLISHMENT RATE OF GROWTH IS ADEQUATE. ANY METHODS OR PRODUCTS DEEMED NOT NORMAL OR DETRIMENTAL TO GOOD PLANT GROWTH SHALL BE REPORTED TO THE OWNER IN WRITING. FAILURE TO INSPECT AND REPORT WILL BE INTERPRETED AS APPROVAL, AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL REPLACEMENTS.

**TREE PLANTING NOTES:**

- ALL TREES SHALL BE TEN (10) FEET MINIMUM FROM GAS LINES AND SANITARY SEWER, AND FIVE (5) FEET MINIMUM FROM FIRE HYDRANTS AND ALL OTHER UNDERGROUND UTILITIES.
- IF TREES ARE INSTALLED NEAR A BUILDING, LANDSCAPE CONTRACTOR SHALL COORDINATE WITH GENERAL CONTRACTOR TO ENSURE THAT NO ROOF OR FOUNDATION DRAIN LINES ARE LOCATED WITHIN FIVE (5) FEET OF SCHEDULED TREES.
- TREES SHALL BE SET PLUMB AND LEVEL.
- CAREFULLY LIFT AND SET TREES BY THE ROOT BALL ONLY. DO NOT LIFT USING THE TRUNK OR STEM. IF TREES ARE TOO LOW OR NOT PLUMB AFTER SETTING IN THE HOLE, RE-SET BY ADJUSTING THE ROOT BALL AS NEEDED.
- FOR TREES PLANTED IN TREE ISLANDS: EXCAVATE THE ENTIRE TREE ISLAND TO A DEPTH OF 12", LOOSEN ANY HARDPAN, AND BACKFILL WITH TOPSOIL PRIOR TO DIGGING THE PLANTING HOLE.
- REFER TO PLANTING DETAILS FOR ADDITIONAL INFORMATION.

**TREE STAKING NOTES:**

- TREES 2" CALIPER AND SMALLER SHOULD NOT BE STAKED.
- STAKING STRAPS SHALL BE SOFT, FLEXIBLE MATERIAL, 3/4" WIDTH, MANUFACTURED FOR THE PURPOSE OF TREE ANCHORING SUCH AS WOVEN POLYPROPYLENE WEBBING.
- STRAPS SHALL BE ATTACHED IN THE LOWEST BRANCH CROTCH POSSIBLE, BUT NO HIGHER THAN 2/3 THE HEIGHT OF THE MAIN TRUNK.
- STRAPS SHALL LAY FLAT AGAINST THE TREE TRUNK AND SHALL NOT BE TWISTED OR FOLDED.
- STRAPS SHALL BE FIRMLY ATTACHED TO DEADMEN AND HAVE SUFFICIENT SLACK TO ALLOW TRUNK TO SWAY APPROXIMATELY 1" TO 2" IN ANY DIRECTION.

**PLANT BED PREPARATION NOTES:**

- FLAG OR STAKE PROPOSED TREE LOCATIONS AND MARK ALL BEDLINES WITH ORANGE OR WHITE PAINT. NOTIFY THE OWNER'S REPRESENTATIVE FOR REVIEW AND FIELD ADJUSTMENTS, AS NEEDED.
- UPON APPROVAL OF BEDLINES, SPRAY ALL EXISTING VEGETATION WITHIN PLANT BEDS WITH LEGALLY APPROVED WEED KILLER. REMOVE VEGETATION AFTER IT HAS BEEN KILLED.
- DIG A TRENCH AROUND THE PERIMETER OF EACH BED, PER "TRENCH EDGER DETAIL", SHEET L-500.
- TILL EACH PLANT BED TO A DEPTH OF SIX (6) INCHES WITH A ROTOTILLER.
- REMOVE ANY LARGE ROCKS, ROOTS, TRASH OR OTHER HARMFUL MATERIAL FROM THE BED(S).
- ADD TWO (1) INCHES OF TOP SOIL TO THE SURFACE OF EACH BED. (SEE "LANDSCAPE NOTE" #6).
- ADD ONE HALF (1/2) INCH OF AGED COMPOST TO THE SURFACE OF EACH BED. (SEE "LANDSCAPE NOTE" #7).
- AFTER APPLYING TOP SOIL AND COMPOST, TILL EACH BED A SECOND TIME TO THOROUGHLY INCORPORATE AMENDMENTS INTO THE TOP SIX (6) INCHES OF SOIL.
- AFTER TILLING, RAKE BEDS SMOOTH, FORM A SLIGHT CROWN IN THE CENTER, AND ROLL COMPACT WITH A SOD ROLLER.
- RE-STAKE ANY PROPOSED TREES WITHIN THE PLANT BEDS FOR APPROVAL PRIOR TO PLANTING. INSTALL TREES PER PLANTING DETAILS.
- ONCE TREES ARE INSTALLED, LAYOUT REMAINING PLANT MATERIAL IN EACH BED FOR REVIEW AND APPROVAL BY OWNER'S REPRESENTATIVE.
- INSTALL REMAINING PLANT MATERIAL PER PLANTING DETAILS.
- APPLY AN APPROVED PRE-EMERGENT HERBICIDE (WEED INHIBITOR) TO THE ENTIRE PLANT BED AFTER PLANTS HAVE BEEN INSTALLED. APPLY PER MFG. RECOMMENDATIONS. USE A SEASONALLY APPROPRIATE PRODUCT DEPENDING ON INSTALLATION DATES. SUBMIT PRODUCT INFORMATION TO OWNER'S REPRESENTATIVE FOR APPROVAL.
- REMOVE ALL PLANT TAGS AFTER APPROVAL OF PLANT INSTALLATION BY LANDSCAPE ARCHITECT.
- MULCH ALL PLANT BEDS AND TREE RINGS WITH FRESH, CLEAN PINESTRAW TO A MINIMUM DEPTH OF THREE (3) INCHES (UNLESS OTHERWISE NOTED ON PLANS). DO NOT PILE MULCH AROUND THE BASE OF PLANTS OR TREE TRUNKS. ALL MULCH EDGES SHALL BE NEATLY TUCKED. ALL STRING AND/OR BAILING WIRE SHALL BE REMOVED. "DUST" SHRUBS AND GROUND COVER AFTER MULCHING TO REMOVE LOOSE PINESTRAW FROM THE PLANTS.
- WATER PLANT BEDS IMMEDIATELY AFTER INSTALLING MULCH. ALL PLANTS SHALL BE WATERED THE SAME DAY THEY ARE INSTALLED. APPLY AT LEAST ONE (1) INCH OF WATER TO EACH BED.



Date: 07/16/2024  
 Project No: 2022-024  
 Drawn By: PS  
 Checked By: MK

**Johns Homestead Park & Dam Improvements**  
 City of Tucker Parks and Recreation  
 4898 Lavista Road  
 Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

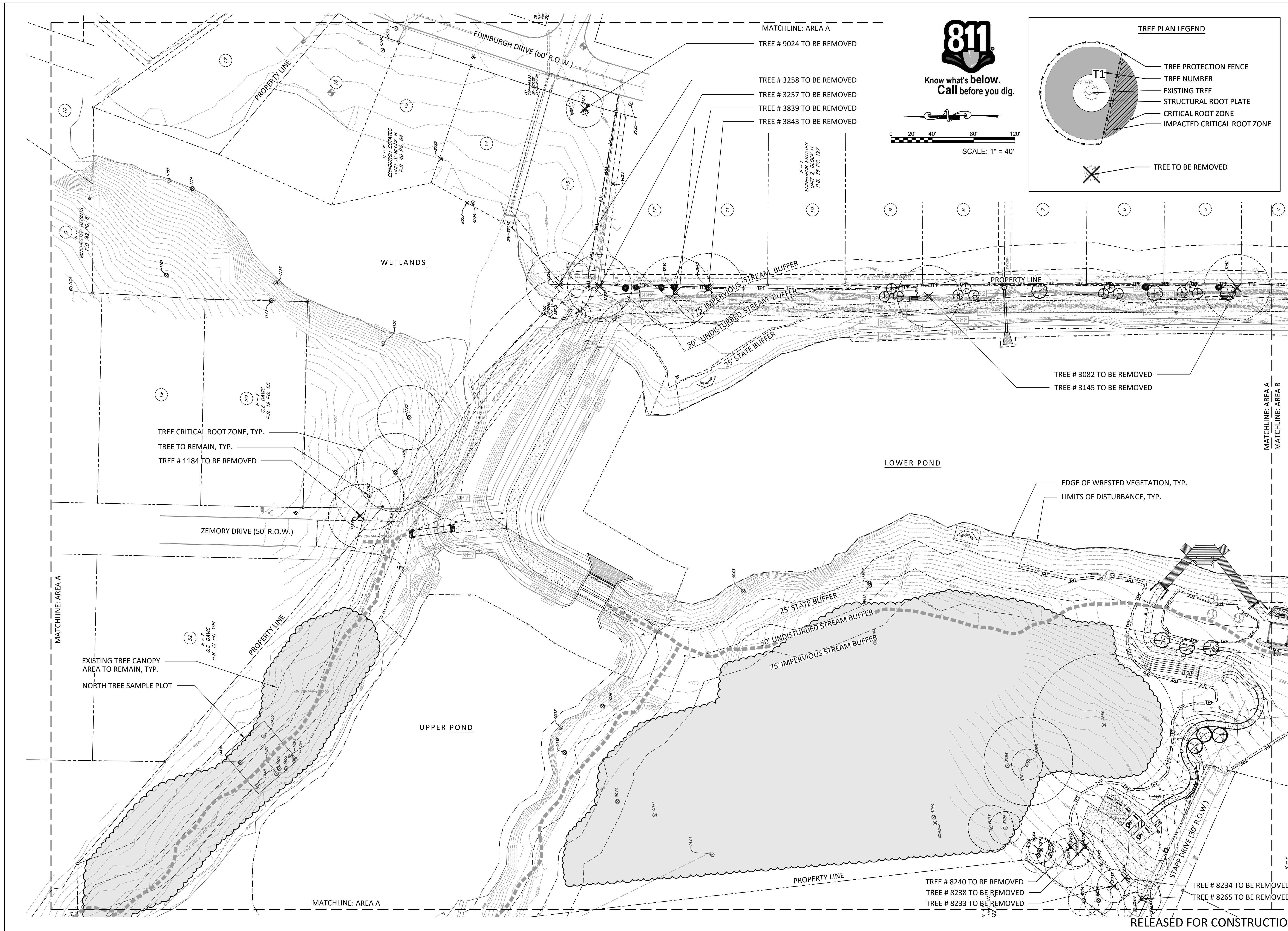
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**Tree Plan Notes**

Sheet No:  
**LT000**

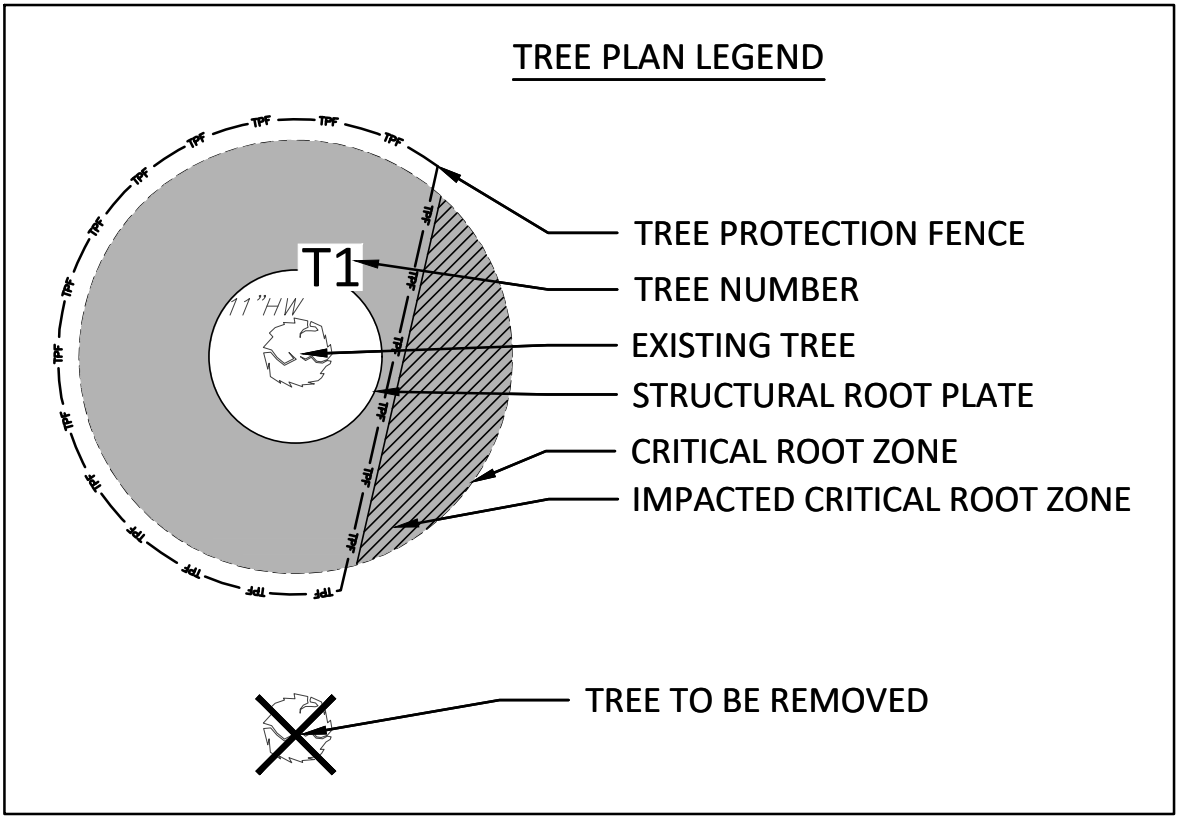
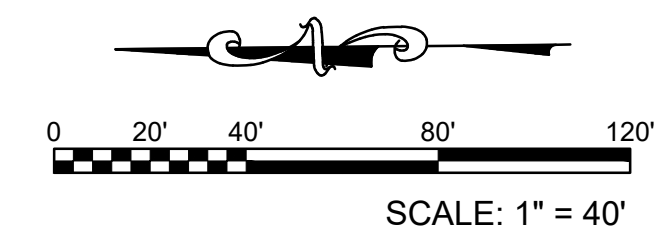
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City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

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Sheet Title:  
**Tree Protection and Removal Plan: Area A**

Sheet No:  
**LT100**

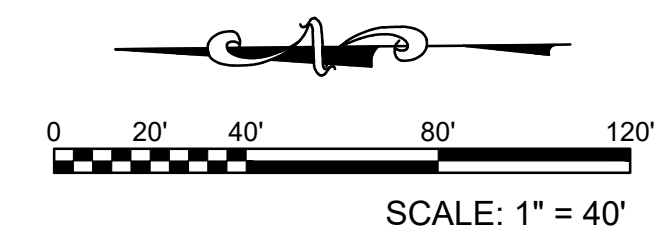
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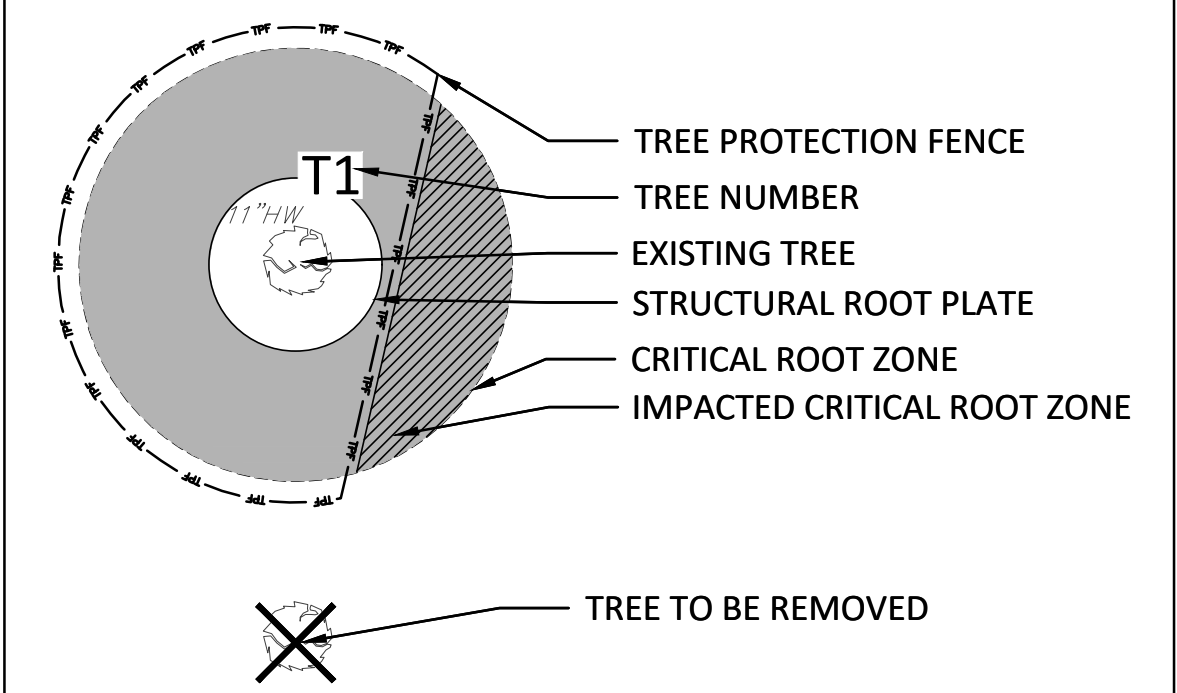
MATCHLINE: AREA B



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TREE PLAN LEGEND



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4898 Lavista Road  
Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Tree Protection and Removal Plan: Area B**

Sheet No:  
**LT101**

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TREE # 3081 TO BE REMOVED  
TREE # 9020 TO BE REMOVED  
TREE # 9018 TO BE REMOVED

N = F  
EDINBURGH WOODS  
UNIT 1, BLOCK A  
P.B. 60 PG. 34

PROPERTY LINE

SOUTH TREE SAMPLE PLOT  
EXISTING TREE CANOPY AREA TO REMAIN, TYP.

LOWER POND

LIMITS OF DISTURBANCE, TYP.  
TREE # 9002 TO BE REMOVED  
EDGE OF WRESTED VEGETATION, TYP.

TREE TO REMAIN, TYP.  
TREE CRITICAL ROOT ZONE, TYP.

TREE # 9004 TO BE REMOVED  
TREE # 9003 TO BE REMOVED

N = F  
MONTREAL WOODS  
UNIT 2  
P.B. 41 PG. 141

N = F  
R.O. (N.O. TRAIL)  
P.B. 41 PG. 141

N = F  
MONTREAL WOODS  
UNIT 2  
P.B. 41 PG. 141

N = F  
ROLAND ROBERTS  
D.B. 16756 PG. 17

MATCHLINE: AREA B

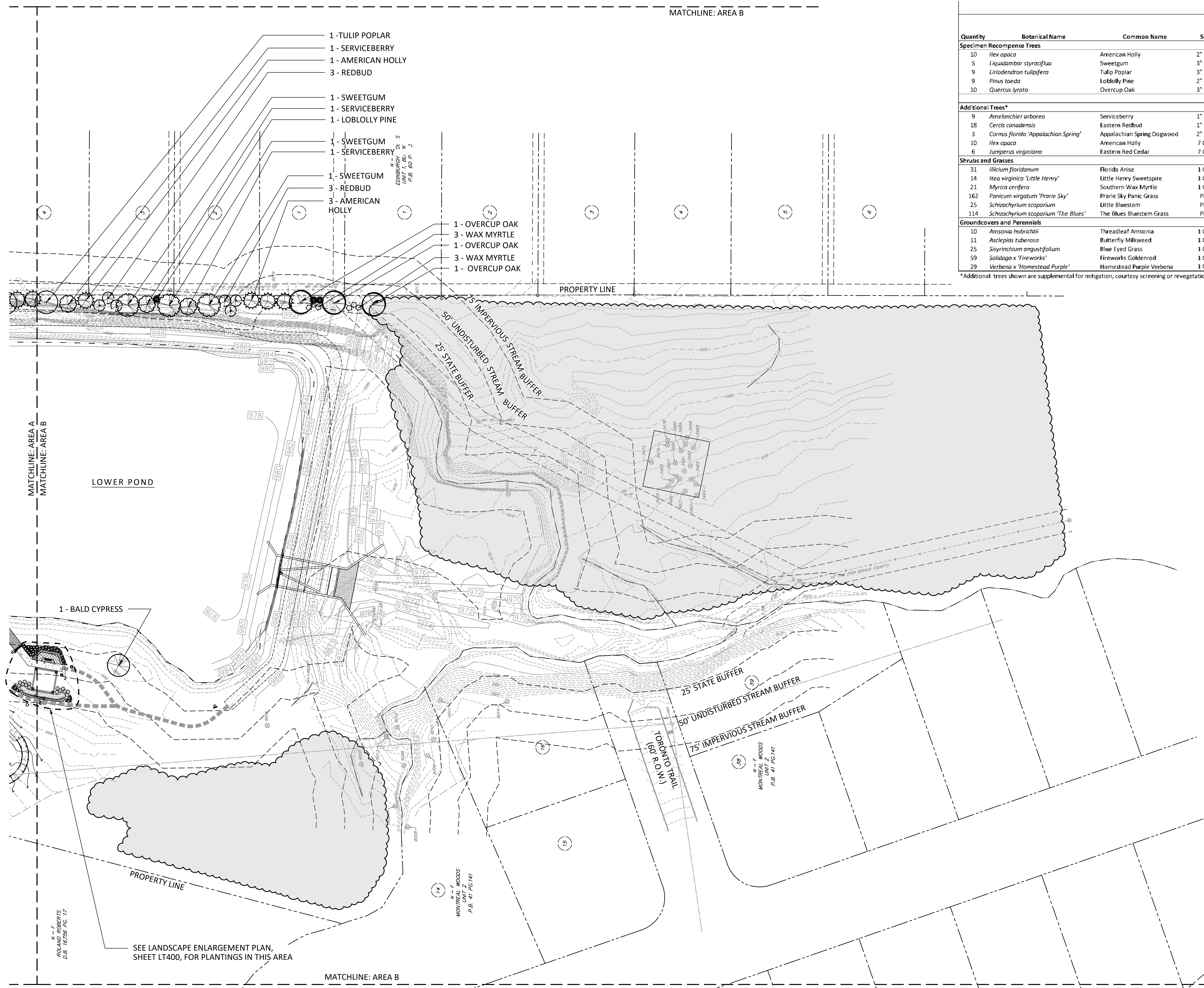
MATCHLINE: AREA A  
MATCHLINE: AREA B

MATCHLINE: AREA B









- 1 - TULIP POPLAR
- 1 - SERVICEBERRY
- 1 - AMERICAN HOLLY
- 3 - REDBUD
- 1 - SWEETGUM
- 1 - SERVICEBERRY
- 1 - LOBLOLLY PINE
- 1 - SWEETGUM
- 1 - SERVICEBERRY
- 1 - SWEETGUM
- 3 - REDBUD
- 3 - AMERICAN HOLLY

- 1 - OVERCUP OAK
- 3 - WAX MYRTLE
- 1 - OVERCUP OAK
- 3 - WAX MYRTLE
- 1 - OVERCUP OAK

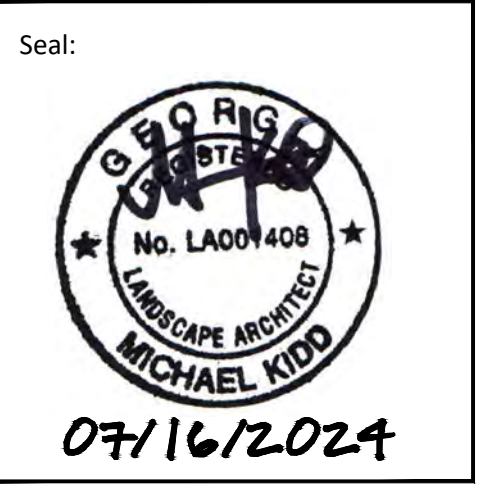
1 - BALD CYPRESS

Quantity	Botanical Name	Common Name	Size	Spacing	Remarks	Specimen Recompense Inches
<b>Specimen Recompense Trees</b>						
10	<i>Ilex opaca</i>	American Holly	2" Cal.	As Shown	1, 3, 4, 5, 6, 7, 10, 11	20
5	<i>Liquidambar styraciflua</i>	Sweetgum	3" Cal.	As Shown	1, 3, 4, 5, 6, 10, 11	15
9	<i>Liriodendron tulipifera</i>	Tulip Poplar	3" Cal.	As Shown	1, 3, 4, 5, 6, 10, 11	27
9	<i>Pinus taeda</i>	Loblolly Pine	2" Cal.	As Shown	1, 3, 4, 5, 6, 10, 11	18
10	<i>Quercus lyrata</i>	Overcup Oak	3" Cal.	As Shown	1, 3, 4, 5, 6, 10, 11	30
<b>Total Recompense Inches</b>						<b>110</b>
<b>Additional Trees*</b>						
9	<i>Amelanchier arborea</i>	Serviceberry	1" Cal.	As Shown	1, 3, 4, 5, 6, 9, 10, 12	
18	<i>Cercis canadensis</i>	Eastern Redbud	1" Cal.	As Shown	1, 3, 4, 5, 6, 9, 10, 12	
3	<i>Cornus florida 'Appalachian Spring'</i>	Appalachian Spring Dogwood	2" Cal.	As Shown	1, 3, 4, 5, 6, 9, 10, 12	
10	<i>Ilex opaca</i>	American Holly	7 Gal.	As Shown	1, 3, 4, 5, 6, 7, 10, 11	
6	<i>Juniperus virginiana</i>	Eastern Red Cedar	7 Gal.	As Shown	1, 3, 4, 5, 6, 7, 10, 11	
<b>Shrubs and Grasses</b>						
31	<i>Illicium floridanum</i>	Florida Anise	1 Gal.	As Shown	5, 6, 7, 10, 12	
14	<i>Itea virginica 'Little Henry'</i>	Little Henry Sweetspire	1 Gal.	3' O.C.	5, 6, 7, 10, 12	
21	<i>Myrica cerifera</i>	Southern Wax Myrtle	1 Gal.	As Shown	5, 6, 7, 10, 12	
162	<i>Panicum virgatum 'Prarie Sky'</i>	Prarie Sky Panic Grass	Plug	3' O.C.	5, 6, 10, 12	
25	<i>Schizachyrium scoparium</i>	Little Bluestem	Plug	As Shown	5, 6, 10, 12	
114	<i>Schizachyrium scoparium 'The Blues'</i>	The Blues Bluestem Grass	Plug	As Shown	5, 6, 10, 12	
<b>Groundcovers and Perennials</b>						
10	<i>Ansonia hubrichtii</i>	Threadleaf Ansonia	1 Gal.	2' O.C.	5, 6, 10, 12	
11	<i>Asclepias tuberosa</i>	Butterfly Milkweed	1 Gal.	3' O.C.	5, 6, 10, 12	
25	<i>Sisyrinchium angustifolium</i>	Blue Eyed Grass	1 Gal.	As Shown	5, 6, 10, 12	
59	<i>Solidago x 'Fireworks'</i>	Fireworks Goldenrod	1 Gal.	As Shown	5, 6, 10, 12	
29	<i>Verbena x 'Homestead Purple'</i>	Homestead Purple Verbena	1 Gal.	As Shown	5, 6, 10, 12	

\*Additional trees shown are supplemental for mitigation, courtesy screening or revegetation and may be specified in smaller sizes.

REMARKS:

1. Single Trunk	7. Full to Ground
2. Multi-Trunk, 3-5 canes, 1" min./cane	8. Tree Form
3. Straight Trunk	9. Specimen
4. Central Leader	10. Matched Set
5. Natural Branching	11. Balled and Burlapped (B&B)
6. Full Canopy / Head	12. Container



Date: 07/16/2024  
 Project No: 2022-024  
 Drawn By: PS  
 Checked By: MK

**Johns Homestead Park & Dam Improvements**  
 City of Tucker Parks and Recreation  
 4898 Lavista Road  
 Tucker, GA 30084

Revisions:

NO.	DATE	DESCRIPTION

Sheet Title:  
**Tree and Landscape Plan: Area B**

Sheet No:  
**LT103**

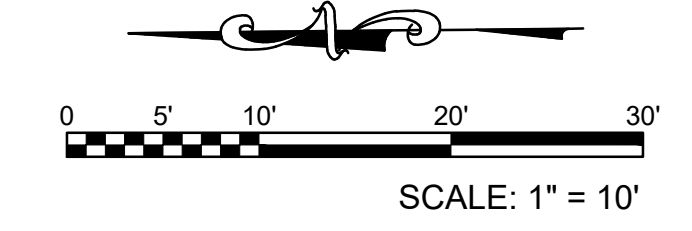
RELEASED FOR CONSTRUCTION



SEE SHEET LT103 FOR PLANT SCHEDULE



Know what's below. Call before you dig.



**ROOT DESIGN STUDIO**  
landscape architecture

2300 Henderson Mill Road  
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Atlanta, Georgia 30345  
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Seal:

07/16/2024

Date: 07/16/2024  
Project No: 2022-024  
Drawn By: PS  
Checked By: MK

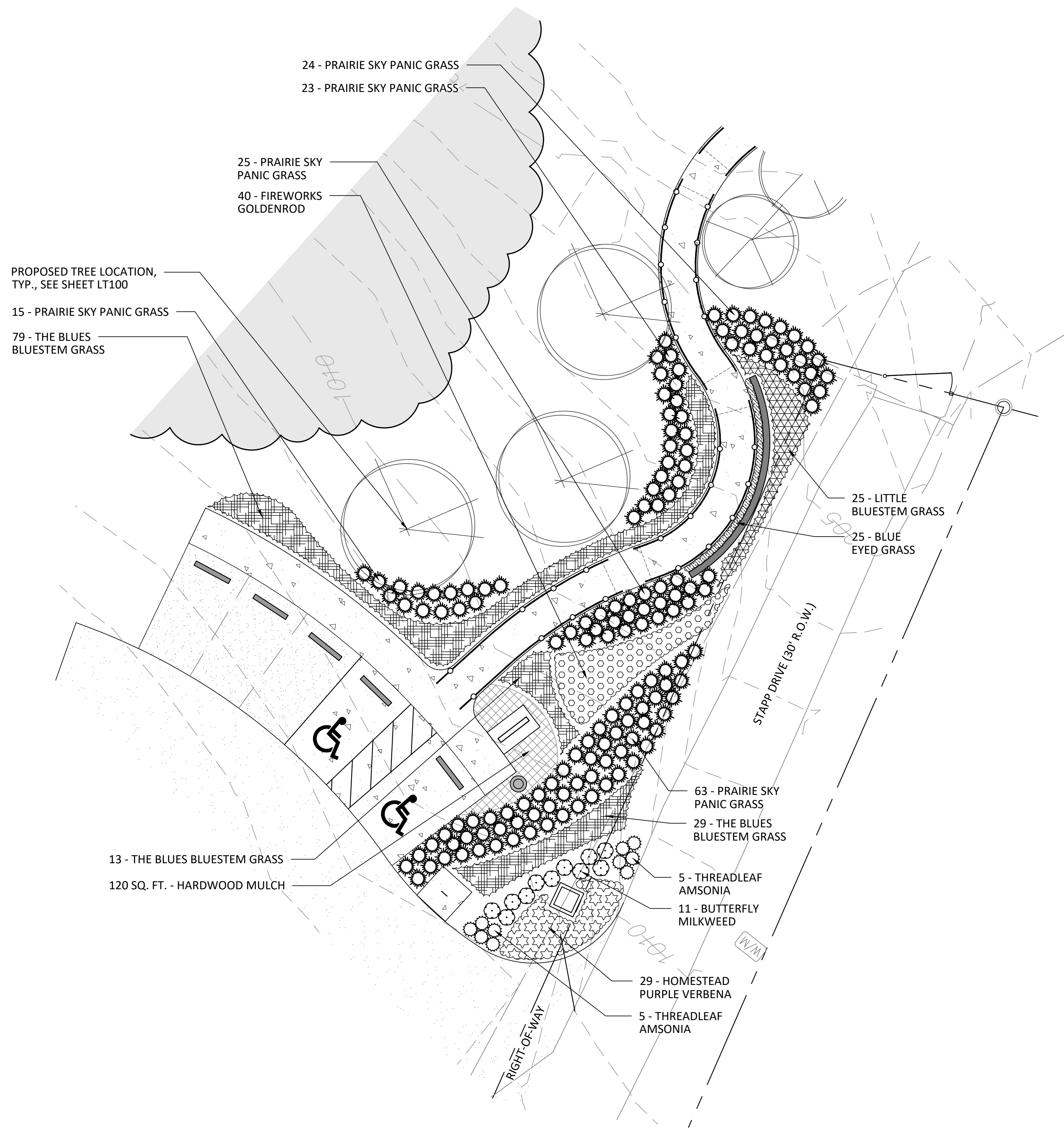
**Johns Homestead Park & Dam Improvements**  
City of Tucker Parks and Recreation  
4898 Lavista Road  
Tucker, GA 30084

Revisions:

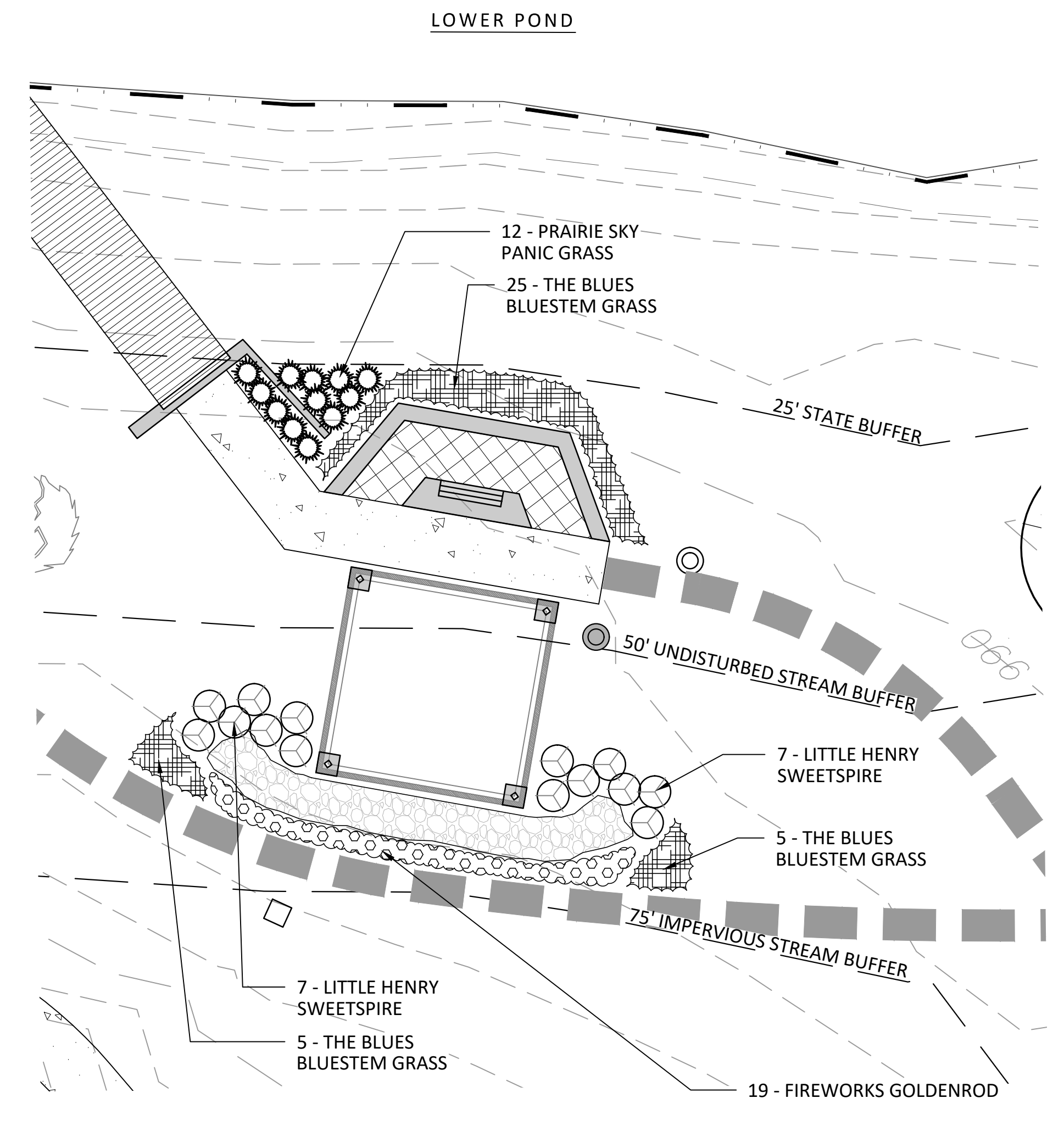
NO.	DATE	DESCRIPTION

Sheet Title:  
**Landscape Enlargement Plans**

Sheet No:  
**LT400**



**1 STAPP DRIVE PARKING LOT LANDSCAPE ENLARGEMENT PLAN**  
SCALE: 1" = 10'

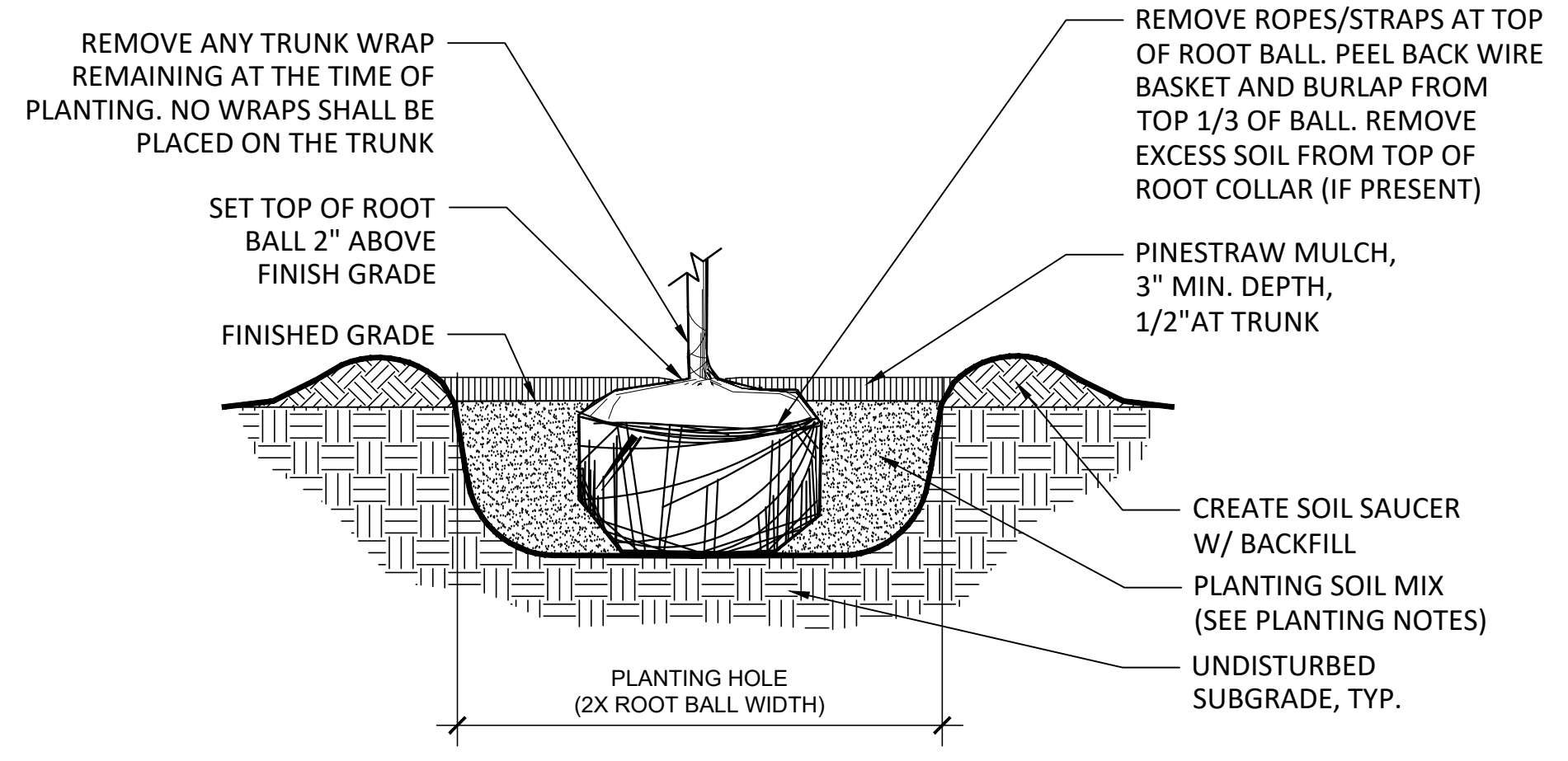


**2 PAVILION AREA LANDSCAPE ENLARGEMENT PLAN**  
SCALE: 1" = 10'

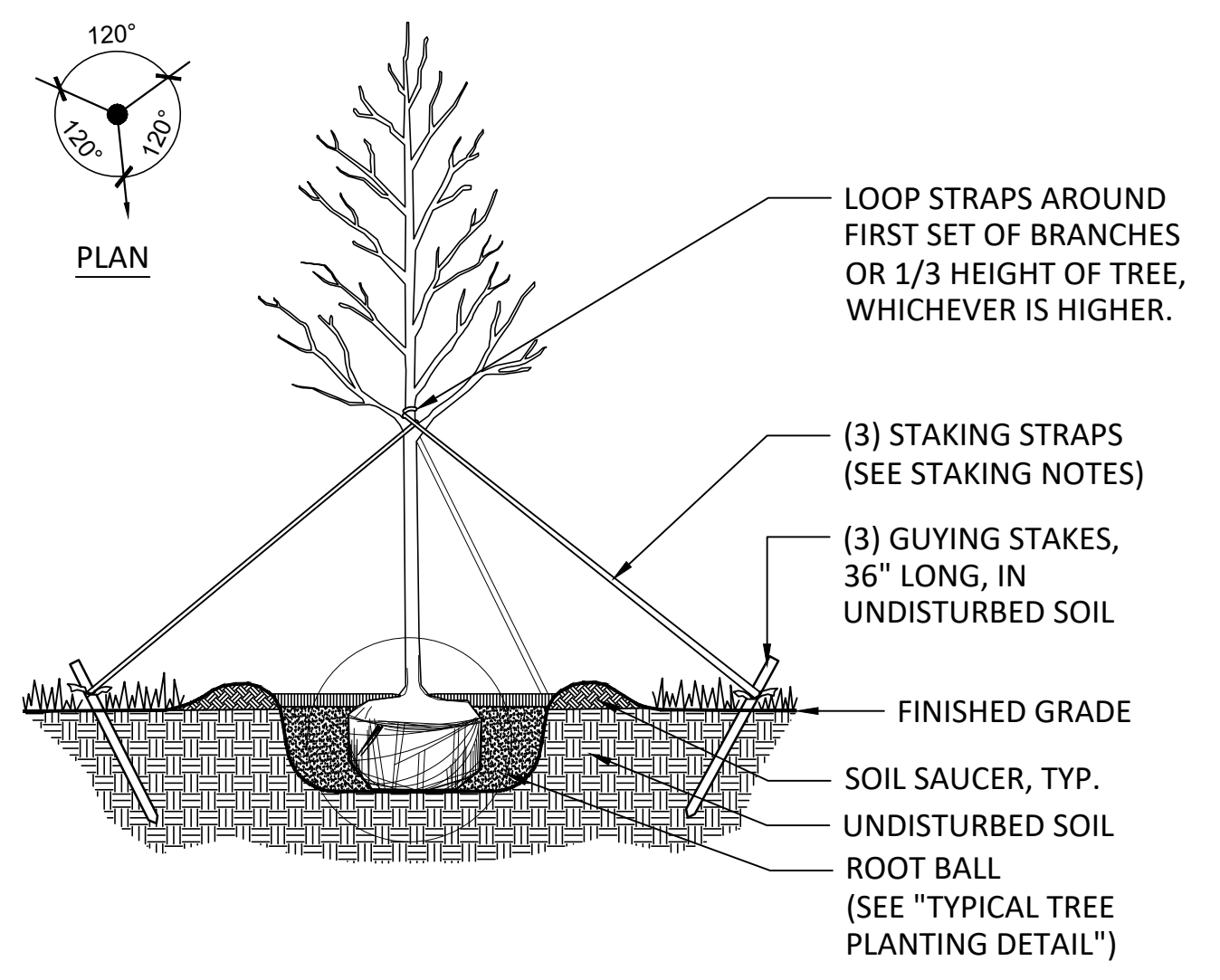


Revisions:

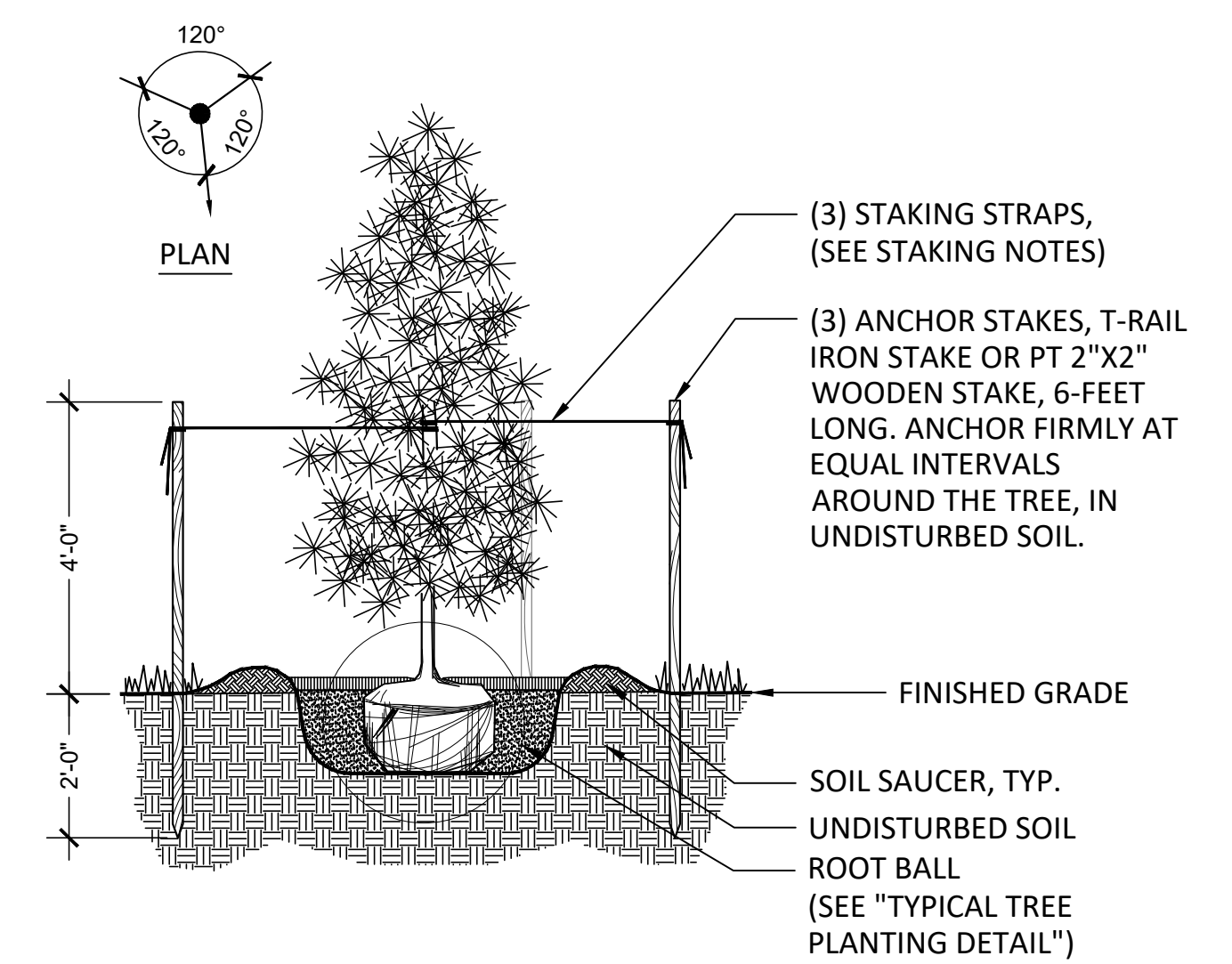
NO.	DATE	DESCRIPTION



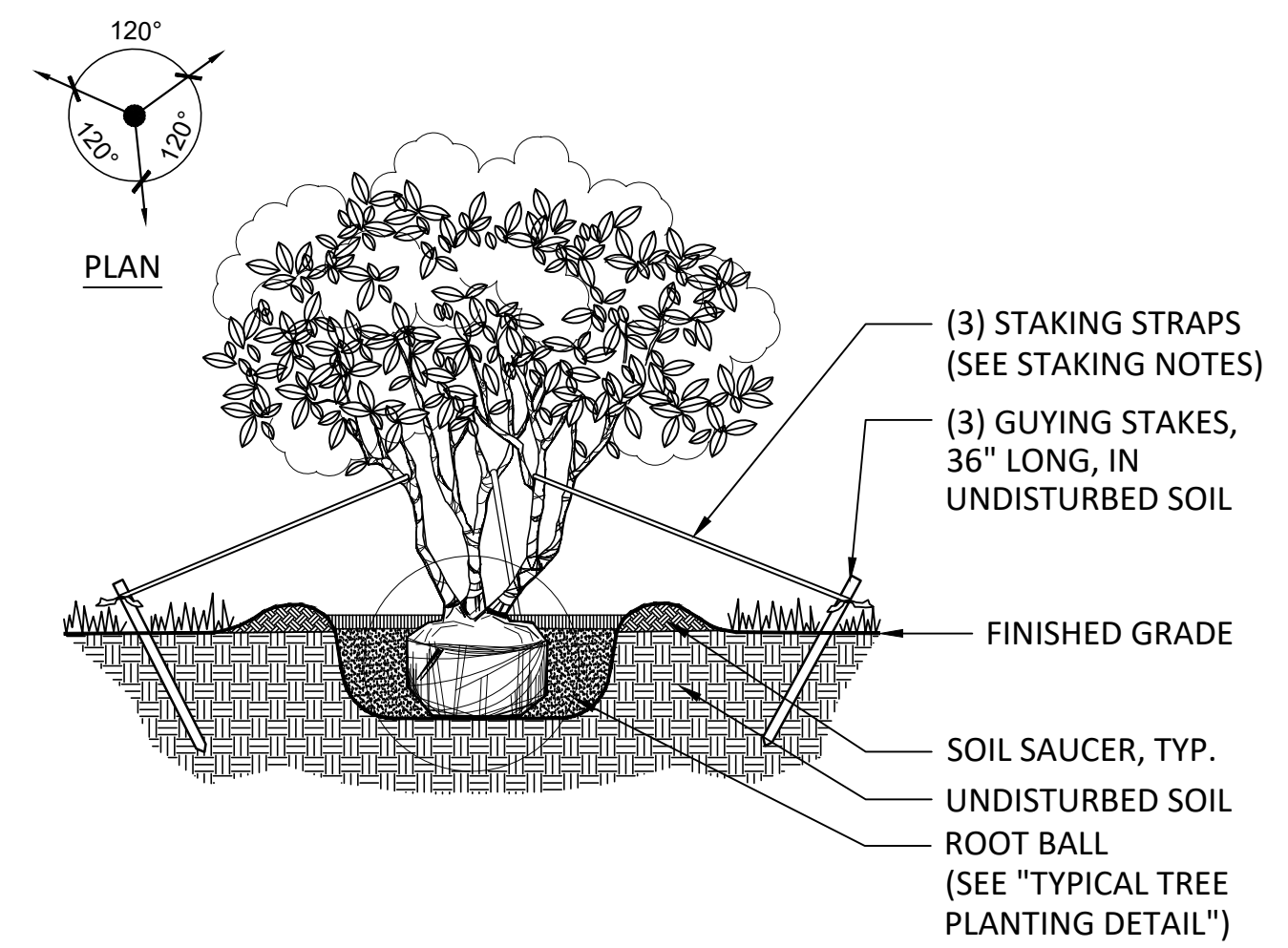
**1** TYPICAL TREE PLANTING DETAIL  
SCALE: NTS



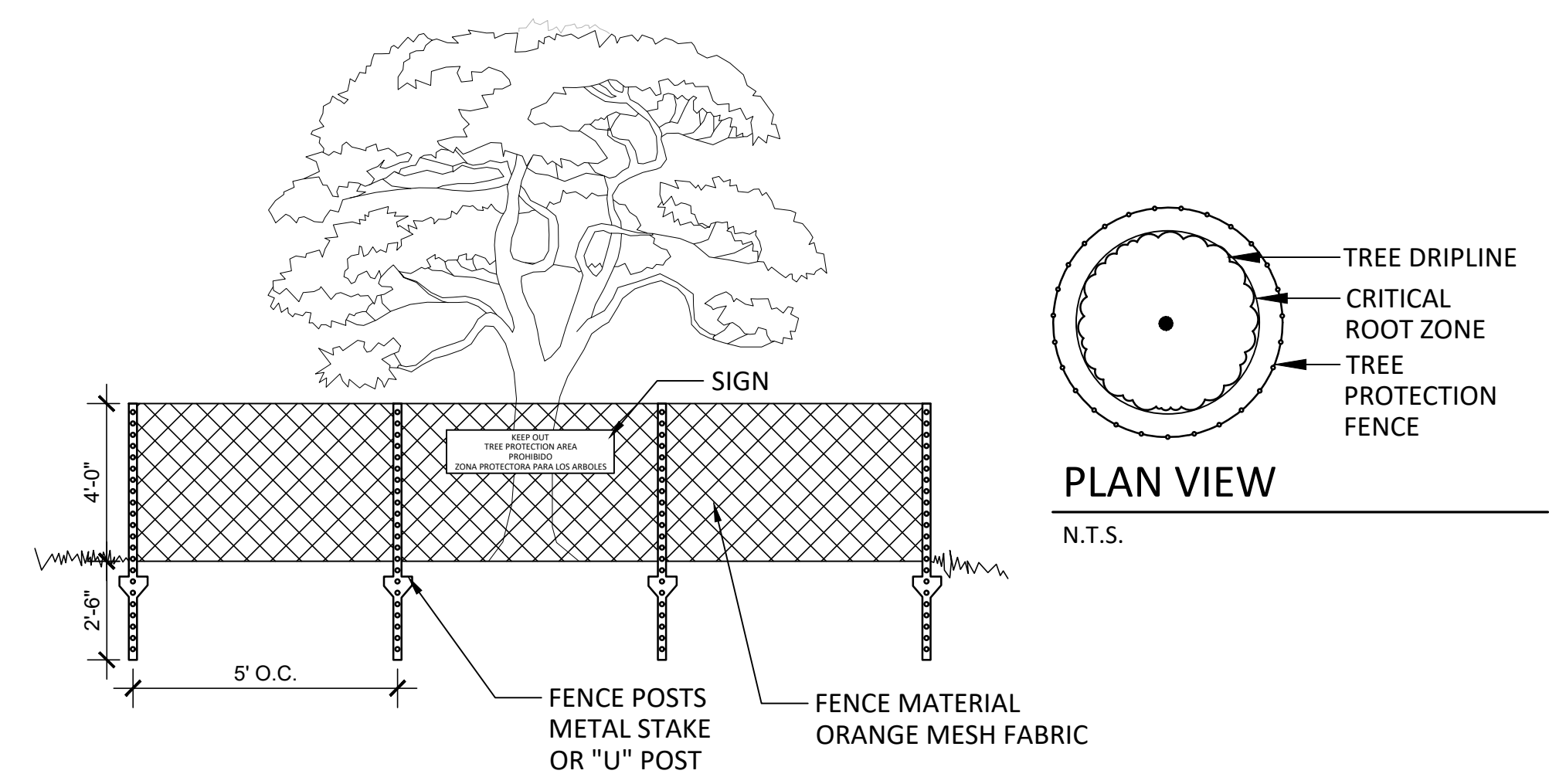
**2** DECIDUOUS TREE STAKING DETAIL (> 2" CALIPER)  
SCALE: NTS



**3** EVERGREEN TREE STAKING DETAIL  
SCALE: NTS



**4** MULTI-TRUNK TREE STAKING DETAIL  
SCALE: NTS



**5** TREE PROTECTION FENCE DETAIL  
SCALE: NTS



Know what's below.  
Call before you dig.

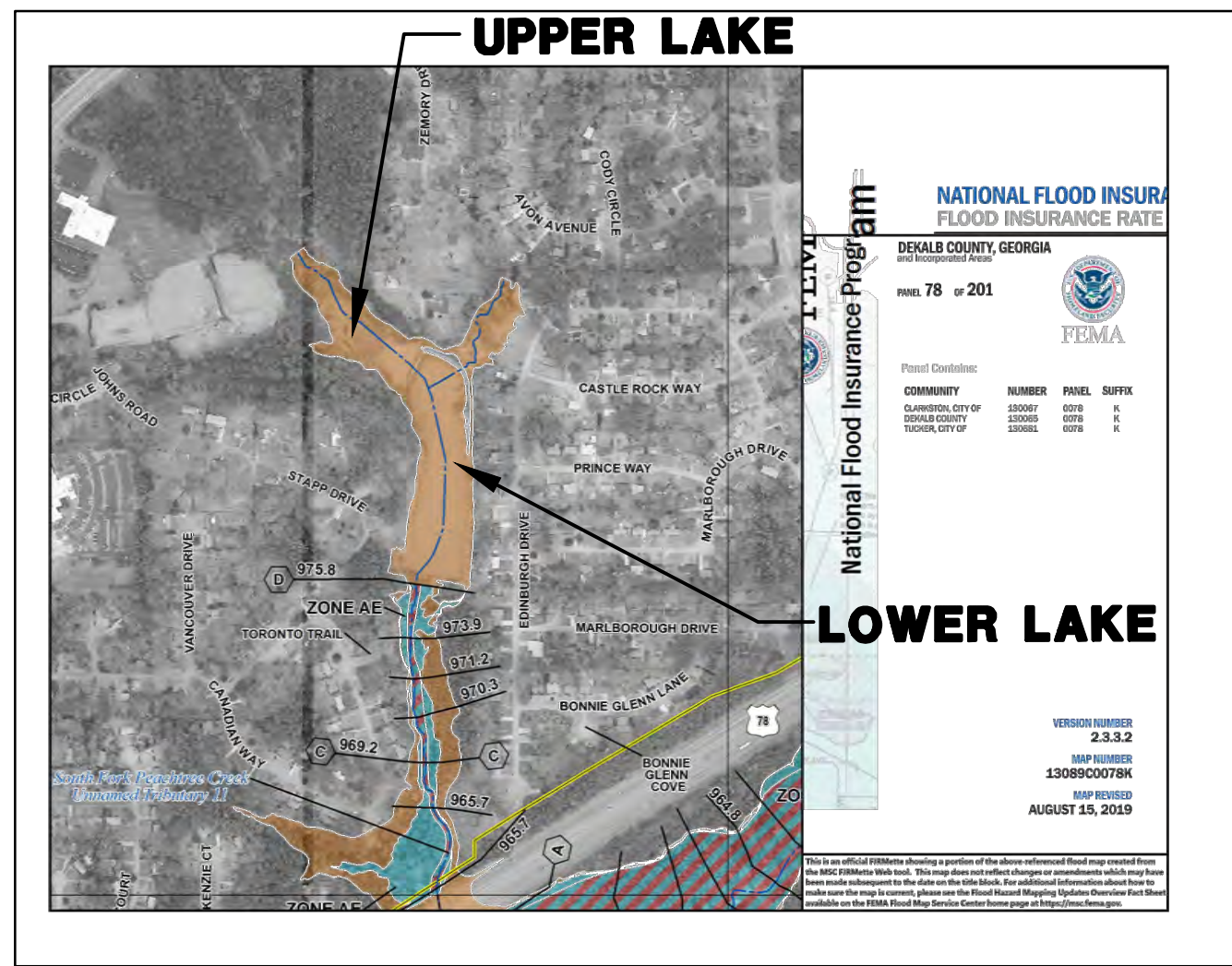
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# JOHNS HOMESTEAD PARK

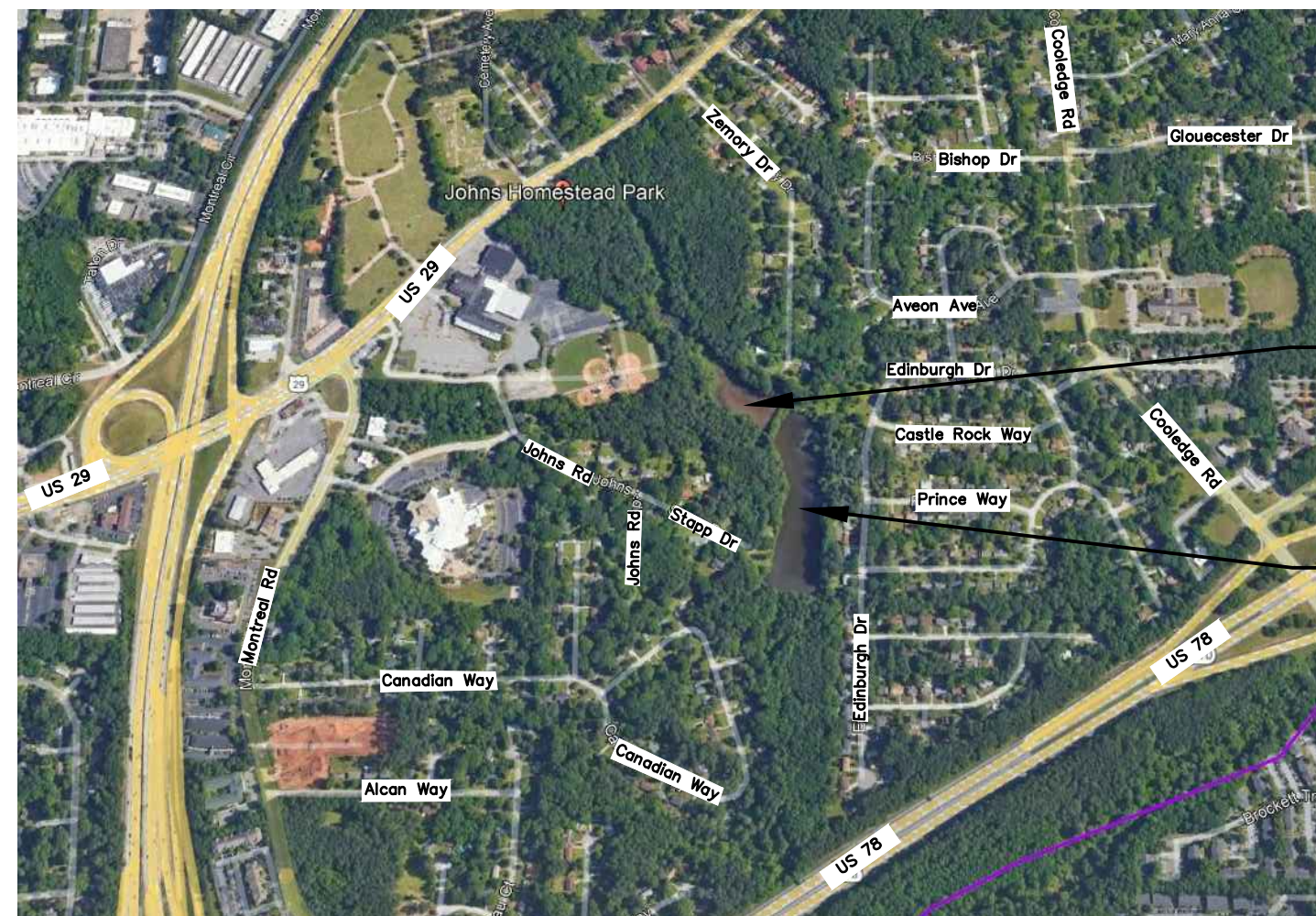
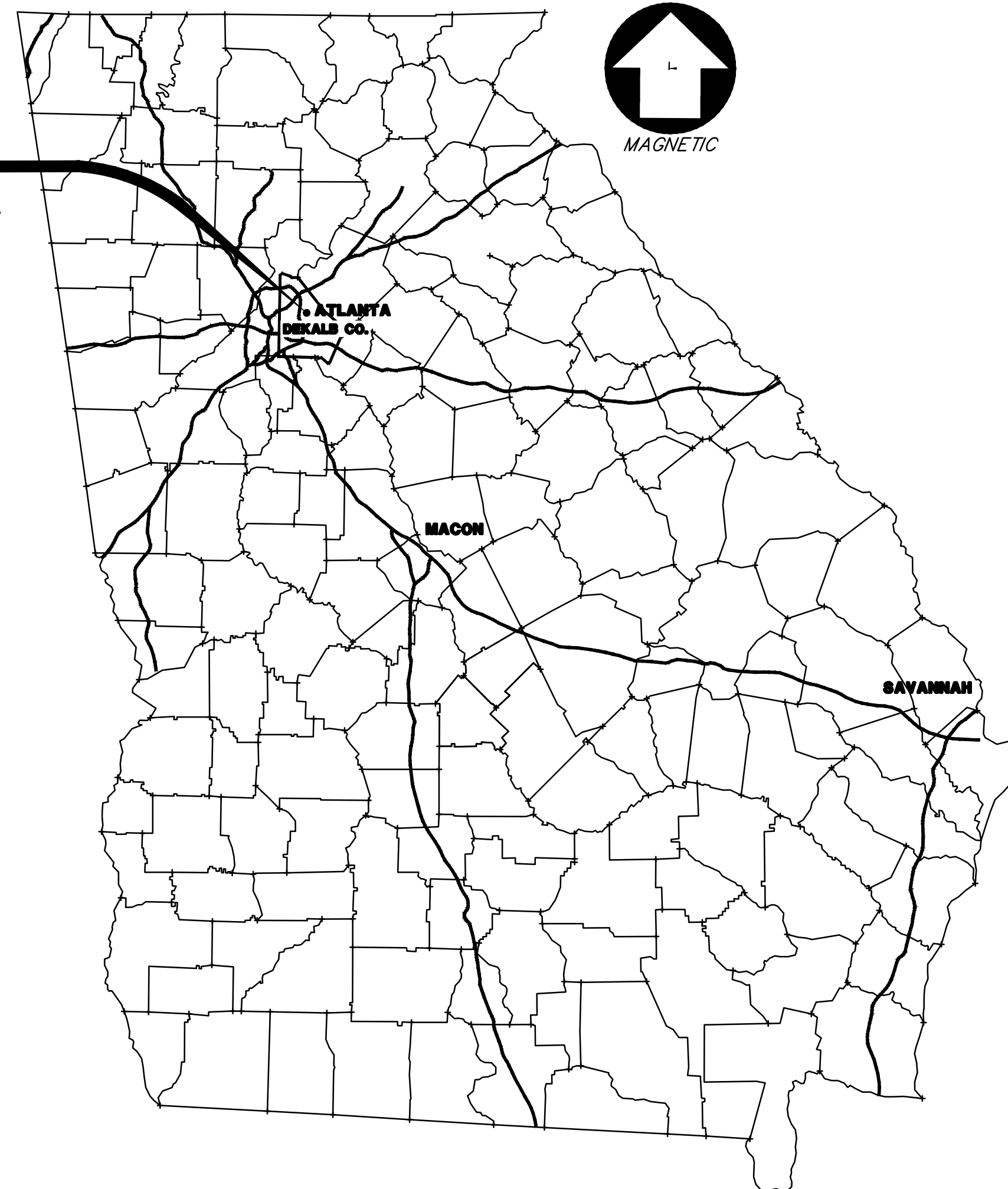
## UPPER AND LOWER DAM REHABILITATION PLANS

### LAND LOTS 143, 144, 165 & 166, OF THE 16TH DISTRICT, CITY OF TUCKER, DEKALB COUNTY



**F.I.R.M. PANEL**  
FIRM MAP NUMBER 13089C0078K  
AUGUST 15, 2019  
SCALE: N.T.S.

**JOHNS HOMESTEAD PARK - LAKES**  
CITY OF TUCKER, DEKALB COUNTY, GEORGIA



**LOCATION MAP**  
SCALE: N.T.S.

#### DRAWING LIST

42016-C-100	SITE WORK	- GENERAL NOTES
42016-C-101A	SITE WORK	- GEOTECHNICAL NOTES 1 of 3
42016-C-101B	SITE WORK	- GEOTECHNICAL NOTES 2 of 3
42016-C-101C	SITE WORK	- GEOTECHNICAL NOTES 2 of 3
42016-C-102	SITE WORK	- RESERVOIR PLAN
42016-C-103A	SITE WORK	- EXISTING CONDITIONS PLAN - LOWER LAKE
42016-C-103B	SITE WORK	- EXISTING CONDITIONS PLAN - UPPER LAKE
42016-C-104	SITE WORK	- BORING PLAN
42016-C-105	SITE WORK	- DEMOLITION PLAN
42016-C-106	SITE WORK	- DAM SITE LAYOUT PLAN
42016-C-107A	SITE WORK	- GRADING PLAN - LOWER LAKE
42016-C-107B	SITE WORK	- GRADING PLAN - UPPER LAKE
42016-C-108	SITE WORK	- SECTIONS AND DETAILS
42016-C-109	SITE WORK	- SECTIONS AND DETAILS
42016-C-110	SITE WORK	- SECTIONS AND DETAILS
42016-C-111	SITE WORK	- SECTIONS AND DETAILS
42016-C-112	SITE WORK	- SECTIONS AND DETAILS
42016-C-112A	SITE WORK	- SANITARY SEWER PROFILE
42016-C-113	SITE WORK	- SECTIONS AND DETAILS
42016-C-200	EROSION CONTROL	- NPDES NOTES
42016-C-201	EROSION CONTROL	- MONITORING NOTES
42016-C-202A	EROSION CONTROL	- INITIAL PLAN
42016-C-202B	EROSION CONTROL	- INITIAL, INTERMEDIATE, FINAL NOTES
42016-C-203	EROSION CONTROL	- INTERMEDIATE PLAN
42016-C-204	EROSION CONTROL	- FINAL PLAN
42016-C-205	EROSION CONTROL	- DETAILS
42016-C-206	EROSION CONTROL	- DETAILS
42016-C-207	EROSION CONTROL	- DETAILS
42016-C-208	EROSION CONTROL	- DETAILS
42016-C-209	EROSION CONTROL	- CHECKLIST
42016-N-300	STRUCTURAL	- UPPER DAM SPILLWAY
42016-N-301	STRUCTURAL	- UPPER DAM SPILLWAY
42016-N-302	STRUCTURAL	- UPPER DAM SPILLWAY
42016-N-303	STRUCTURAL	- LOWER DAM SPILLWAY
42016-N-304	STRUCTURAL	- LOWER DAM SPILLWAY
42016-N-305	STRUCTURAL	- LOWER DAM SPILLWAY
42016-N-306	STRUCTURAL	- DETAILS

PLEASE REFER TO PLANS PREPARED BY ROOT DESIGN STUDIO (404) 606-3654 FOR THE BRIDGES OVER THE SPILLWAYS AND OTHER PARK IMPROVEMENTS ASSOCIATED WITH THIS PROJECT.

**U.S. ARMY C.O.E., SAVANNAH DISTRICT PCN APPROVAL**  
GA PRE-CONSTRUCTION NOTIFICATION, REQUESTING USE OF NATIONWIDE PERMIT No. 3 HAS BEEN ISSUED ON MARCH 1, 2024 FOR THE RENOVATION OF THE TWIN BROTHERS DAMS. THE APPROVED IMPACTS ARE LISTED ON THIS DRAWING. NOTIFICATION OF THE CORPS IN WRITING 10-DAYS BEFORE COMMENCING WORK ACTIVITIES IS A REQUIREMENT. WITHIN 30-DAYS OF COMPLETION, A CERTIFICATE OF COMPETITION MUST BE SENT TO THE CORPS OFFICE. THE VERIFICATION IS VALID UNTIL MARCH 14, 2026 UNLESS IT IS MODIFIED, REISSUED OR REVOKED.

**STATE WATERS BUFFER VARIANCE APPROVAL:**  
GEORGIA D.N.R., E.P.D. REVIEWED AND APPROVED A STATE WATERS BUFFER VARIANCE UNDER O.C.G.A. 12-7-6(b)(15) FOR THE JOHN HOMESTEAD PARK DAM IMPROVEMENTS, DATED: NOV. 15, 2023 WITH THE FOLLOWING CONDITIONS:  
1. ALL GRADED SLOPES 3:1 OR GREATER BE HYDROSEEDING WITH GDOT APPROVED MATTING.  
2. LAND DISTURBANCE BE KEPT TO A MINIMUM.  
3. ALL DISTURBED AREAS MUST BE PROTECTED UNTIL PERMANENT VEGETATION IS ESTABLISHED.  
4. DOUBLE ROWS OF GADOT TYPE C SILT FENCE BE USED IN HIGH PERFORMANCE AREAS AND ADJACENT TO STATE WATERS.  
5. BUFFER VARIANCE BE INCORPORATED INTO L.D.P. BY CITY OF TUCKER.  
6. STRICT ADHERENCE TO THE APPROVED EROSION AND SEDIMENTATION CONTROL PLAN.

BUFFER DISTURBANCE LEGEND		
TYPE OF DISTURBANCE	AREA	LINEAR FEET
BUFFER DISTURBANCE RELATED TO DAM REPAIR	38,208 SF	987 LF
BUFFER DISTURBANCE RELATED TO BUFFER MITIGATION	27,065 SF	1,365 LF
BUFFER DISTURBANCE RELATED TO TRAIL IMPROVEMENTS	8,991 SF	60 LF

**STATE WATERS BUFFER IMPACTS:**  
UPPER LAKE DAM BUFFER: 15,440 SF, 475 LF  
UPPER LAKE (BESIDE) SPILLWAY BUFFER: 1,874 SF, 77 LF  
SIDE CHANNEL SPILLWAY BUFFER: 21,746 SF, 885 LF  
BESIDE LOWER POND BUFFER: 10,541 SF, 425 LF  
LOWER LAKE DAM BUFFER: 22,795 SF, 660 LF  
TOTAL STATE WATER BUFFER IMPACTS: 72,395 SF, 2,522 LF

**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205



**OWNER**  
CITY OF TUCKER  
PARKS AND RECREATION DEPT.  
4898 LAVISTA RD, TUCKER, GA 30084  
CONTACT: RIP ROBERTSON, DIRECTOR  
(470) 481-0205

**PROJECT DESIGN ENGINEER**  
WALDEN, ASHWORTH & ASSOCIATES, INC.  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
(770) 956-7879  
CONTACT: MARTIN L. WALDEN, P.E., EOR

**GEOTECHNICAL CONSULTANT**  
ATLAS CONSULTING, INC.  
P.O. BOX 1997  
ROSWELL, GEORGIA 30077  
(770) 752-9205, EXT. 229  
CONTACT: JONATHAN SHARPE, P.E. EOR



PARK IMPROVEMENTS  
DESIGN CONSULTANT



03/18/2024

**EROSION CONTROL CERTIFICATION**  
JASON RAPPLEAN, PE, EOR  
LEVEL 2 CERTIFICATION # 3031  
EXPIRATION DATE: 12/27/2026  
EMAIL: rapplean@waldenashworth.com

PROPERTY (AREA) = 49.4 ACRES  
DISTURBED AREA = 4.5 ACRES  
NPDES FEES: CITY 4.5 x \$40 = \$180.00  
STATE 4.5 x \$40 = \$180.00

INITIAL DATE 06/03/2024  
REV. DATE 07/11/2024



**WALDEN, ASHWORTH & ASSOCIATES, INC.**

CONSULTING ENGINEERS  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
WA&A JOB NO. 4201600

**Dekalb County Development File#**

SCOPE OF WORK: Renovate the two (2) existing dams that are in poor condition. Replace the existing PVC sanitary sewer line with a DIP line from MH1 to MH.

THE ESCAPE OF SEDIMENT FROM THE SITE SHALL BE PREVENTED BY THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND PRACTICES PRIOR TO LAND-DISTURBING ACTIVITIES.

EROSION CONTROL MEASURES WILL BE MAINTAINED AT ALL TIMES. IF FULL IMPLEMENTATION OF THE APPROVED PLAN DOES NOT PROVIDE EFFECTIVE EROSION CONTROL, ADDITIONAL MEASURES SHALL BE IMPLEMENTED TO CONTROL OR TREAT THE SEDIMENT SOURCE.

ANY DISTURBED AREA LEFT EXPOSED FOR A PERIOD GREATER THAN 14 DAYS SHALL BE STABILIZED WITH MULCH OR TEMPORARY SEEDING.

CONSTRUCTION EXIT TO BE PLACED AT THE FOLLOWING COORDINATES: N 33.0775° E-84.5527°

NOTE: DEKALB COUNTY IS NOT RESPONSIBLE FOR ANY ERRORS OR OMISSIONS OF ENGINEERS OR OTHER DESIGN PROFESSIONALS ON THE DESIGN OR COUNTY CODE REQUIREMENTS FOR THIS PROJECT.



**GENERAL NOTES**

- ALL WORK SHALL COMPLY WITH ALL APPLICABLE STATE, FEDERAL, AND LOCAL CODES. ALL NECESSARY LICENSES AND PERMITS SHALL BE OBTAINED. CONTRACTOR SHALL HAVE ONE COMPLETE SET OF "SAFE DAMS APPROVED" DRAWINGS ON SITE AT ALL TIMES.
- THE CONTRACTOR SHALL NOT DEVIATE FROM THESE PLANS AND SPECIFICATIONS WITHOUT WRITTEN APPROVAL OF THE DESIGN ENGINEER AND THE GEORGIA DEPARTMENT OF NATURAL RESOURCES, SAFE DAMS PROGRAM.
- THE DRAWINGS AND SPECIFICATIONS ARE INTENDED TO COVER A COMPLETE PROJECT, READY TO USE, AND ALL ITEMS NECESSARY FOR A COMPLETE AND WORKABLE JOB SHALL BE FURNISHED AND INSTALLED.
- CONTRACTOR SHALL FURNISH AND MAINTAIN ANY AND ALL NECESSARY BARRICADES AROUND THE WORK AND PROVIDE PROTECTION AGAINST WATER DAMAGE AND SOIL EROSION.
- ALL CONSTRUCTION TO COMPLY WITH THE DEKALB COUNTY AND/OR CITY OF TUCKER STANDARDS.
- CONTRACTOR SHALL VERIFY TOPOGRAPHIC AND UTILITY DATA WITH ACTUAL FIELD CONDITIONS PRIOR TO BEGINNING ANY CONSTRUCTION.
- OWNER:  
CITY OF TUCKER PARKS AND RECREATION  
ATTENTION: RIP ROBERTSON, DIRECTOR  
PHONE NO.: (470) 481-0205
- ALL OPEN DRAINAGE SWALES SHALL BE GRASSED, AND RIP RAP MUST BE PLACED AS DIRECTED BY THE ENGINEER.
- MAXIMUM CUT SLOPE IS 2H:1V. FILL SLOPES OUTSIDE OF THE DAM EMBANKMENT SUCH AS SOIL WASTE AREAS, SHALL BE A MAXIMUM OF 4.0H:1V.
- TOPOGRAPHIC INFORMATION PROVIDED BY: GASKINS-LECRAW, INC. PROJ. NO. 2207051, DATED: 10/03/22.
- CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL UTILITIES AND NOTIFYING THE APPROPRIATE UTILITY COMPANY PRIOR TO BEGINNING CONSTRUCTION.
- CONTRACTOR SHALL VERIFY HORIZONTAL AND VERTICAL LOCATION OF ALL EXISTING STORM SEWER STRUCTURES, PIPES, AND ALL UTILITIES PRIOR TO CONSTRUCTION.
- CONTRACTOR IS RESPONSIBLE FOR REPAIRS OF DAMAGE TO ANY EXISTING IMPROVEMENTS DURING CONSTRUCTION, SUCH AS, BUT NOT LIMITED TO, DRAINAGE, UTILITIES, PAVEMENT, STRIPING, CURBS, ETC. REPAIRS SHALL BE EQUAL TO OR EXCEEDING EXISTING CONDITIONS.
- PROJECT IS LOCATED IN LAND LOTS 143, 144, 165 AND 166 OF THE 16TH DISTRICT, CITY OF TUCKER, DEKALB COUNTY, GA.
- APPROXIMATELY 4.5 ACRES WILL BE DISTURBED AS A PART OF THIS PROJECT.
- THE UPPER DAM AND LOWER DAM'S LENGTH, HEIGHT AND EXISTING CONDITIONS AND PROPOSED CONDITIONS PEAK RUNOFF INFORMATION IS ON DRAWING C-106.
- ALL WETLANDS, BUFFERS AND TREE SAVE AREAS SHALL BE CLEARLY IDENTIFIED WITH PROTECTIVE FENCING BY THE CONTRACTOR PRIOR TO COMMENCEMENT OF ANY LAND DISTURBANCE.
- A 25-FOOT UNDISTURBED BUFFER SHALL BE MAINTAINED ADJACENT TO ALL STREAMS AND BODIES OF WATER EXCEPT WHERE SPECIFIC ENCROACHMENTS ARE INDICATED ON THESE DRAWINGS.
- WETLANDS ARE WITHIN 200 FEET OF THE SITE.
- A SUBCONTRACTOR EXPERIENCED IN THE INSTALLATION OF GEORGIA DOT GUARDRAIL MUST INSTALL THE GUARDRAIL DENOTED ON THE LAYOUT PLAN, DWG C-105, AND ENSURE THAT THE GUARDRAIL MEETS ALL THE GDOT REQUIREMENTS FOR ANCHORAGE, POSTS, HARDWARE, ETC.

**STRUCTURAL NOTES**

- DESIGN IN ACCORDANCE WITH THE 2018 INTERNATIONAL BUILDING CODE WITH GEORGIA AMENDMENTS AND ACI 350-20.
- CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS AND COMPLETE REINFORCING DETAILS TO THE DESIGN ENGINEER FOR REVIEW AND APPROVAL.
- PROVIDE BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES."
- CONCRETE PROTECTION FOR REINFORCEMENT:  
CONCRETE CAST AGAINST EARTH:.....3 INCHES  
CONCRETE EXPOSED TO EARTH OR WEATHER:  
#5 BARS AND SMALLER:.....2 INCHES  
#6 BARS AND LARGER:.....2 INCHES
- ENGINEER WILL APPROVE FOR THE DESIGN BEARING PRESSURE OR MAKE RECOMMENDATIONS AS TO HOW TO PROCEED WITHIN 24 HOURS OF THE CONTRACTOR'S REQUEST. MEANS TO EVALUATE THE BEARING PRESSURE ARE THE RESPONSIBILITY OF THE GEOTECHNICAL ENGINEER.
- LAP ALL BARS IN ACCORDANCE WITH THE LAP SPLICE SCHEDULE UNLESS OTHERWISE NOTED. ALL REINFORCING SHALL CONFORM TO ASTM A615-GRADE 60.
- EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" 45° CHAMFER UNLESS SHOWN OTHERWISE.
- CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR CONCRETE WALL STEEL FABRICATION TO WALDEN, ASHWORTH & ASSOCIATES, INC. FOR REVIEW PRIOR TO CONSTRUCTION.
- NOTIFY THE ENGINEER IN WRITING OF CONDITIONS ENCOUNTERED IN THE FIELD THAT ARE CONTRADICTORY TO THOSE SHOWN ON THE CONTRACT DOCUMENTS.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, FORMWORK, OR TEMPORARY SUPPORTS REQUIRED.
- THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF DIMENSIONS SHOWN ON THE CONTRACT DRAWINGS. NOTIFY THE ENGINEER OF ANY DISCREPANCY BEFORE STARTING SHOP DRAWINGS OR ANY WORK.
- REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR IS ALSO RESPONSIBLE FOR ALL MEANS, METHODS, TECHNIQUES, SEQUENCES, & PROCEDURES OF CONSTRUCTION, AS WELL AS, JOBSITE SAFETY. THE CONTRACTOR SHALL COMPLY WITH ALL FEDERAL, STATE, AND LOCAL REQUIREMENTS AND REGULATIONS.
- INSPECTION OF REINFORCING STEEL IS REQUIRED. CONTRACTOR SHALL NOTIFY DESIGN ENGINEER A MINIMUM OF 24 HOURS PRIOR TO COMPLETION OF STEEL PLACEMENT TO ARRANGE AN INSPECTION. CONCRETE SHALL NOT BE POURED UNTIL STEEL PLACEMENT HAS BEEN INSPECTED AND APPROVED BY THE DESIGN ENGINEER.
- FIELD BENDING OF REINFORCING STEEL WILL BE ALLOWED AS LONG AS ALL BARS ARE BENT AROUND A MINIMUM RADIUS PIN AS REQUIRED IN ACI 318. DO NOT HEAT BARS. BARS MUST BE BENT COLD.
- NO HEAVY COMPACTION EQUIPMENT SHALL BE ALLOWED BEHIND RETAINING WALLS WITHIN A 45 DEGREE LINE FROM THE BOTTOM OF THE WALL OUTWARD TO THE TOP OF THE SOIL.
- TOWED OR SELF-PROPELLED VIBRATING ROLLERS SHALL NOT BE USED WITHIN 5 FEET OF ANY STRUCTURES. BACKHOE-MOUNTED POWER TAMPERS, OR VIBRATING COMPACTORS AND MANUALLY DIRECTED VIBRATING ROLLERS SHALL NOT BE USED WITHIN 3 FEET OF ANY STRUCTURE. ADJACENT TO THE CONCRETE WALLS COMPACTION SHALL BE ACCOMPLISHED BY HAND TAMPING OR MANUALLY DIRECTING POWER TAMPERS, PLATE VIBRATORS, WALK-BEHIND, MINIATURE OR SELF-PROPELLED ROLLERS. LIFT THICKNESS SHALL BE REDUCED TO 4 INCHES WHEN SMALLER COMPACTION EQUIPMENT IS USED. EARTH FILL SHALL NOT BE PLACED BEHIND THE SPILLWAY CHUTE TRAINING WALL OR SAF SIDE WALLS UNTIL THE CONCRETE IN THE STRUCTURE ACHIEVES THE DESIGN COMPRESSIVE STRENGTH OF 4,500 LBS/IN<sup>2</sup> (PSI).

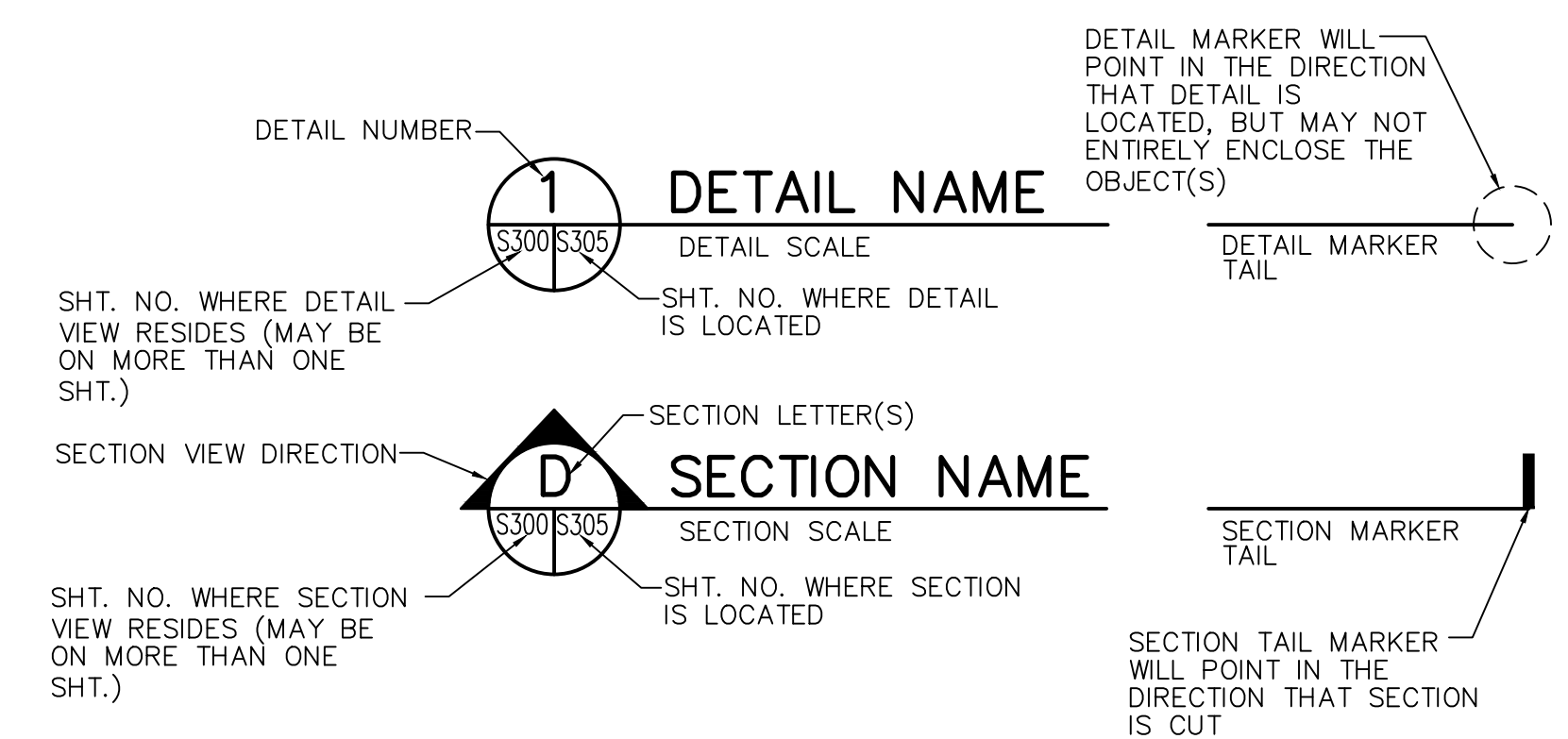
**SPILLWAY BRIDGES - DESIGNED BY OTHERS:**

- THE BRIDGES ARE "GIRDER STYLE" PEDESTRIAN BRIDGES, BY WESTERN WOOD STRUCTURES, (OR AN APPROVED EQUIVALENT). CONTACT JAMIE AGIDIUS AT 503-692-6900 AND/OR AT jamie@wwsi.com FOR BRIDGE PLANS AND DETAILS.
- WOODEN PEDESTRIAN RAILS TO BE 42" MIN. HEIGHT, PER AASHTO WITH A 6-FT CLEAR WIDTH BETWEEN RAILS.
- BEAMS SHALL BE GLULAM, WITH 100% WATERPROOF GLUE.
- SAWN LUMBER SHALL BE DOUGLAS FIR No. 1
- ALL HARDWARE SHALL BE GALVANIZED STEEL.
- GLULAMS PRESSURE TREATMENT WITH .075 CuNAP AND HI-CLEAR II PER AWPA SPECIFICATIONS.
- SHOP DRAWINGS SHALL BE FURNISHED BY THE CONTRACTOR, STAMPED BY A GEORGIA PE WITH ALL NECESSARY DIMENSIONS AND CALLOUTS TO CONSTRUCT BRIDGES.

**LEGEND**

48" EXISTING STORM SEWER	960 EXISTING 10' CONTOUR
48" PROPOSED STORM SEWER	970 EXISTING 2' CONTOUR
WETLANDS	960.5x EXISTING SPOT ELEVATION
RIPRAP AND/OR WAVE PROTECTION	960.5x PROPOSED SPOT ELEVATION
EXISTING CONCRETE	EXISTING TREE LINE
ASPHALT DRIVE (PVMT REPLACEMENT)	EXISTING TREE (HARDWOOD AND PINE)
BUILDING	EXISTING FENCE
GUARDRAIL	EXISTING UNDERGROUND UTILITY LINE (WATER, POWER, GAS, COMM., ETC.)
RETAINING WALL	SAN EXISTING SANITARY SEWER
PERMANENT EASEMENT	SAN PROPOSED SANITARY SEWER
LIMITS OF DISTURBANCE	WATER VALVE AND FIREHYDRANT
R/W RIGHT OF WAY	U/C-U/P PROPOSED UNDERGROUND UTILITY LINE (WATER, POWER, GAS, COMM., ETC.)
STATE WATERS BUFFER LIMITS	EXISTING SLOPE LINE
WATER SURFACE (DATE OF SURVEY)	PROPOSED SLOPE LINE
B-1 SOIL BORING LOCATIONS (S-1 FOR SEWER BORINGS)	EXISTING BENCHMARK
HA-1 HAND AUGER BORING LOCATIONS	

*NOTE: EXISTING FEATURES, SHOWN IN THE ABOVE LEGEND OR NOT, ARE DENOTED WITH DASHED AND/OR SCREENED LINES AND PROPOSED FEATURES ARE DENOTED WITH SOLID AND/OR DARKER LINES. SOME LINES AND TEXT MAY BE THICKER, LIGHTER AND/OR DARKER ON THE FOLLOWING DRAWINGS, AS NECESSARY DEPENDING ON THE PRIMARY PURPOSE OF THE DRAWING.*



**STRUCTURAL DESIGN CRITERIA**

- CODE REFERENCES  
ACI 350-20  
ASCE 7-16  
IBC 2018  
210-VI-NEH
- GRAVITY LOADS  
DEAD LOAD, CONCRETE SELF-WEIGHT 145 PCF  
LIVE LOAD, SURCHARGE 60 PSF (WALKWAYS, ELEVATED PLATFORMS OTHER THAN EXITS)
- SOIL LOADS  
MAXIMUM NET ALLOWABLE BEARING PRESSURE 3,000 PSF  
COEFFICIENT OF FRICTION 0.40  
UNIT WEIGHT OF SOIL 120 PCF  
AT-REST EQUIVALENT FLUID PRESSURE 60 PCF  
PASSIVE EQUIVALENT FLUID PRESSURE 300 PCF
- WIND LOADS  
RISK CATEGORY III  
WINDSPEED 115 MPH  
EXPOSURE CATEGORY B  
VELOCITY PRESSURE 19 PSF  
EXTERNAL PRESSURE COEFFICIENT, WINDWARD 0.8  
EXTERNAL PRESSURE COEFFICIENT, LEeward -0.5  
EXTERNAL PRESSURE COEFFICIENT, SIDEWALL -0.7
- SEISMIC LOADS  
IMPORTANCE FACTOR, Ie 1.25  
SITE CLASSIFICATION D 0.30 g  
0.2-s SPECTRAL RESPONSE ACCELERATION, Sms 0.30 g  
1.0-s SPECTRAL RESPONSE ACCELERATION, Sml 0.21 g  
SHORT PERIOD SPECTRAL RESPONSE PARAMETER, Sds 0.20 g  
LONG PERIOD SPECTRAL RESPONSE PARAMETER, Sdl 0.16 g  
SEISMIC RESPONSE COEFFICIENT, Cs 0.1 g

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**811**  
Know what's below.  
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NO.	REVISIONS	MADE	OKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

**GEORGIA REGISTERED PROFESSIONAL ENGINEER**  
No. 30441  
JASON F. RAPPLEAN  
06/19/2024

SCALE: NONE	DATE
DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE:	12/07/26

**JOHNS HOMESTEAD PARK  
DAM RENOVATION  
GENERAL AND STRUCTURAL NOTES**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
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**GEOTECHNICAL NOTES**

**1.0 SUMMARY**

The following is a summary of our findings and recommendations. The reader is referred to the remaining text of this report for elaboration on these items.

- The lakes should be drained well in advance of beginning repair of the dams. Surface water control will be required to keep the lakes from refilling during the construction.
- The Lower Dam existing principal spillway pipe must be properly abandoned by pressure grouting. If any section extends downstream of the new toe drain that section of pipe should be removed. The other two spillway pipes should be removed. The side channel spillway should be filled structurally along sections 50 feet above and below the dam(s) centerline.
- Substantial undercutting will be required in numerous areas including where the new dam footprint(s) extend into the existing lakes. In particular, the new left arm of the Upper Dam will cross an area currently within the Lower Lake and substantial poor subgrade conditions are expected.
- The slopes should be re-graded to a 3H:1V configuration.
- We recommend installation of a toe drain for the Lower Dam. No toe drain is recommended for the Upper Dam as its downstream slope toe is submerged in the Lower Lake.
- All new structural fill should be compacted to at least 95% of the soil's standard Proctor maximum dry density, with a moisture content at or above the soil's optimum. No borrow area has been identified or tested. It is assumed that no significant amount of the soils excavated for the project will be suitable for use as structural fill.
- Substantial groundwater control (temporary dewatering) will be needed. It may be prudent for the contractor to engage a specialty subcontractor with significant experience in dewatering to assist in this planning and execution.
- Due to the presence of an apparent existing sewer thru its crest, no borings were drilled in the Upper Dam. Thus, the subsurface conditions are unknown. We have made some assumptions based on our experience with earth dams about the conditions likely to be encountered and have made recommendations based, in part, on those assumptions. It should be expected that unanticipated conditions will be discovered which may necessitate changes. As such we recommend our engineers be involved in any such discovered unanticipated conditions and to provide recommendations, we believe are appropriate.
- During construction, all activities should be monitored by personnel experienced in dam design and construction, who will verify that the construction proceeds in accordance with approved plans and specifications, and that any unexpected conditions are properly addressed. For continuity, we recommend engineers from Atlas Technical Consultants, LLC be involved in this important role to maintain continuity between the design and construction. With existing dams with no record of design or construction, unexpected conditions often arise.

**2.0 GOVERNING RULES**

See Atlas Report for Safe Dams Act information and Georgia Safe Dams Act information. This dam is currently listed as an "exempt" structure on their Dams Inventory list, and their report and this design is based on this classification.

**3.0 PROPOSED CONSTRUCTION**

See Atlas Report for information regarding proposed construction, these plans are intended to cover that scope.

**3.1 UPPER DAM**

Modification of the Upper Dam will include widening the crest, flattening the existing slopes, and turning/extending the left side of the dam 90 degrees downstream. That new left arm of the dam will be about 270 feet long, will extend across a shallow portion of the Lower Lake and will terminate across the current Side Channel Spillway. The new dam section (Left Arm) crest will be eight feet above the Lower Pond's normal pool elevation. The modified dam will be about 10-feet tall at its tallest section. The new top of dam and normal pool will be at elevations of 992.0 feet and 987.0 feet, respectively.

The new principal spillway will be 28 feet wide, have concrete training/side walls and wing walls both upstream and downstream with the wing walls extending into the lake(s). The control section will be a weir wall. The bottom of the base slab will be at an elevation of 982.0 feet, 10-feet below the top of dam, will have cutoff walls at the upstream and downstream ends and will be underlain by a drainage layer.

The top of the base slab will be at elevation of 984.0 feet, which is one (1) foot below the Lower Lake normal pool elevation. Thus, the base slab will be submerged when the Lower Lake is at normal pool. There will be a 5-foot-wide pedestrian bridge across this spillway, supported directly on the spillway side walls. There will be no other "emergency" spillway.

**3.2 LOWER DAM**

We understand that modifications to the Lower Dam will include shifting the centerline downstream, flattening both upstream and downstream slopes, abandonment/removal of the existing spillways and construction of a new labyrinth principal spillway. The modified dam will be 15 or so feet tall. The new top of dam and normal pool will be at elevations of 988.0 feet and 985.0 feet, respectively. The Lower Dam will be extended to its left and cross the existing Side Channel Spillway.

The new labyrinth principal spillway will be about 25 feet wide, have concrete training/side walls and wing walls both upstream and downstream with the upstream wing wall extending into the Upper Lake. The control section will be a labyrinth wall with a top elevation of about 985 feet. The base section will have cutoff walls at the upstream and downstream ends and will be underlain by a drainage layer. The top of the base slab will be at an elevation of 976.0 feet, creating nine (9) foot tall labyrinth wall(s). There will be no other "emergency" spillway.

**3.3 SIDE CHANNEL SPILLWAY**

Our work did not include any investigation of the Side Channel Spillway and the following is provided for reference only as its abandonment impacts the dam(s) hydrology and hydraulics.

We understand the Side Channel Spillway will be filled in completely along its entire length. The channel carries constant flow from the Upper Pond. It varies in width, depth and cross section.

**4.0 METHODS OF EXPLORATION**

Our work included a visual reconnaissance of the dams, drilling soil test borings on the crest of the Lower Dam and hand auger borings below the Lower Dam. However, there is reportedly a sewer line under the crest of the Upper Dam which could not be located. Therefore, the soil test borings on the Upper Dam were eliminated. Our work did not include an evaluation of the dam's spillway capacity nor any hydrology and hydraulics (H&H) work. All references looking left and right in the remainder of this report are made as if facing downstream.

The subsurface conditions of the Lower Dam were explored by drilling four soil test borings (L-1 thru L-4) to depths of 30 feet below the existing grade. The borings were located in the field by measuring distances and estimating directions from identifiable site. Therefore, their locations as shown on the Site and Boring Location Plan in the Appendix should be considered approximate.

The soil borings were advanced by twisting continuous hollow stem auger flights into the ground. At selected intervals, Standard Penetration Resistance Testing (SPT) was performed in general accordance with ASTM Standard D-1586, and soil samples were collected for visual classification. The results of the penetration tests, when properly evaluated, provide an indication of the relative consistency of the soil being sampled, the potential for difficult excavation, and the soil's ability to support loads. A more detailed description of the drilling and sampling process is included in the Appendix of Atlas' report.

Soil samples recovered during the drilling process were returned to the office where they were classified in general accordance with the Unified Soil Classification System (USCS). Detailed descriptions of the materials encountered at each boring location, along with a graphical representation of the Standard Penetration Test results, are shown on the Soil Boring Records in the Appendix. Elevations on the Soil Boring Records were interpolated from the topographical contours on the plan provided to us and should be considered approximate. Following completion of drilling the boreholes were filled with bentonite pellets and the top was plugged with concrete.

Four hand auger borings (HA-1 thru HA-4) were drilled just beyond the downstream toe of the Lower Dam to depths ranging from 3.5 feet to 4 feet. The hand auger borings were performed by manually rotating a sharpened steel bucket auger into the ground. The soils encountered during the auguring process were classified in general accordance with the Unified Soil Classification System (USCS) by our engineer. At selected intervals, the auger was removed, and the soil consistency was measured with a portable Dynamic Cone Penetrometer (DCP). The conical point is first seated 2 inches to penetrate any loose cuttings and then driven an additional 1-3/4 inches with blows of a 15-pound hammer falling 20 inches. The number of hammer blows required to achieve this penetration is recorded as an index to the soil's strength. Please refer to the Summary of Hand Auger Borings in the Appendix of Atlas' report.

**5.0 OBSERVATIONS**

On March 9, 2023, Atlas engineer W. Michael Ballard, P.E., a Georgia Safe Dams Engineer of Record, visited the site to perform a visual inspection of the embankments. The weather was clear and site conditions were dry on this day and the previous several days. The following items were noted in no order of importance:

**5.1 UPPER DAM**

- The Upper Dam is estimated to be about 10 feet tall and 200 feet long.
- The crest of the dam was straight and 8 to 10 or so feet wide. The width is irregular due to erosion on both slopes. The crest was covered with crushed stone and is a walking path. Small trees and brush were on the crest edges. The crest of the dam was about 5 or so feet above the Upper Lake current pool level.
- The downstream slope is very steep. The toe of the downstream slope is submerged below the surface of the Upper Lake. The slope is heavily wooded with small trees and brush. There was no sign of seepage on the slope itself but there is so much vegetation to both absorb a significant amount of seepage and to obscure evidence of that seepage.
- The portion of the upstream slope visible above the lake level has an irregular and very steep configuration caused by erosion and perhaps overtopping. As with the downstream slope, the upstream slope is vegetated with small trees and brush.
- The dam has two visible spillways which include a CMP pipe in the right abutment and a side channel spillway in the left abutment. The side channel spillway has no defined control section, is unvegetated and was flowing water during this inspection. It is not known if there is an additional low-level pipe which previously served as an additional spillway or was used in the original construction and, if so, it is no longer visible. It is unknown but doubtful that the CMP spillway pipe carries unimpeded flows when the lake is higher.
- We observed no toe drains or instrumentation (monitoring wells, etc.) and none was expected.
- The new Upper Dam configuration will cause the left end of the dam to be turned 90 degrees downstream and will be extended another 270 or so feet across a portion of the Upper Lake. This area was under water of the Upper Lake at the time of our visit. We assume that shallow water conditions exist, and that the subgrade will consist of soft soils and possibly washed in sediments.

**5.2 LOWER DAM**

- The Lower Dam is estimated to be about 15 feet tall and 250 feet long.
- The crest of the dam is straight and 10 to 12 or so feet wide. The crest was covered with a combination of crushed stone, exposed earth and some vegetation and is a walking path. Large trees were on the downstream crest edge. The elevation of the crest of the dam was about 2 to 3-feet above the current lake level.
- The downstream slope is very steep. The slope is heavily wooded with mature trees and underbrush. There was no sign of seepage on the slope itself but there is so much vegetation to both absorb a significant amount of seepage and to obscure evidence of that seepage.
- A portion of the upstream slope visible above the lake level is faced with concrete for most of its width. The facing does not extend to the left abutment.
- The dam has two visible spillways which include a low level/principal spillway to the left of the right abutment and a smaller pipe placed at or above normal pool in the right abutment. The side channel spillway extends around the dam thru the left abutment.
- The existing low level/principal spillway is a 12-inch diameter corrugated metal pipe (CMP) placed near the base of the dam, connected to a CMP Riser. Part of the CMP riser is visible and has rusted badly. We do not know if the top of the riser is at or below the original normal pool. The outlet end of the low-level CMP is exposed at the base of the dam, close to the right abutment. The end is badly corroded. The pipe was carrying flow at the time of my inspection. The amount of outflow was visually similar to the amount of in-flow in the top of the riser. Spillway pipe flows are released on the ground and not into a protected plunge pool.

- There is a secondary spillway consisting of a 12-inch diameter reinforced concrete pipe (RCP) in the right abutment. At the time of my visit, the lake was about 12 to 18 inches below the pipe invert.

The side channel spillway goes around the left side of the dam and has no defined control section, is unvegetated and was flowing water during this inspection.

- We observed no toe drains or instrumentation (monitoring wells, etc.) and none was expected.

**5.3 SIDE CHANNEL SPILLWAY**

- The side channel spillway begins at the left side of the Upper Dam and extends around the left side of the Lower Lake, passing around the left side of the Lower Dam. It has an irregular and varied cross section and depth. In places the channel is very narrow. There are numerous large trees immediately adjacent to the channel top(s).
- The spillway carried all surface water around the Upper and Lower Dams. At the time of my inspection and there was a considerable amount of flowing water.
- In places where I could observe, the bottom of the channel appears to have eroded into firm residuum.

**6.0 GEOLOGY AND SUBSURFACE CONDITIONS**

**6.1 GEOLOGY**

The site is located in the Piedmont Physiographic Province of Georgia. The residual soils in the Piedmont are the result of the chemical and physical weathering of the underlying parent rock. The weathering profile usually results in fine grained clayey silts and silty clays near the surface, where weathering is more advanced. With depth, sandy silts and silty sands are found, often containing mica. Below the residual soils, partially weathered rock is often found as a transition above relatively unweathered rock. In local practice, partially weathered rock is arbitrarily defined as residual soils with Standard Penetration Resistances in excess of 100 blows per foot (50 blows per 6 inches), and which can be penetrated by a power auger.

**6.2 SUBSURFACE CONDITIONS (LOWER DAM)**

No drilling was performed for the Upper Dam due to an underground utility that could not be located.

**6.2.1 SUBSURFACE CONDITIONS**

The surface of the crest is mostly a walking path, and no topsoil was encountered at the boring locations. The hand auger borings performed just downstream encountered 3 to 4 inches of topsoil.

**6.2.2 PREVIOUSLY PLACED FILL**

Fill soils are those soils that have been placed or reworked in conjunction with past construction activities, grading, or farming. All four borings encountered fill to 13 feet. The fill was typically classified as firm to stiff slightly sandy clayey silt (MH-ML), with Standard Penetration Test (SPT) results ranging from 5 to 9 blows per foot (bpf). Based on the SPT results, the soil represented by these samples would be considered lightly to moderately compacted.

In the hand auger borings performed just downstream of the toe, borings HA-2, HA-3 and HA-4 encountered fill to depths of 8 inches at almost 2 feet.

**6.2.3 ALLUVIUM**

Alluvium is soil that has been transported and deposited by moving water. All four soil test borings encountered alluvium immediately below the fill and the alluvium extended from depths of 13 to around 16 or 17 feet below the crest. The alluvium was classified as loose to medium dense slightly silty sand. Standard Penetration Test (SPT) results ranged from 7 to 21 blows per foot (bpf).

All four hand auger borings drilled just downstream of the dam toe encountered alluvium either immediately below the topsoil or below shallow surface fills. The topsoil extended to depths of 3.5 to 4 feet where each boring encountered hand auger refusal. Our engineer stated the hand auger refusals were likely caused by gravels which are typically found near the bottom of the alluvium. The hand auger borings could not penetrate the gravels.

**6.2.4 RESIDUUM**

Residuum, formed by in-place weathering of the parent rock, was encountered below the alluvium in each soil test boring. The residuum was classified as medium dense to dense silty sands and was of moderate consistency. Standard Penetration Test results ranged from 15 to in excess of 40 blows per foot.

**6.2.5 PARTIALLY WEATHERED ROCK**

Partially weathered rock (PWR) is a transitional material between soil and rock, which retains the relic structure of the rock and has very hard or very dense consistencies. Partially weathered rock was encountered in borings L-1 and L-3 beginning at 28 feet below the dam crest. The material was classified as silty medium to fine sand. All four borings were terminated at 30 feet without encountering drilling refusal (rock).

**CONTINUED ON DWG. C-101B**

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NO.	REVISIONS	MADE	OKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS



SCALE: NONE	DATE
DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	3031
LEVEL 2 CERTIFICATION #	12/07/26
EXPIRATION DATE:	

**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**GEOTECHNICAL NOTES**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**

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4201600 **42016- C-101A** **0**



**GEOTECHNICAL NOTES**  
**CONTINUED FROM DWG. C-101A**

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**6.2.6 GROUND WATER**

Groundwater was measured at the time of drilling at depths which varied from 11 to 14 feet below the dam crest. For safety, the boreholes were backfilled immediately following completion of drilling and no stabilized groundwater levels were measured. Those would be somewhat higher.

The hand auger borings did not encounter groundwater to their refusal depths of 3.5 to 4 feet, as measured at the time of drilling.

Groundwater fluctuations of 5 feet or more are common in this geology.

The conditions described in the preceding paragraphs, and those shown in the Appendix, have been based on interpolation of the results of the previously described data using generally accepted principles and practices of geotechnical engineering. However, conditions in this geology may vary intermediate of the tested locations, and even more so on previously developed property.

Although individual soil test and hand auger borings are representative of the subsurface conditions at the precise boring locations on the day drilled, they are not necessarily indicative of the subsurface conditions at other locations or other times. The nature and extent of variation between the borings may not become evident until the course of construction. If such variations are then noted, it will be necessary to reevaluate the recommendations of this report after on site observation of the conditions.

**7.0 CONCLUSIONS AND RECOMMENDATIONS**

The following conclusions and recommendations are based on the data gathered during this exploration, our understanding of the proposed construction, our experience with similar site and subsurface conditions and generally accepted principles and practices of geotechnical engineering. Should the proposed construction change significantly from that described in this report, we request that we be advised so that we may amend these recommendations accordingly. This report, and the conclusions and recommendations provided herein, are provided exclusively for the use Root Design Studio and their consultants and is intended solely for design of the referenced project.

**7.1 GENERAL ASSESSMENT**

Following completion of the proposed modifications, the deficiencies which currently exist will no longer pose a hazard to the long-term performance of the dams.

**7.2 SURFACE WATER CONTROL**

As an initial step in the construction process, the lakes should be drained. This should be performed as early in the construction process as is practical (a minimum of 30 days prior to beginning clearing is recommended) to facilitate drying of surface soils and some lessening of the difficulty of temporary dewatering and subgrade preparation. Surface water control must be implemented prevent the lakes from re-filling during the construction. The contract documents should clearly indicate that the contractor is responsible for the care and diversion of water during the construction. The contractor should be required to submit plans for surface water control and obtain approval from the engineers prior to implementation in the field.

**7.3 TEMPORARY GROUNDWATER CONTROL**

Significant temporary groundwater control is necessary and critical during various parts of the construction. Groundwater will likely impact subgrade preparation and other activities that will take place in the lower topographic areas. These activities will include, but are not limited to, undercutting and backfilling of undercut areas, drain construction and construction of any required plunge pools, stilling basins, principal spillway structures or foundations associated with the outlet structures and wing walls. In particular significant temporary groundwater control will be needed in the undercutting of subgrades in preparation for filling for the new "left arm" of the Upper Dam where it extends from the existing dam and crosses thru an area impounded by the Upper Lake. Similarly, where the existing slopes of both dams are extended into the lake(s), significant undercutting and associated significant temporary groundwater control will be needed and critical.

The dewatering system implemented should function continuously 24 hours a day, 7 days a week until the excavations are properly backfilled, or structures are placed to at least 2 to 3 feet above the ambient stabilized groundwater levels. We recommend that the contract documents clearly indicate that the design and implementation of the dewatering system is the contractor's responsibility, and that these documents establish performance criteria for assessing the effectiveness of the dewatering system actually installed. The performance criteria should require that the dewatering system successfully lower the prevailing groundwater levels at least three feet below the lowest anticipated subgrade (undercut) levels in advance of any excavation. This is typically confirmed by shallow observation wells scattered around the area where groundwater is anticipated to be problematic. The project specifications should require that the contractor submit a detailed dewatering plan for the engineers' review prior to implementation. These plans should be provided early in the overall construction process to allow adequate time for review, re-submittals if necessary, and implementation of the plans in a timely fashion so as not to impact the contractor's schedule. Any dewatering system implemented must be properly abandoned.

With the magnitude of temporary groundwater control likely to be needed for several parts of this project, particularly undercutting in areas currently within the lake(s) footprints it may be prudent for the contractor to engage a subcontractor with significant experience in groundwater control.

**7.4 SITE AND SUBGRADE PREPARATION**

All the unsuitable vegetation and trees should be removed from the existing embankments. The initial removal process should be thorough and should remove all the major root systems. This may require the removal of a considerable quantity of material from the outer slopes of the dam. Draining the lake(s) could cause failures of the upstream slope(s) due to rapid drawdown. Any soils on the slope(s) which move, or slide should be excavated and replaced.

After clearing and grubbing, it will be necessary to undercut any unsuitable soils in the area(s) which is to be covered by new fills, spillways, wing walls and other structural items. All alluvium should be removed in areas to be covered by new fills. Our borings indicate the alluvium downstream of the Lower Dam will extend to depths of about 4 to 5 or so feet across the entire dam footprint. The depth of alluvium under the Lower Dam crest and under the footprint of the Labyrinth Spillway is shown on the boring logs. We have no information regarding the depth of alluvium in other areas to include upstream/downstream of the Upper Dam and under the new Left Arm of the Upper Dam. Temporary dewatering will be needed to perform most if not all the undercutting.

For flattening of slopes, the upstream and downstream limits of the undercutting should extend at least to where the projected 3(H):1(V) slope contacts the residual subgrade (under the alluvium or lake sediments). The temporary downstream and upstream edges of the undercutting beginning at the toe of the existing dam(s) should be created no steeper 1.5(H):1(V) so as to not undermine those slopes.

**7.4 cont.**

The new left arm of the Upper Dam will be extended about 270 feet across a section of the Lower Lake which is currently under water. Those subgrades will be problematic. The area was under water and inaccessible during our work, so no investigation of those subgrades was undertaken. It is reasonable to assume the subgrades will include saturated soils and sediments. We expect the area will not be accessible for some time after the surface water of the Upper Lake is removed. After the surface soils and sediments have dried somewhat the contractor may have to use low ground pressure or other specialty equipment or process to remove the soils judged unsuitable or which would impair compaction of the to be placed fills as recommended by the geotechnical engineer. The average and maximum depth of undercutting is unknown. We expect groundwater to be shallow and temporary dewatering will be needed. Rock may not be used to stabilize subgrades.

The side channel spillway was carrying significant surface water at the time of our work. Where we were able to observe the spillway bottom, the surface water had scoured away sediments and the bottom of the spillway was observed to be firm residual soils. While it will be necessary to shape the spillways to be able to backfill adequately, no substantial excavation of unsuitable soils are expected, although pockets may exist.

Rock may not be placed to stabilize subgrades or for any backfill purposes unless approved by the geotechnical engineer. All areas that are undercut should be replaced with the lower permeability soils (select fill) which are helpful to aid in the reduction of seepage. These select backfills include CL, ML, MH and SC soils. Any excavation into the dam(s) itself should be backfilled with select fills.

All undercutting and subgrade preparation should be performed under the full-time observation of a geotechnical engineer experienced in dam design and construction. This individual can provide field recommendations as required to deal with specific areas. All subgrades should be approved by the geotechnical engineer prior to initiating the next phase of work. Depending on the dewatering methods selected by the contractor, it may be necessary to prepare and approve small areas and allow initial fill placement to begin prior to preparing the next adjacent area(s).

**7.5 SLOPE CONFIGURATION**

Our extensive experience in Georgia indicates that dams constructed with appropriate consideration for seepage and built at no steeper than a 3H:1V configuration are stable under all design scenarios. No detailed stability analysis was performed in this scope of work and none is thought necessary.

**7.6 ABANDONMENT OF EXISTING SPILLWAY PIPE(S) AND SIDE CHANNEL SPILLWAY**

We recommend the existing low-level spillway pipe of the Lower Dam be abandoned in place utilizing pressure grouting techniques in lieu of excavation and removal, if possible. However, it is possible that the undercutting recommended for the Lower Dam Spillway (see report section 7.9) will get close to the existing pipe and, if so, it may be cost effective to remove the pipe along with the adjacent undercutting. This decision which should take into account that the alluvium probably to be found under the removed pipe would impair backfill compaction and thus should be removed.

Grouting would require that both ends of the pipe be exposed, the interior pressure washed to remove any sediments and other infill materials, and verification that the pipe is reasonably intact. If these conditions are satisfied, our experience would indicate that it would likely be more cost effective to grout the pipe in place, rather than to utilize direct removal. Where both ends of the conduit are exposed, cement grout is typically pumped through a bulkhead from the downstream end and allowed to migrate the entire length of the pipe to the upstream end, which is also either bulk headed or utilizes the existing valve, if in place, to contain the flow. If a bulkhead is used, temporary venting will be required. If the existing valve can be utilized, it should be possible to leave the valve partially open until the grout flow is observed, and then to shut the valve. Once clean grout is seen from the vent at the upstream end, the vent is closed, and additional pressure applied to the grout to help assure that any voids around the conduit caused by openings in the pipe are filled. The downstream end of this pipe, downstream of the toe drain, should be removed and a sand blanket built as described in report section 7.7 Seepage Control. If it is determined that the low-level pipe is not reasonably intact, and that grouting would likely not be sufficient, complete removal would be necessary. If removal is required this would cause the embankment to be breached, the pipe removed, and the breach backfilled with "select" fill. The breach width at the pipe elevation would need to be wide enough to operate compaction equipment and the side slopes of the breach would be required to be sloped up to the crest no steeper than a 1.5(H):1(V) configuration. Remediation of the subgrade is likely to be required to place fill, undercutting would be necessary, thus deepening and subsequently widening the breach. Thus, removal of the pipe would result in a significant portion of the embankment being removed.

The smaller second spillway pipe for the Lower Dam (12-inch RCP) and the CMP Spillway Pipe for the Upper Dam should be removed, and the excavations backfilled with select fill.

A portion of the side channel spillway should be filled for dam safety. The fill in the side channel from 50 feet upstream of the dam(s) centerline to 50 feet downstream of the dam(s) centerline should be treated as structural. The surface water must be stopped, and temporary groundwater control implemented as discussed in appropriate report sections. These length(s) of channel should have the side slopes flattened as described on plan sheets C-109 thru C-112. The bottom will probably need to be widened to allow operation of compaction equipment. Approved select fill should be placed and compacted as described in the appropriate report sections. The new fill should be benched into the sides of the channel, as shown on those plan sheets. In our opinion, the lengths of the channel beyond those described in this paragraph may be treated as landscape fills, as far as dam safety is concerned.

**7.7 SEEPAGE CONTROL**

Seepage through and beneath an earthen dam is expected. However, it should be controlled to prevent high exit gradients which may cause the possible loss of soil fines and ultimately "piping" beneath or through the dam. There is no evidence of any internal drainage system in the existing dams.

The internal drain system recommended for the Lower Dam includes a combination toe/foundation drain placed generally parallel to the downstream toe of the existing embankment. The downstream toe of the Upper Dam is submerged by the Lower Lake and no toe drain is recommended for the Upper Dam.

The toe/foundation drain will be covered by the fill placed to flatten the downstream slope. Remedial subgrade preparation (undercutting of alluvium) is required and will likely be completed either prior to or in conjunction with constructing portions of this recommended drainage system. Once the toe/foundation drain is completed, additional fill materials will be placed to cover the drain system and create the new downstream toe configuration recommended.

**7.7 cont.**

The toe drain portion of the system would consist of a nominal four feet high by two feet wide trench drain filled with "washed" GDOT No. 57 stone. The filter system for this drain will consist of a suitable geotextile filter fabric placed between the soil and gravel. Embedded within this toe drain should be a six-inch diameter, schedule 40, perforated PVC pipe placed with an invert approximately 12 inches above the base of the toe drain section. The toe drain system should extend along the downstream toe of the existing dam from abutment to abutment, but not across the left residual soil abutment toward the Side Channel Spillway.

The toe drain alignment will follow the toe of the existing dam. However, from a practical construction standpoint it is difficult for excavating equipment to operate at the toe of slope without benching into the existing downstream toe. Therefore, it may be more practical to provide an approximately 5-foot minimum horizontal offset to the drain alignment from the existing toe so that it can be excavated without need for cutting into the existing embankment.

Where existing fill or new backfill materials are placed (primarily areas where alluvium is undercut), it is recommended for a "foundation drain" extension to be placed below the toe drain to penetrate these fill materials and extend at least one foot into the underlying residual soil below. The "foundation" part of the drain would consist of ASTM C-33 concrete sand. The foundation drain will predominantly be along the undercut area required at the downstream toe of the Lower Dam. Where the foundation drain is required, i.e., where the toe drain itself does not penetrate into residual soils, the filter fabric wrapped toe drain should be constructed with the filter fabric draped into both sides of the trench in an "open bottom" configuration prior to placement of the foundation drain sand. The filter fabric should be embedded at least 12 inches into the upper portion of the foundation drain excavation.

Solid outlet pipes should be provided for the toe drain collection system. Two outlets should be sufficient for this dam. All perforated drainpipes and outlet pipes should provide for at least a minimal amount of slope for proper drainage. In lieu of traditional cleanouts, we recommend maximum 22.5-degree bends to be used at all turns and elevation changes so that the entire pipe system could be cleaned from the downstream end, if needed in the future. Small animal guards and headwalls should be included at the outlet ends at all discharge pipes. The outlet structures should be constructed so that flows from the pipes can be collected and monitored. Positive drainage away from these headwall structures should be provided. We recommend that headwall construction be such that a minimum 12-inch drop below the pipe can be maintained. The outlet pipe should extend approximately six inches beyond any headwall surface, primarily to accommodate the small animal guards.

The filter fabric required in conjunction with the drain construction should consist of a nominal eight ounce per square yard, needle-punched, non-woven polypropylene fabric intended specifically for this purpose. The contractor should submit his fabric and aggregate information to us for review prior to implementing into the construction. It is important for the fabric to be placed in imminent contact with a relatively undisturbed soil interface to reduce the potential for clogging of the fabric. No fabric should be placed in a wet or muddy excavation or subgrade. The filter system for the drain consists not only of the filter fabric, but also the soil materials immediately adjacent to the fabric, creating a composite system.

Unless it is completely removed, the grouted Lower Dam spillway pipe should be cut off upstream of the toe drain. We recommend a sand blanket 2 1/2 feet thick, extending 2 feet outside the pipe diameter be placed immediately downstream and in contact with that pipe and should be constructed to be in positive contact with the toe drain.

The contractor should take care during construction to prevent contamination of the drain material prior to completing its installation. As such, all portions of the crushed stone and pipe should be protected from siltation, erosion, or surface runoff by filter fabric, even temporarily such as at the end of the day. Should the stone or pipe somehow become contaminated during construction, the geotechnical engineer will determine the magnitude and length of contaminated drain, requiring that portion of drain to be excavated and replaced.

**CONTINUED ON DWG. C-101C**

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 CHECKED BY: JR      01/23  
 CDA: PEF 000707      EXP: 06/30/2024  
 JASON RAPPLEAN, PE, EDR  
 LEVEL 2 CERTIFICATION # 3931  
 EXPIRATION DATE: 12/07/26  
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**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**GEOTECHNICAL NOTES**  
  
**CITY OF TUCKER**

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**GEOTECHNICAL NOTES**  
**CONTINUED FROM DWG. C-101B**

**7.8 EARTHWORK**

Prior to placement of any fill, all subgrades should be approved by the geotechnical engineer. This is to verify that all unsuitable materials have been removed and that the subgrade can receive the necessary compactive effort without additional remediation. Substantial undercutting should be expected in the areas currently under the lake(s) and in the area into which the Lower Dam will be extended downstream. Stone may not be used to stabilize subgrades.

In some areas, the contractor will be working in near proximity to groundwater. As such, we recommend that the contractor use a relatively light or wide tracked dozer, such as a Caterpillar D4 or equivalent and a small ride-on pad foot vibratory compactor for these initial lifts of fill. Use of heavy or narrow tracked equipment on what is likely to be a wetter and weaker than normal subgrade may cause excessive pumping and create other destabilizing forces in the underlying. Once "out of the bottom" the contractor can revert to heavier equipment if he desires.

All fill material placed should be compacted to at least 95 percent of the soil's standard Proctor maximum dry density. The fill should be placed at or above the soil's optimum moisture content to reduce the soil's permeability. All fill materials should consist of clean soil, with rock fragments no larger than approximately three inches. The material should be placed in relatively thin essentially horizontal lifts across as much as the work area as possible. Each lift of fill should be thoroughly compacted with towed or self-propelled sheepsfoot rollers, or other similar approved equipment. Each lift should be left in a non-smooth condition to assure adequate bonding with overlying fill lifts. Should a smooth condition occur, it will be necessary to lightly scarify each lift prior to additional fill placement. Any fill that is saturated or overly dry, or materials that become frozen, should be removed, or reconditioned prior to additional fill placement. Some moisture conditioning of the fill may be required to assure that adequate compaction can be achieved and to help control moisture contents as discussed.

All fill placement should be monitored on essentially a full-time basis by a senior engineering technician working directly under an experienced geotechnical engineer. This individual will verify that suitable materials are placed at the required compacted effort and moisture contents. In addition, the fill placement procedures, including preparation of each fill lift for subsequent fill placement, will be monitored by this individual.

During construction, the new fill should be adequately "benched" into the existing fill or abutment soils to ensure adequate bonding between the new and existing soils and to prevent a predefined plane of weakness along that interface.

Soils with very low densities or containing excessive mica are not generally suitable for dam construction due to the increase potential for erosion and "piping" of soils caused by seepage. We recommend that all soils within the embankment be compacted to a dry density of at least 90 pcf unless specifically approved by the geotechnical engineer, in addition to meeting the other requirements of percent compaction and moisture content. The decision as to whether soils contain too much mica is subjective and will be made by the engineer or engineering technician.

For the purposes of this project, we have used the terminology "select" and "common" to represent different classes of soil materials and their general placement within the embankment. "Select" fill should be used to backfill in all undercut areas and where excavations are made in the dam(s) such as to construct new spillways, remove existing spillway pipes or other areas of excavation in the dams themselves. All other areas can be backfilled using "common" soils. "Select" fills are defined as soil materials having USCS designations CL, ML, MH and SC and "common" soils can be all of the "select" designations plus SM. All SC and SM materials are required to have at least 30% passing the #200 sieve and a Plasticity Index of at least 5.

Any soils removed from the existing dams, side channel spillway, areas adjacent to the side channel spillway or soils undercut, are likely to be unsuitable for reuse as structural fill. These soils should be wasted or used as "landscape" fills. This includes excavations adjacent to and beyond the lake edge such as those shown on sheets C-109 Section B, C-110 Section D and other similar excavations. It is likely that no excavation represented on the site drawings will produce a significant volume of soils suitable for use as structural fills.

**7.9 PRINCIPAL SPILLWAY(S)**

As part of the upgrade(s), new concrete spillways will be constructed for both dams.

The elevation at the bottom of the spillway base slab of the Lower Dam will be around 974 feet. Alluvial soils were encountered in borings L-1 and L-2 just a few feet below that elevation. We recommend all alluvium and overlying fill be excavated from below the labyrinth slab with the limits of the alluvium removal extending 5 feet laterally beyond the base slab area. This requirement includes the wing walls.

The borings encountered 4 to 5 feet of alluvium at the drilled locations, alluvium may be deeper intermediate of the boring locations.

As discussed previously, no borings were drilled for the Upper Dam. We expect similar conditions will be discovered in the construction. If alluvium is found to exist below the principal spillway of the Upper Dam our engineers should evaluate the specific conditions encountered but we expect our recommendations will be to remove the alluvium and any existing fills which overlie the alluvium. This requirement will probably include the wing walls.

All exposed subgrades should be in a compact condition at the time of underdrain and/or concrete slab construction. All disturbed materials should either be compacted in place to the required density, with appropriate moisture conditioning, if needed, or the material should be removed and replaced with new structural fill. For minor depths of over-excavation needed to remove disturbed materials, an option may be to utilize additional aggregates for the underdrain system and/or additional concrete where slabs-on-grade are created to replace the over excavated materials. This should be left to the discretion of the contractor, based on discussions with the engineers, and should be provided at no additional cost to the project.

**7.10 EROSION CONTROL**

After final grading and proper compaction of the exposed slopes and crest, suitable erosion protection should be provided. Low maintenance grasses are most commonly employed on the downstream slope, the exposed portion of the upstream slope and portions of the crest not under pavements. It has been our experience that on dams where a good vegetative cover is not established early on, problems with erosion resulting in higher long-term maintenance may occur. The crest of the dams should be sloped slightly upstream.

**8.0 QUALIFICATIONS OF RECOMMENDATIONS**

This evaluation of the geotechnical aspects of the proposed design and construction has been based on our understanding of the project and the data obtained during these studies. The general subsurface conditions used in our evaluation were based on interpolation of the subsurface data between the test locations. Regardless of the thoroughness of a subsurface exploration, there is the possibility that conditions will differ between test locations, that conditions are not as anticipated by the designers, or that the construction process has modified the soil conditions. Therefore, experienced soil engineers and technicians should evaluate earthwork and foundation construction to verify that the conditions anticipated in design actually exist. Otherwise, we assume no responsibility for construction compliance with the design concepts, specifications, or recommendations.

The recommendations contained in this report have been developed on the basis of the previously described project characteristics and subsurface conditions. If project criteria change, we should be permitted to determine if the recommendations should be modified. The findings of such a review will be presented in a supplemental report. Even after completion of a subsurface study, the nature and extent of variation between test locations may not become evident until the course of construction. If such variations then become evident, it will be necessary to reevaluate the recommendations of this report after on-site observations of the conditions.

These professional services have been performed, the findings derived, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all warranties either expressed or implied. This company is not responsible for the conclusions, opinions or recommendations of others based on these data.

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**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**GEOTECHNICAL NOTES**

**CITY OF TUCKER**

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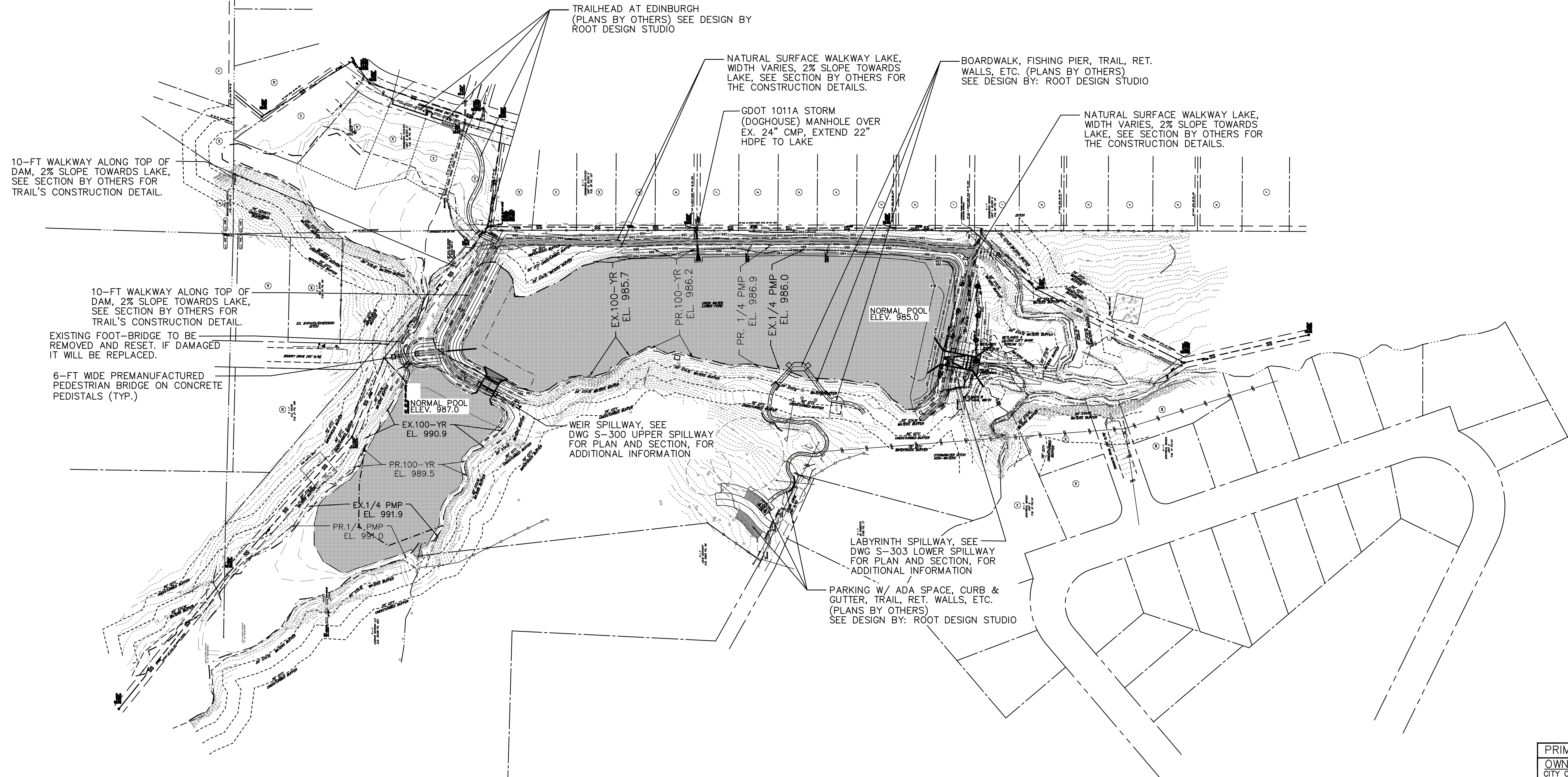
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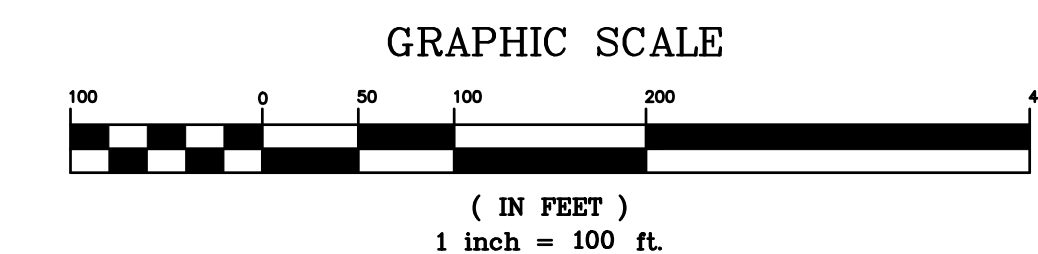
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 JASON RAPPLEAN  
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**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**RESERVOIR PLAN**

**CITY OF TUCKER**

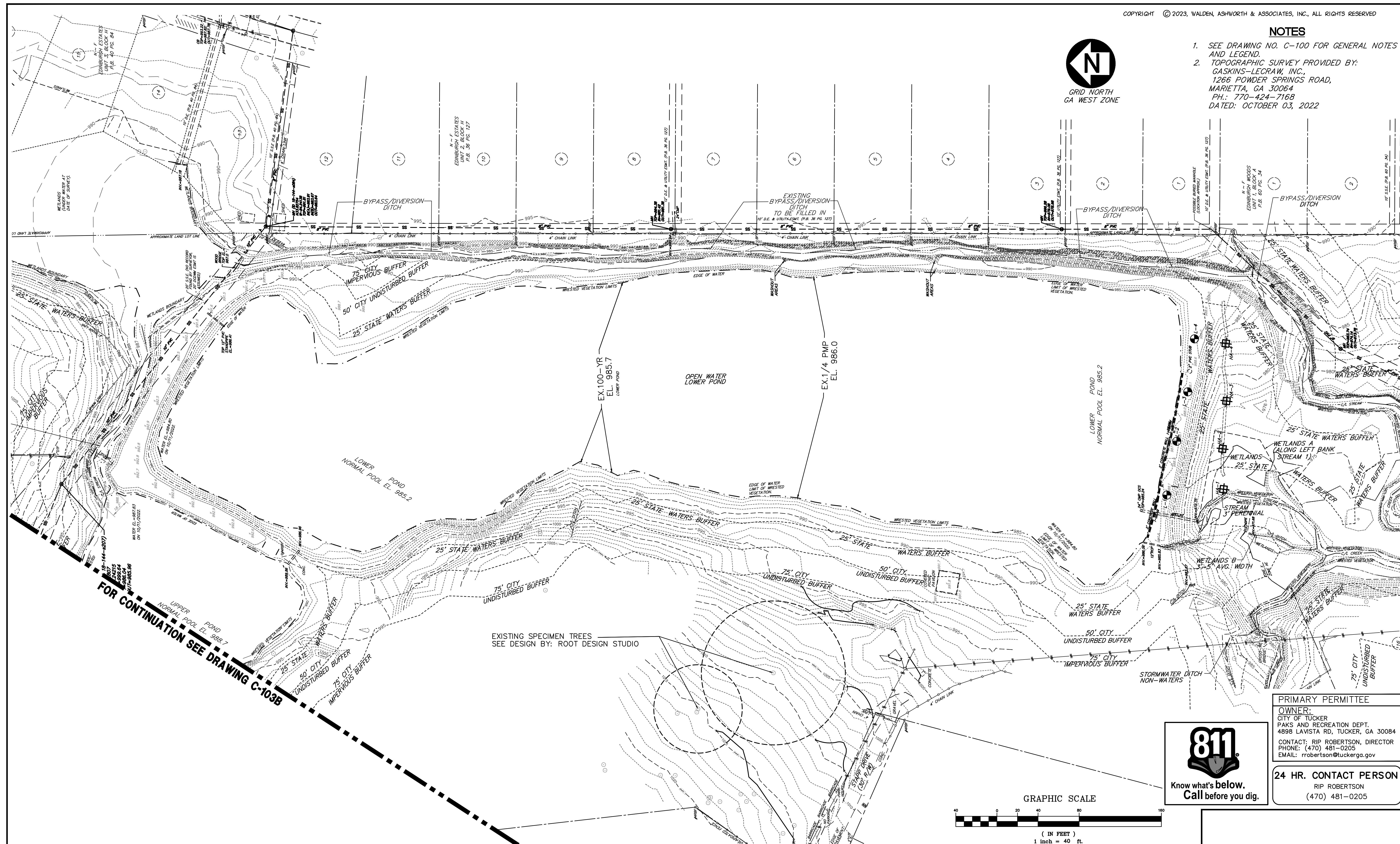
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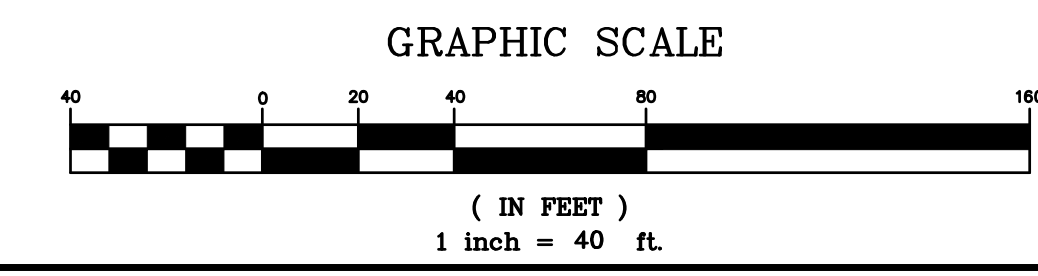
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**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**EXISTING CONDITIONS PLAN - LOWER POND**

**CITY OF TUCKER**

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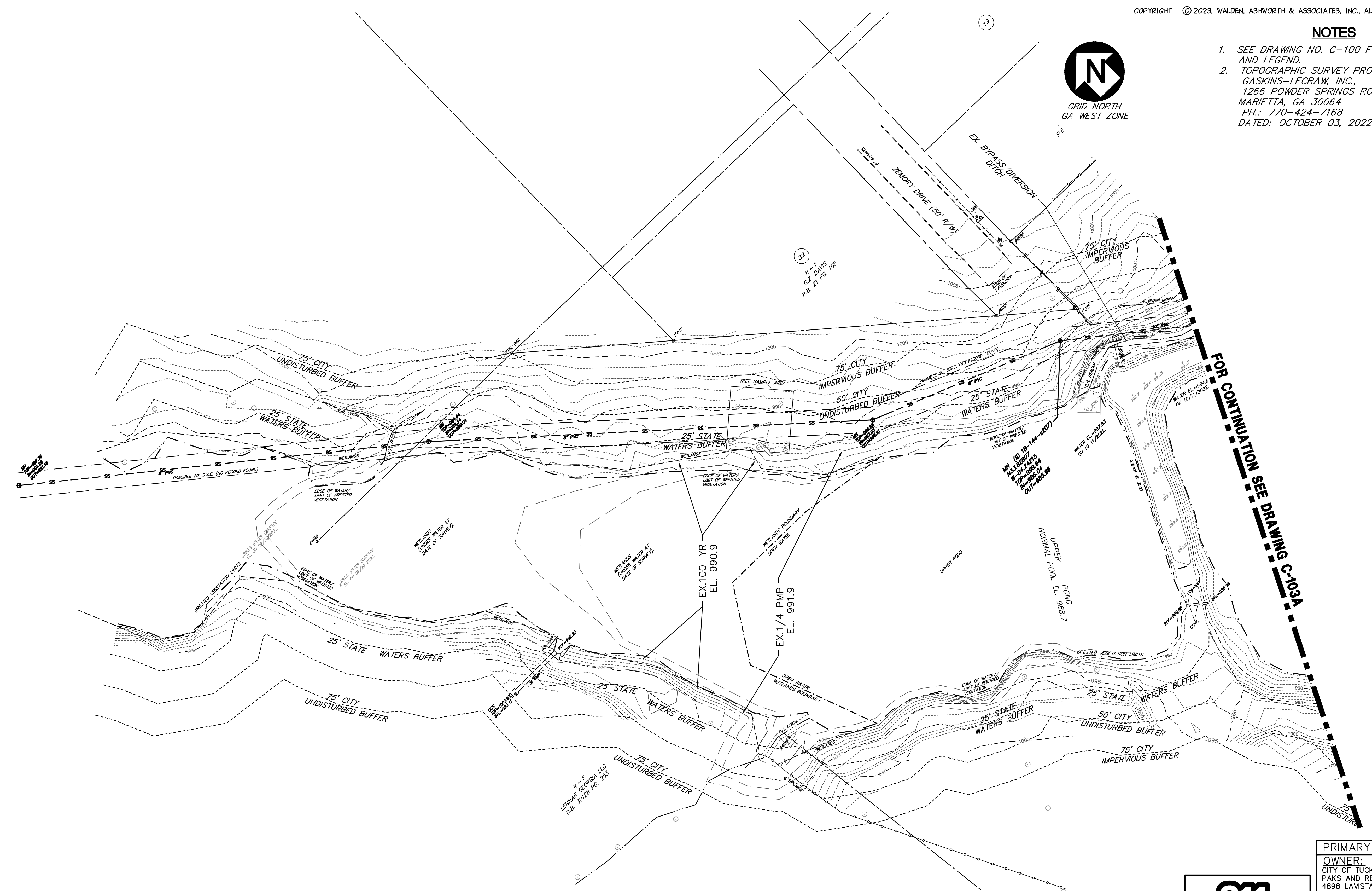


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GRID NORTH  
GA WEST ZONE



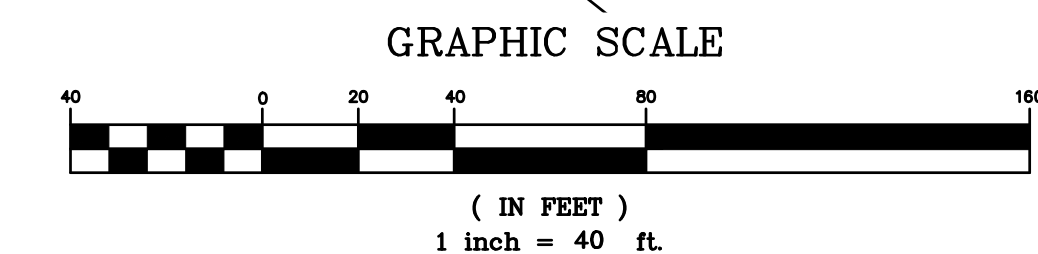
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**JOHNS HOMESTEAD PARK  
DAM RENOVATION  
EXISTING CONDITIONS PLAN - UPPER POND**

**CITY OF TUCKER**

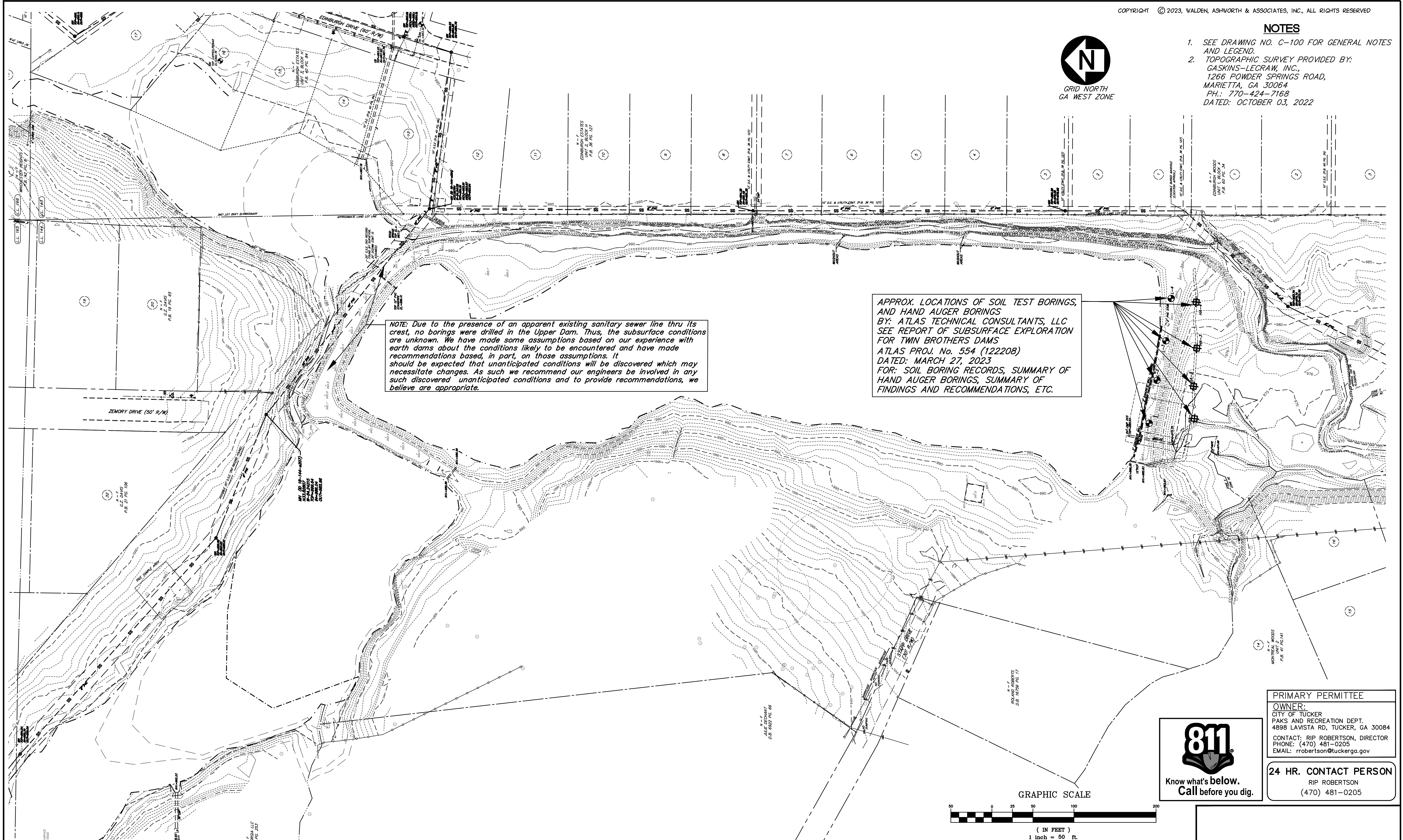
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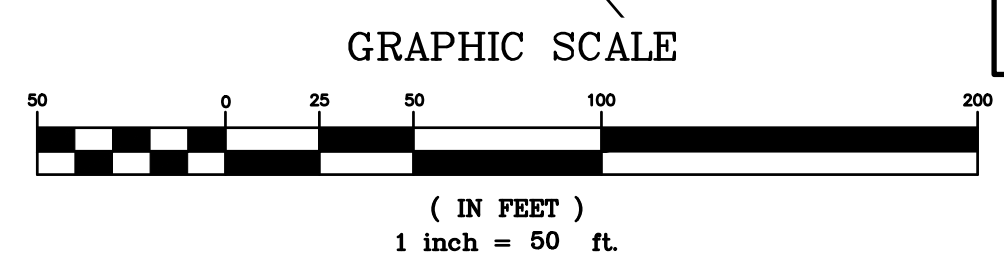
NOTE: Due to the presence of an apparent existing sanitary sewer line thru its crest, no borings were drilled in the Upper Dam. Thus, the subsurface conditions are unknown. We have made some assumptions based on our experience with earth dams about the conditions likely to be encountered and have made recommendations based, in part, on those assumptions. It should be expected that unanticipated conditions will be discovered which may necessitate changes. As such we recommend our engineers be involved in any such discovered unanticipated conditions and to provide recommendations, we believe are appropriate.

APPROX. LOCATIONS OF SOIL TEST BORINGS, AND HAND AUGER BORINGS BY: ATLAS TECHNICAL CONSULTANTS, LLC SEE REPORT OF SUBSURFACE EXPLORATION FOR TWIN BROTHERS DAMS ATLAS PROJ. No. 554 (122208) DATED: MARCH 27, 2023 FOR: SOIL BORING RECORDS, SUMMARY OF HAND AUGER BORINGS, SUMMARY OF FINDINGS AND RECOMMENDATIONS, ETC.



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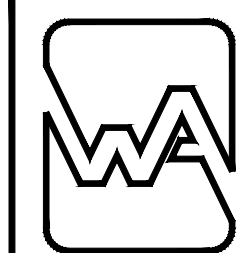
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**JOHNS HOMESTEAD PARK  
 DAM RENOVATION  
 BORING PLAN**

**CITY OF TUCKER**



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2. TOPOGRAPHIC SURVEY PROVIDED BY: GASKINS-LECRAW, INC., 1266 POWDER SPRINGS ROAD, MARIETTA, GA 30064 PH.: 770-424-7168 DATED: OCTOBER 03, 2022
3. OWNER SHALL ACQUIRE ALL NECESSARY EASEMENTS FOR ACCESS AND CONSTRUCTION PRIOR TO BIDDING THE PROJECT.



REFER TO JOHN HOMESTEAD PARK IMPROVEMENTS BY ROOT DESIGN STUDIO FOR ADDITIONAL DEMOLITION NECESSARY TO CONSTRUCT PARK IMPROVEMENTS, ETC.

POTENTIAL CONSTRUCTION TRAILER, PARKING AND STAGING AREA (LOT 13).

REPLACE FROM MH TO MH, 10-INCH PVC SAN. SEWER LINE WITH 10-INCH D.I.P., AND WHERE LESS THAN 4-FT OF COVER PLACE IN A STEEL CASING. TEMP. BYPASS PUMP FROM MH TO MH THROUGHOUT THE ENTIRE PROCESS UNTIL DEKALB CO APPROVES THE AS-BUILTS. SEE PROFILE C-112.

TEMPORARILY REMOVE WALKING BRIDGE ACROSS CHANNEL TO BE REMOVED

REMOVE EXISTING 12-INCH PVC STANDPIPE, PROTECT EXISTING 8-INCH SANITARY SEWER LINE THROUGHOUT CONSTRUCTION.

INSTALL TEMPORARY PIPING TO CROSS SIDE CHANNEL SPILLWAY TO GAIN ACCESS TO EXISTING AND PROPOSED UPPER DAMS FOR DEMOLITION AND CONSTRUCTION.

REFER TO JOHN HOMESTEAD PARK IMPROVEMENTS BY ROOT DESIGN STUDIO FOR ADDITIONAL DEMOLITION NECESSARY TO CONSTRUCT PARK IMPROVEMENTS, ETC.

APPROX. LABYRINTH STR. EXCAVATION LIMITS

REMOVE APPROX. 74 LF OF EXISTING 2-FT CONCRETE WALL 1-BEAM, 2" PVC STUB OUT, 24-INCH OCS (RISER) AND 24-INCH CORRUGATED METAL RISER AND THE LOW LEVEL DRAIN PIPE THAT IS WITHIN THE LABYRINTH SPILLWAY'S EXCAVATION LIMITS.

REMOVE EXISTING LOW LEVEL DRAIN.

EXTEND ZEMORY DR. TO UPPER DAM (GRAVEL ROAD). PROTECT ALL EXIST. UTILITIES AND MAKE REPAIRS TO ROADWAY DURING CONSTRUCTION TO KEEP IT PASSABLE FOR THE OWNERS THAT USE IT EVERYDAY. PRIOR TO BEGINNING CONSTR. THE CONTRACTOR AND THE CITY PUBLIC WORKS DEPT. WILL EVALUATE THE ROADWAY TO DETERMINE THE EXTENT OF DAMAGE IT HAS INCURRED PRIOR TO CONST. BEGINNING AND AT THE END OF CONSTRUCTION, THE ROAD WILL BE RE-EVALUATED TO DETERMINE THE PERCENT COST SHARED BETWEEN THE CITY AND THE CONTRACTOR. (TYP. BOTH ROADS)

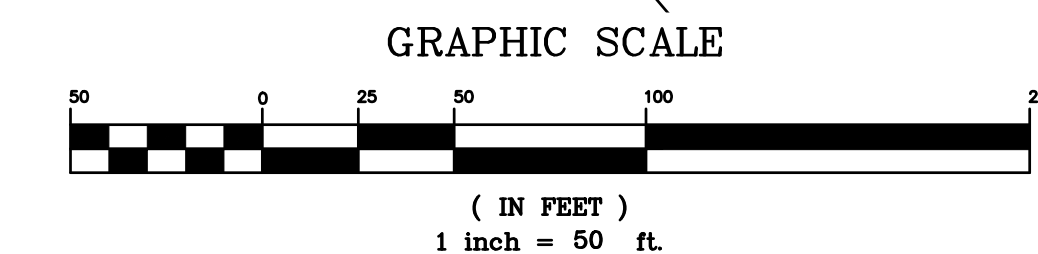
EXTEND STAPP DR. TO LOWER DAM (GRAVEL ROAD). PROTECT ALL EXIST. UTILITIES AND MAKE REPAIRS TO ROADWAY DURING CONSTRUCTION TO KEEP IT PASSABLE FOR THE OWNERS THAT USE IT EVERYDAY. AT THE END OF CONSTRUCTION, REPAIR THE ROAD TO DEKALB CO. ROADWAY STDS.

REMOVE EXISTING 12" DRAIN AND THE 12-INCH RCP PIPE IN RIGHT ABUTMENT.

**PRIMARY PERMITTEE**  
 OWNER:  
 CITY OF TUCKER  
 PAKS AND RECREATION DEPT.  
 4898 LAVISTA RD, TUCKER, GA 30084  
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 PHONE: (470) 481-0205  
 EMAIL: rrobertson@tuckerga.gov



**24 HR. CONTACT PERSON**  
 RIP ROBERTSON  
 (470) 481-0205



NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS



SCALE: 1"=50'	DATE:
DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	3031
LEVEL 2 CERTIFICATION #	12/07/26
EXPIRATION DATE:	

**JOHNS HOMESTEAD PARK  
 DAM RENOVATION  
 DEMOLITION PLAN**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
 CONSULTING ENGINEERS  
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4201600 **42016- C-105** **0**

06/19/2024



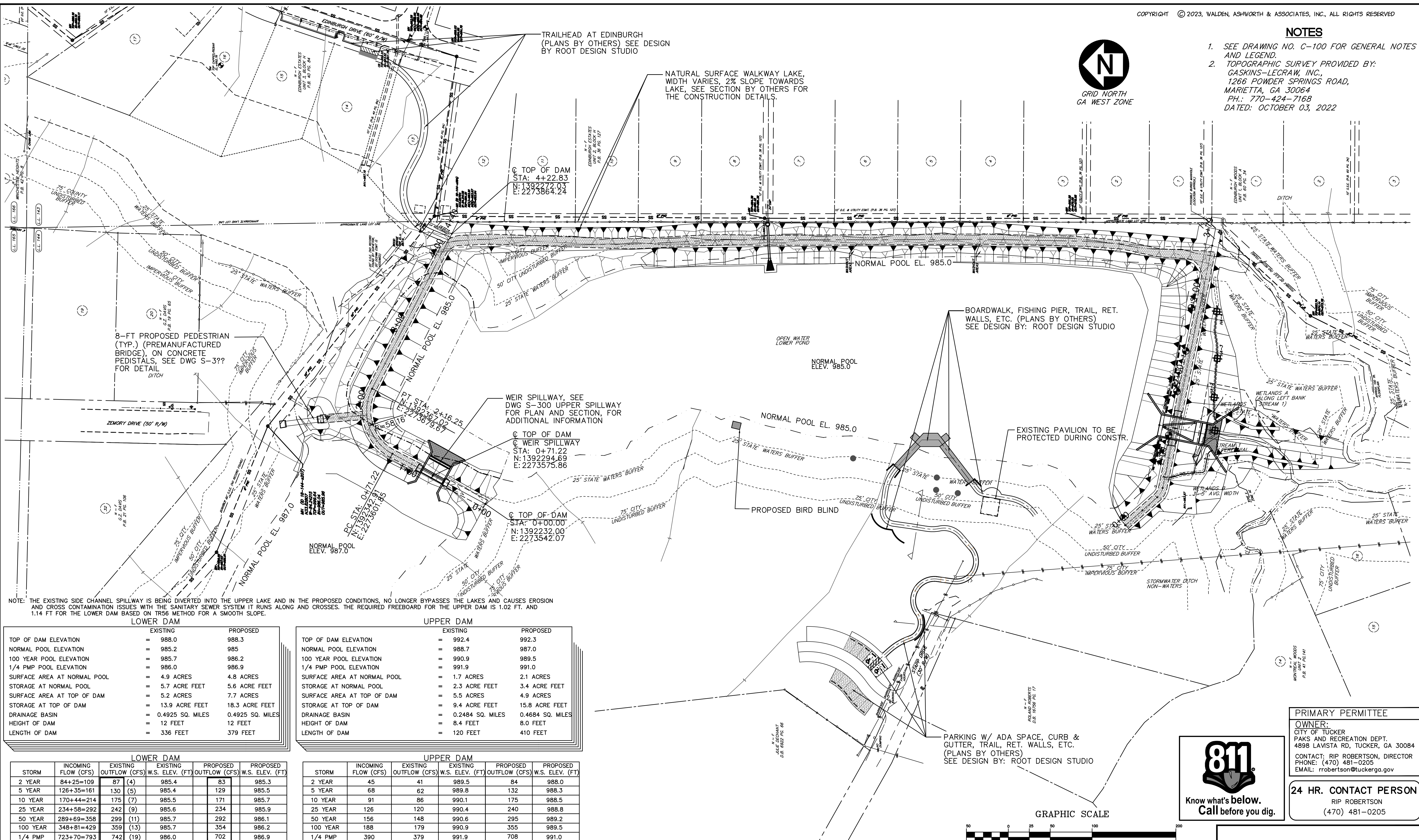
**NOTES**

1. SEE DRAWING NO. C-100 FOR GENERAL NOTES AND LEGEND.
2. TOPOGRAPHIC SURVEY PROVIDED BY: GASKINS-LECRAW, INC., 1266 POWDER SPRINGS ROAD, MARIETTA, GA 30064 PH.: 770-424-7168 DATED: OCTOBER 03, 2022



TRAILHEAD AT EDINBURGH (PLANS BY OTHERS) SEE DESIGN BY ROOT DESIGN STUDIO

NATURAL SURFACE WALKWAY LAKE, WIDTH VARIES, 2% SLOPE TOWARDS LAKE, SEE SECTION BY OTHERS FOR THE CONSTRUCTION DETAILS.



NOTE: THE EXISTING SIDE CHANNEL SPILLWAY IS BEING DIVERTED INTO THE UPPER LAKE AND IN THE PROPOSED CONDITIONS, NO LONGER BYPASSES THE LAKES AND CAUSES EROSION AND CROSS CONTAMINATION ISSUES WITH THE SANITARY SEWER SYSTEM IT RUNS ALONG AND CROSSES. THE REQUIRED FREEBOARD FOR THE UPPER DAM IS 1.02 FT. AND 1.14 FT FOR THE LOWER DAM BASED ON TR56 METHOD FOR A SMOOTH SLOPE.

**LOWER DAM**

	EXISTING	PROPOSED
TOP OF DAM ELEVATION	= 988.0	988.3
NORMAL POOL ELEVATION	= 985.2	985
100 YEAR POOL ELEVATION	= 985.7	986.2
1/4 PMP POOL ELEVATION	= 986.0	986.9
SURFACE AREA AT NORMAL POOL	= 4.9 ACRES	4.8 ACRES
STORAGE AT NORMAL POOL	= 5.7 ACRE FEET	5.6 ACRE FEET
SURFACE AREA AT TOP OF DAM	= 5.2 ACRES	7.7 ACRES
STORAGE AT TOP OF DAM	= 13.9 ACRE FEET	18.3 ACRE FEET
DRAINAGE BASIN	= 0.4925 SQ. MILES	0.4925 SQ. MILES
HEIGHT OF DAM	= 12 FEET	12 FEET
LENGTH OF DAM	= 336 FEET	379 FEET

**UPPER DAM**

	EXISTING	PROPOSED
TOP OF DAM ELEVATION	= 992.4	992.3
NORMAL POOL ELEVATION	= 988.7	987.0
100 YEAR POOL ELEVATION	= 990.9	989.5
1/4 PMP POOL ELEVATION	= 991.9	991.0
SURFACE AREA AT NORMAL POOL	= 1.7 ACRES	2.1 ACRES
STORAGE AT NORMAL POOL	= 2.3 ACRE FEET	3.4 ACRE FEET
SURFACE AREA AT TOP OF DAM	= 5.5 ACRES	4.9 ACRES
STORAGE AT TOP OF DAM	= 9.4 ACRE FEET	15.8 ACRE FEET
DRAINAGE BASIN	= 0.2484 SQ. MILES	0.4684 SQ. MILES
HEIGHT OF DAM	= 8.4 FEET	8.0 FEET
LENGTH OF DAM	= 120 FEET	410 FEET

**LOWER DAM**

STORM	INCOMING FLOW (CFS)	EXISTING OUTFLOW (CFS)	EXISTING W.S. ELEV. (FT)	PROPOSED OUTFLOW (CFS)	PROPOSED W.S. ELEV. (FT)
2 YEAR	84+25=109	87 (4)	985.4	83	985.3
5 YEAR	126+35=161	130 (5)	985.4	129	985.5
10 YEAR	170+44=214	175 (7)	985.5	171	985.7
25 YEAR	234+58=292	242 (9)	985.6	234	985.9
50 YEAR	289+69=358	299 (11)	985.7	292	986.1
100 YEAR	348+81=429	359 (13)	985.7	354	986.2
1/4 PMP	723+70=793	742 (19)	986.0	702	986.9

**UPPER DAM**

STORM	INCOMING FLOW (CFS)	EXISTING OUTFLOW (CFS)	EXISTING W.S. ELEV. (FT)	PROPOSED OUTFLOW (CFS)	PROPOSED W.S. ELEV. (FT)
2 YEAR	45	41	989.5	84	988.0
5 YEAR	68	62	989.8	132	988.3
10 YEAR	91	86	990.1	175	988.5
25 YEAR	126	120	990.4	240	988.8
50 YEAR	156	148	990.6	295	989.2
100 YEAR	188	179	990.9	355	989.5
1/4 PMP	390	379	991.9	708	991.0

NOTE: INFLOW INCLUDES SIDE CHANNEL SPILLWAY FLOW, PLUS LOWER LAKE'S BASIN FLOW, EX. THE OUTFLOW FROM THE LAKE IS IN PARENTHESIS. THE SIDE CHANNEL SPILLWAY IS FILLED IN FOR THE PROPOSED CONDITION ANALYSIS. THE PEAK RUNOFF RATES AT THE STUDY POINT, WHERE THE LOWER LAKE'S DISCHARGE INTERSECTS THE CREEK, IS SHOWN IN A THICKER BOX.

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**24 HR. CONTACT PERSON**  
 RIP ROBERTSON  
 (470) 481-0205

**811**  
 Know what's below.  
 Call before you dig.

**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**SITE LAYOUT PLAN**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
 CONSULTING ENGINEERS  
 P.O. BOX 6462  
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SCALE: 1"=50'  
 DESIGN BY: JR/JH 01/23  
 DRAWN BY: JH/EC 01/23  
 CHECKED BY: JR 01/23  
 CDA# PEF 000707 EXP: 06/30/2024

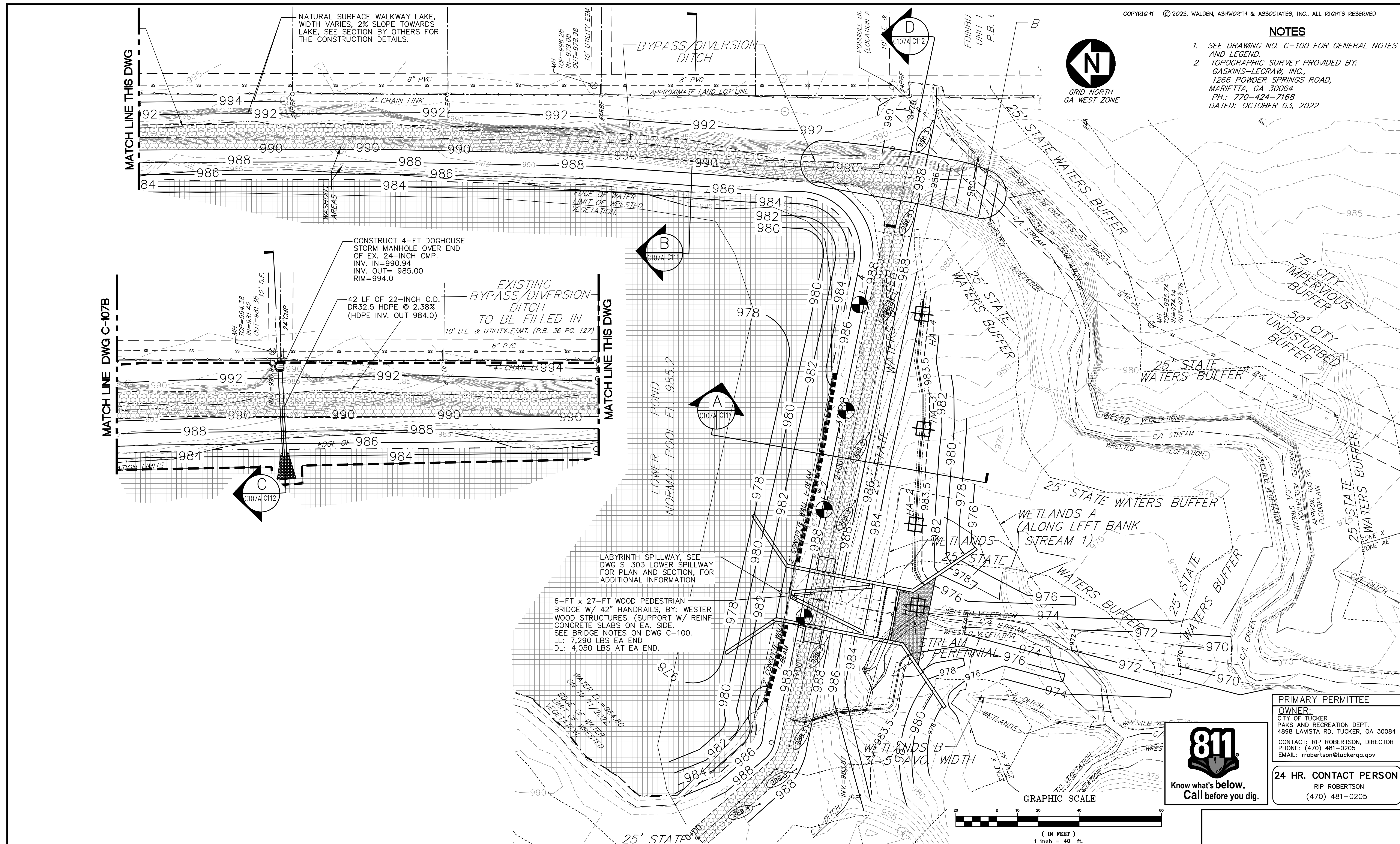
JASON RAPPLEAN, PE, EDR  
 LEVEL 2 CERTIFICATION # 3091  
 EXPIRATION DATE: 12/07/26

06/19/2024

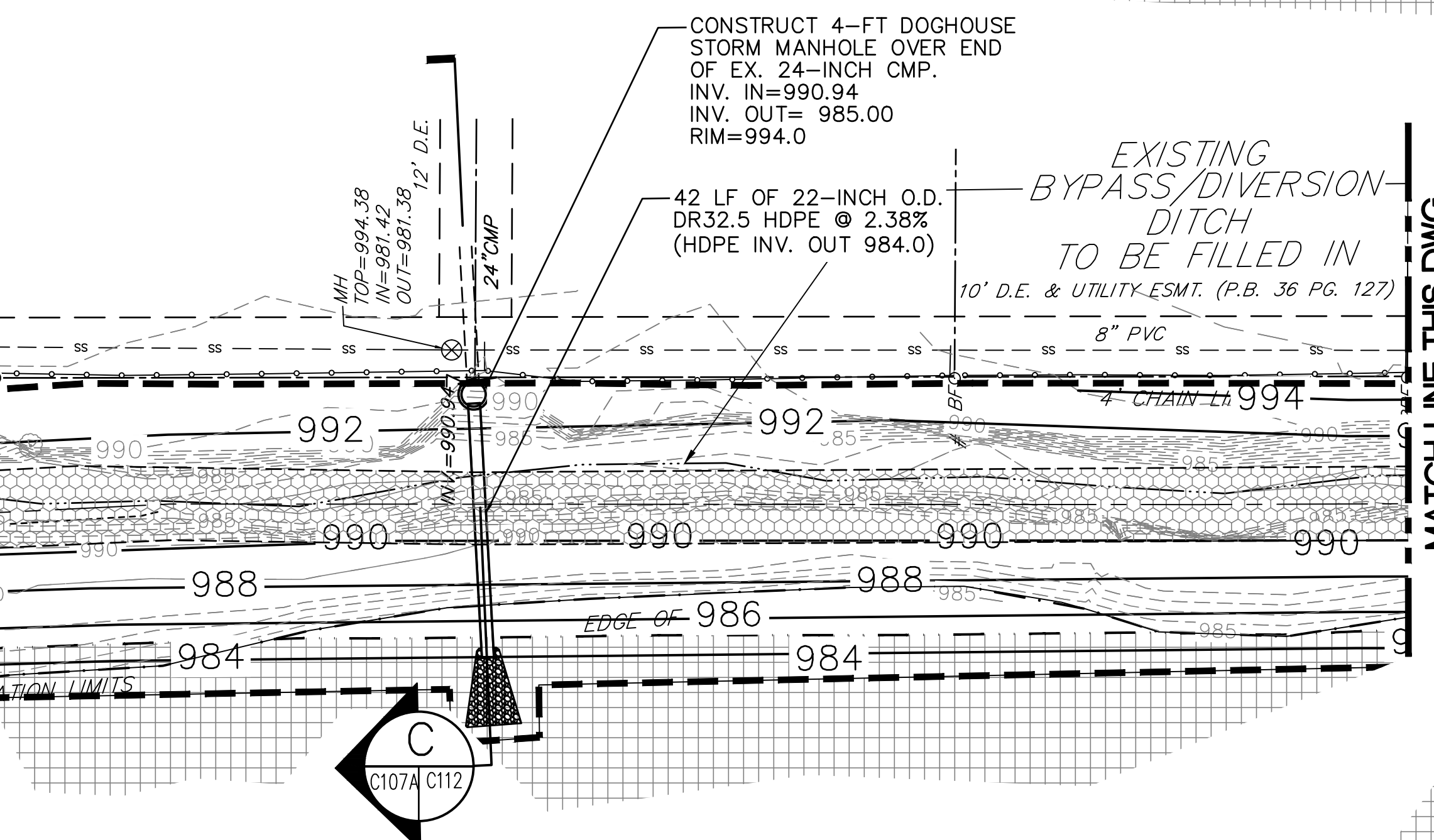


**NOTES**

1. SEE DRAWING NO. C-100 FOR GENERAL NOTES AND LEGEND.
2. TOPOGRAPHIC SURVEY PROVIDED BY: GASKINS-LECRAW, INC., 1266 POWDER SPRINGS ROAD, MARIETTA, GA 30064. PH.: 770-424-7168. DATED: OCTOBER 03, 2022



MATCH LINE DWG C-107B

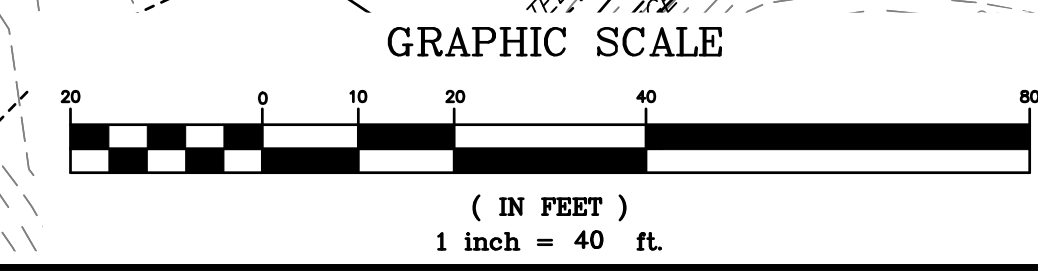


MATCH LINE THIS DWG

**PRIMARY PERMITTEE**  
 OWNER:  
 CITY OF TUCKER  
 PAKS AND RECREATION DEPT.  
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NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

**GEORGIA REGISTERED PROFESSIONAL ENGINEER**  
 JASON RAPPLEAN, PE, EDR  
 No. 30441  
 LEVEL 2 CERTIFICATION # 3091  
 EXPIRATION DATE: 12/07/26

06/19/2024

**JOHNS HOMESTEAD PARK  
 DAM RENOVATION  
 GRADING PLAN - LOWER POND**

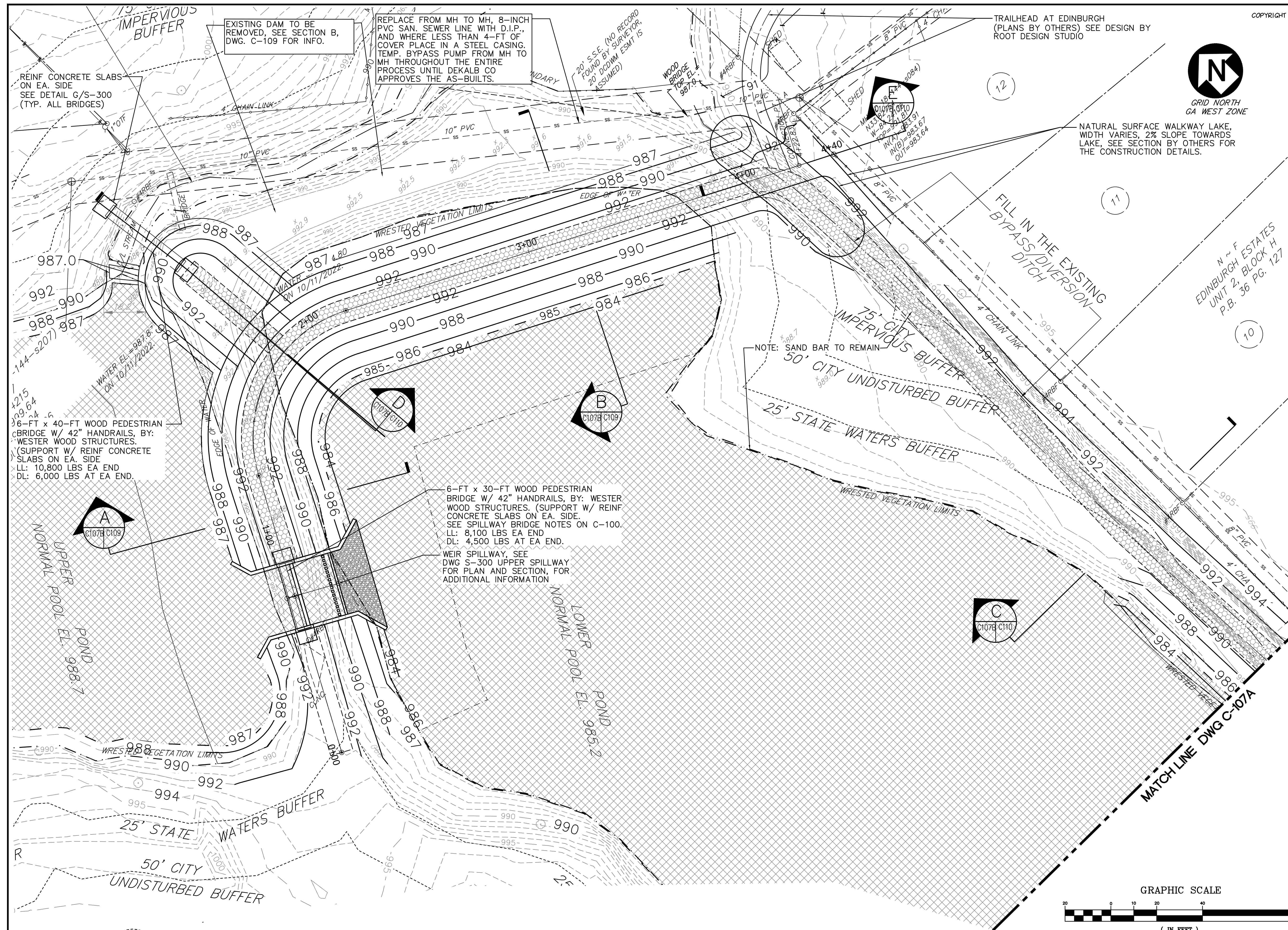
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 DATE: 01/23  
 DESIGN BY: JR/JH  
 DRAWN BY: JH/EC  
 CHECKED BY: JR  
 CD# PEF 000707 EXP. 06/30/2024

**CITY OF TUCKER**

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4201600 **42016- C-107A** 0





**NOTES**

- SEE DRAWING NO. C-100 FOR GENERAL NOTES AND LEGEND.
- TOPOGRAPHIC SURVEY PROVIDED BY: GASKINS-LECRAW, INC., 1266 POWDER SPRINGS ROAD, MARIETTA, GA 30064 PH.: 770-424-7168 DATED: OCTOBER 03, 2022
- SANITARY SEWER LINE REPLACEMENT WITH D.I.P. TO BE DONE IN ACCORDANCE WITH DEKALB COUNTY WATERSHED MANAGEMENT, (SANITARY SEWER DEPT.). NO WORK MAY BE PERFORMED ON THE EXISTING LINE WITHOUT A WATERSHED INSPECTOR BEING ONSITE MONITORING THE WORK FROM MH TO MH. ALL WORK SHALL ADHERE TO THE WATERSHED DEPARTMENT'S LATEST ED. OF STANDARDS AND SPECIFICATIONS. BYPASS PUMPING SHALL BE CONTINUOUS UNTIL THE REPLACED LINE IS ACCEPTED BY THE COUNTY IN WRITING.
- IF/WHERE THE 8" (SS) DIP LINE HAS LESS THAN 4'-FT OF COVER, IT SHALL BE PLACED INSIDE OF A STEEL CASING APPROVED BY DCDWM.
- TRENCHES AND PIPE BEDDING TO BE APPROVED BY DCDWM PRIOR TO PIPE PLACEMENT.
- NEW RESILIENT RUBBER BOOTS SHALL BE PLACED INSIDE THE MANHOLES WHERE THE PIPE IS BEING REPLACED. THE MH TABLES SHOULD BE INSPECTED AND REPAIRED AS NECESSARY TO INSTALL THE REPL. DIP LINE.
- THE TRENCH BOTTOM SHALL BE SHAPED TO THE CONTOURS OF THE PIPE (NO STONE BEDDING), AND BACKFILLED AND HAND-COMPACTED IN 6-INCH LIFTS TO A MIN. DENSITY OF 95% OF MAX. DRY DENSITY PER ASTM D-698. MATERIAL SHALL BE SELECT FILL, THE SAME MATERIAL USED IN THE DAM'S RENOVATION.
- TESTING (AIR TEST) PER DCDWM REQMTS., A CCTV MAY BE REQUIRED BY DCDWM.
- DCDWM MAY REQUIRE A RECORD DWG. OF THE PIPE REPLACEMENT AND A 12-MONTH WARRANTY.

DEKALB CO. DEPT. OF WATERSHED MGMT CHART

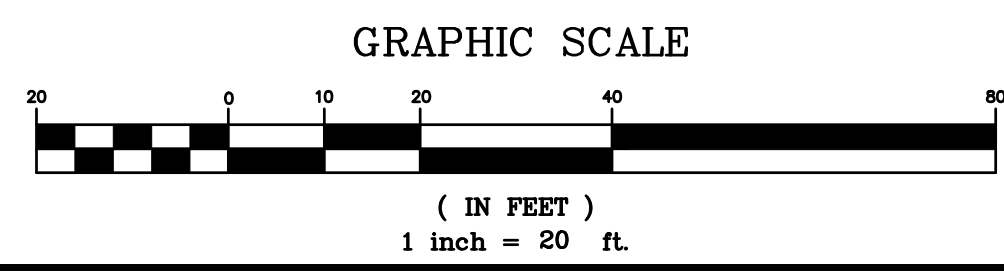
MH #	RIM EL. (FT)	INV. EL. (FT)	PIPE (L) IN (FT)	PIPE DIA. (FT)	SLOPE AND MAT. (%)
A1	999.64	985.96	986.04	319.66	Ex. 8" PVC CHG. TO 8" DIP (SS) CL. 350 0.73%
A2	991.81	983.64	983.91		

NOTE: THE EXISTING 8" PVC (SS) LINE WILL BE REPLACED WITH AN 8" (SS) DIP CL. 350 LINE AT THE SAME INVERTS AND PIPE SLOPE BETWEEN THE EXISTING MANHOLES DUE TO BEING AT THE TOE OF A DAM AND NOT HAVING SUFFICIENT COVER OVER IT FOR PVC (SS) PIPE. NO LATERALS ARE LOCATED OFF OF THE LINE, IT SHOULD BE A SIMPLE MATERIAL CHANGE FROM MANHOLE TO MANHOLE.

**811**  
Know what's below. Call before you dig.

**PRIMARY PERMITEE**  
OWNER:  
CITY OF TUCKER  
PAKS AND RECREATION DEPT.  
4898 LAVISTA RD, TUCKER, GA 30084  
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**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205



NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

SCALE: 1"=20'

DESIGN BY: JR/JH 01/23  
DRAWN BY: JH/EC 01/23  
CHECKED BY: JR 01/23  
CD# PEF 000707 EXP. 06/30/2024

JASON RAPPLEAN, PE, EDR  
LEVEL 2 CERTIFICATION # 3091  
EXPIRATION DATE: 12/07/26

06/19/2024

**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**GRADING PLAN - UPPER POND**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
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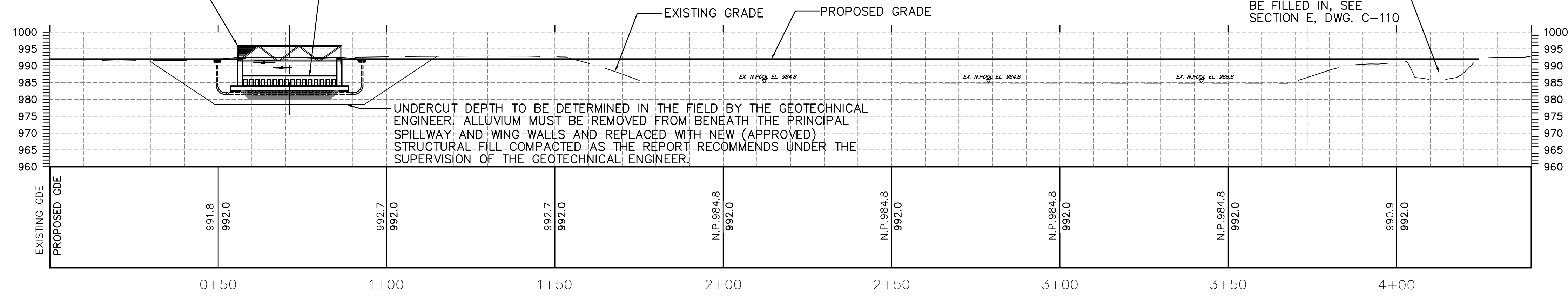
4201600 **42016- C-107B** **0**



PEDESTRIAN BRIDGE W/ MIN. 42" BETWEEN HANDRAILS ON REINF. CONCRETE (SUPPORT) SLABS ON EA. SIDE; DESIGNED BY OTHERS. SEE BRIDGE NOTES ON DWG. C-100.

WEIR SPILLWAY, SEE DWG S-300 UPPER SPILLWAY FOR PLAN AND SECTION, FOR ADDITIONAL INFORMATION

EXISTING CHANNEL TO BE FILLED IN, SEE SECTION E, DWG. C-110



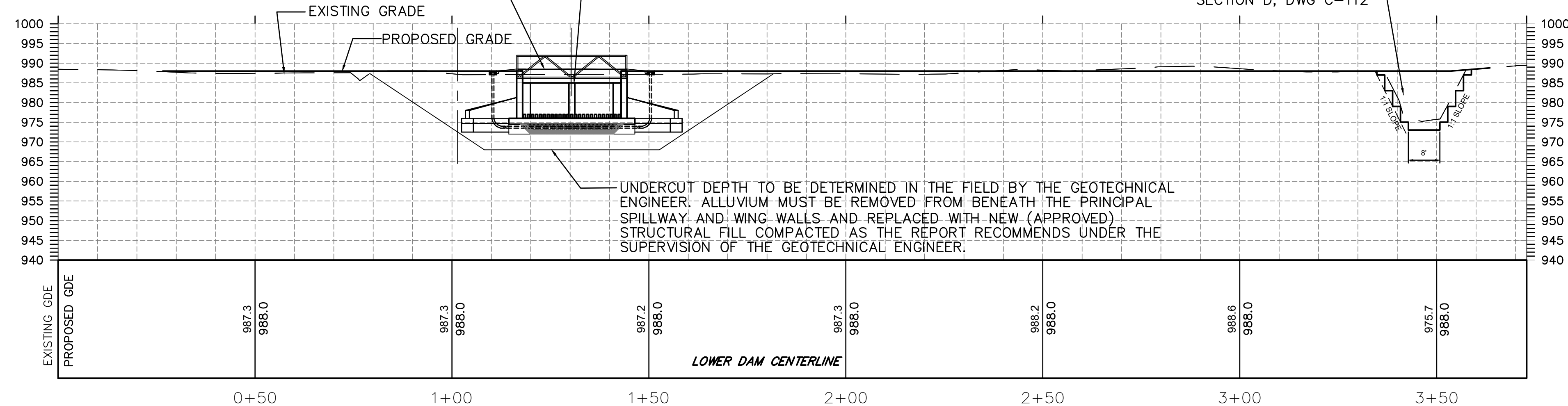
**UPPER DAM CENTERLINE PROFILE**

SCALE: 1"=20' HORIZ. AND VERT.

PEDESTRIAN BRIDGE W/ MIN. 42" BETWEEN HANDRAILS ON REINF. CONCRETE (SUPPORT) SLABS ON EA. SIDE; DESIGNED BY OTHERS. SEE BRIDGE NOTES ON DWG. C-100.

LABYRINTH SPILLWAY, SEE DWG S-303 LOWER SPILLWAY FOR PLAN AND SECTION, FOR ADDITIONAL INFORMATION

EXISTING CHANNEL TO BE FILLED IN, SEE SECTION D, DWG C-112



**LOWER DAM CENTERLINE PROFILE**

SCALE: 1"=20' HORIZ. AND VERT.



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SCALE: AS SHOWN      DATE: 01/23  
 DESIGN BY: JR/JH  
 DRAWN BY: JH/EC  
 CHECKED BY: JR  
 CD# PEF 000707      EXP: 06/30/2024  
 JASON RAPPLEAN, PE, EDR  
 LEVEL 2 CERTIFICATION # 3031  
 EXPIRATION DATE: 12/07/26  
 06/19/2024

**JOHNS HOMESTEAD PARK  
 DAM RENOVATION  
 UPPER AND LOWER POND - CENTERLINE SECTIONS**

**CITY OF TUCKER**

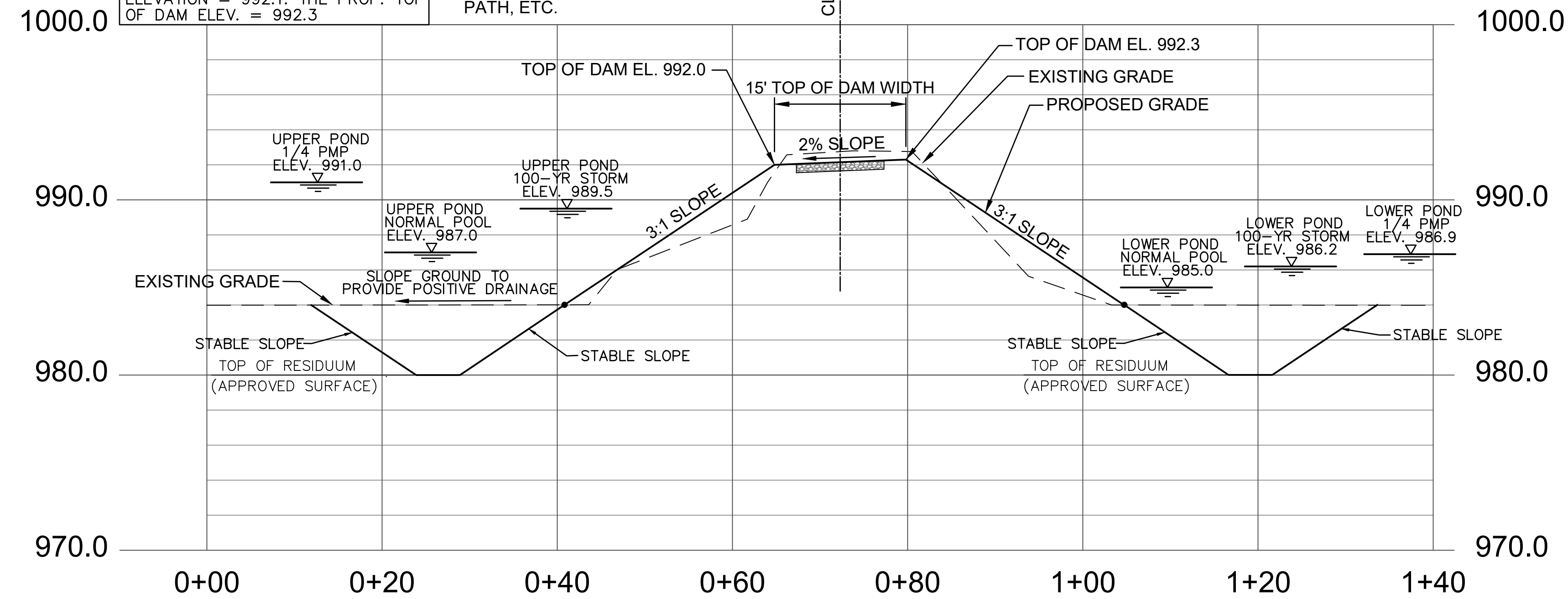
**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
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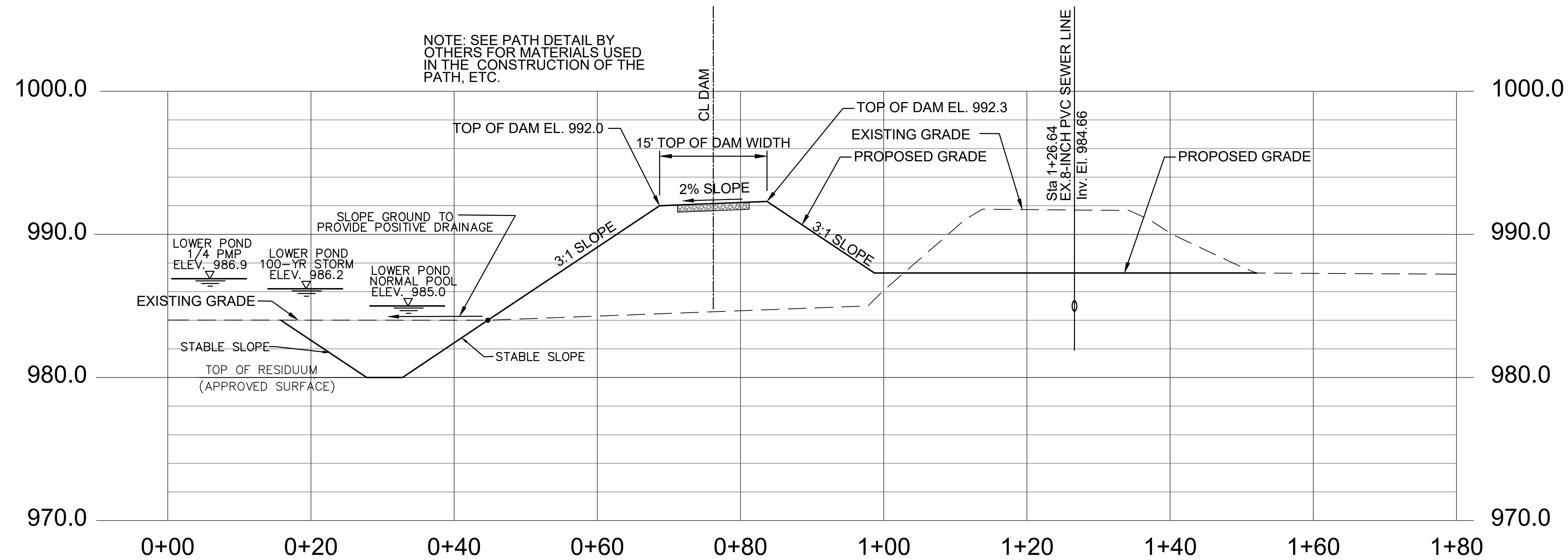


FREEBOARD NOTE:  
TR56 METHOD SMOOTH SLOPE  
REQUIRED FREEBOARD IS 1.02 FT.  
THE 1/4 PMP EL (991.0) + 1.02'  
MAKES THE MIN. TOP OF DAM  
ELEVATION = 992.1. THE PROP. TOP  
OF DAM ELEV. = 992.3

NOTE: SEE PATH DETAIL BY  
OTHERS FOR MATERIALS USED  
IN THE CONSTRUCTION OF THE  
PATH, ETC.



**A** SECTION A  
SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V



**B** SECTION B  
SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V

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NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS



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CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE:	12/07/26

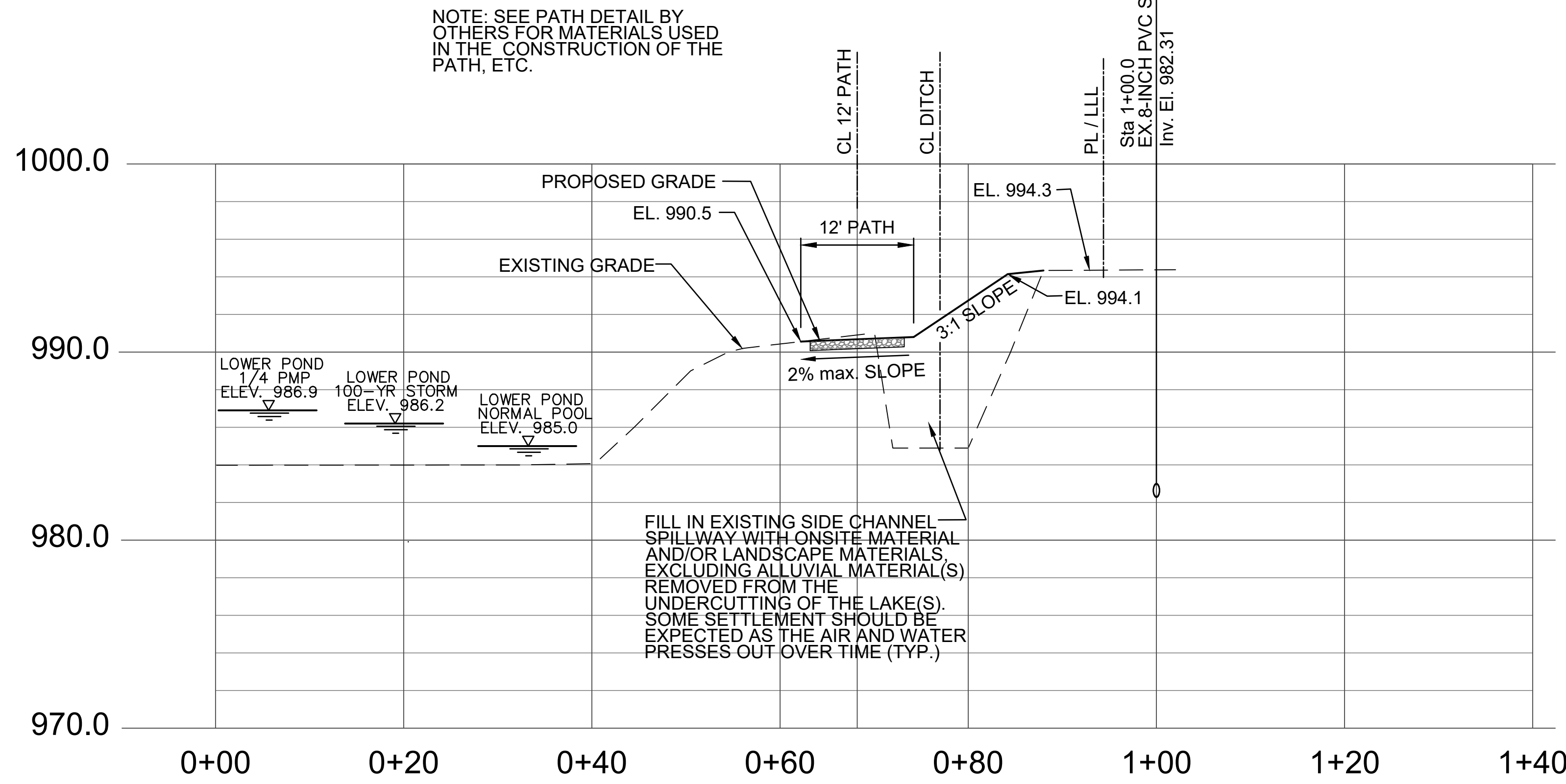
**JOHNS HOMESTEAD PARK  
DAM RENOVATION  
UPPER POND - SECTIONS**

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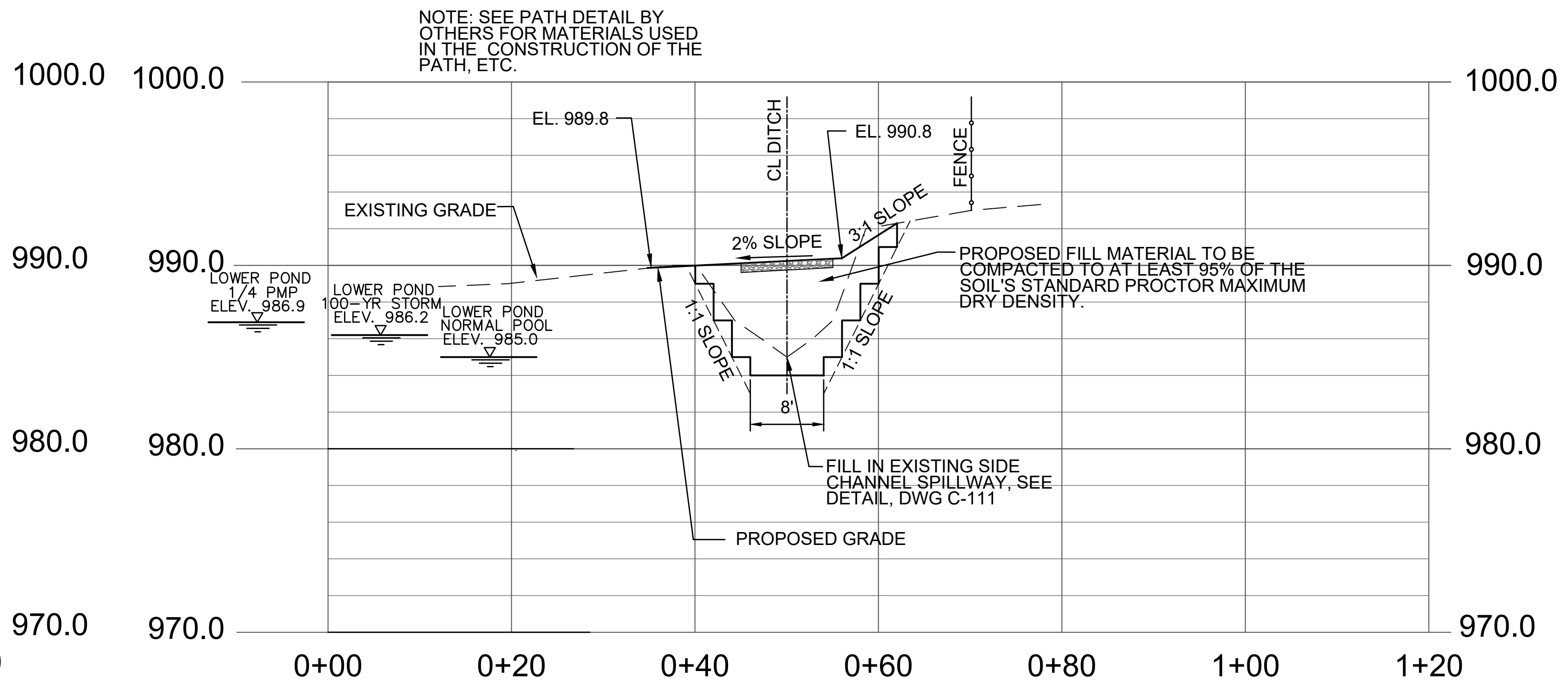
**CITY OF TUCKER**

4201600 **42016- C-109** **0**

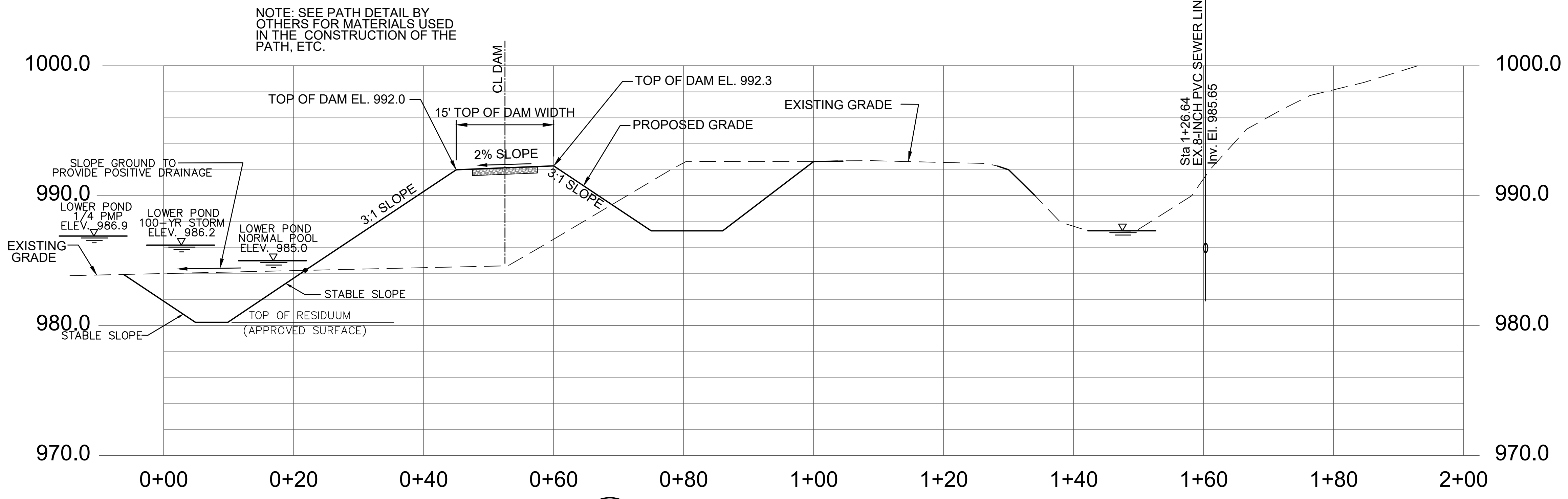




**C SECTION C**  
SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V



**E SECTION E**  
SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V



**D SECTION D**  
SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V



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DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE:	12/07/26

**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**UPPER POND - SECTIONS**

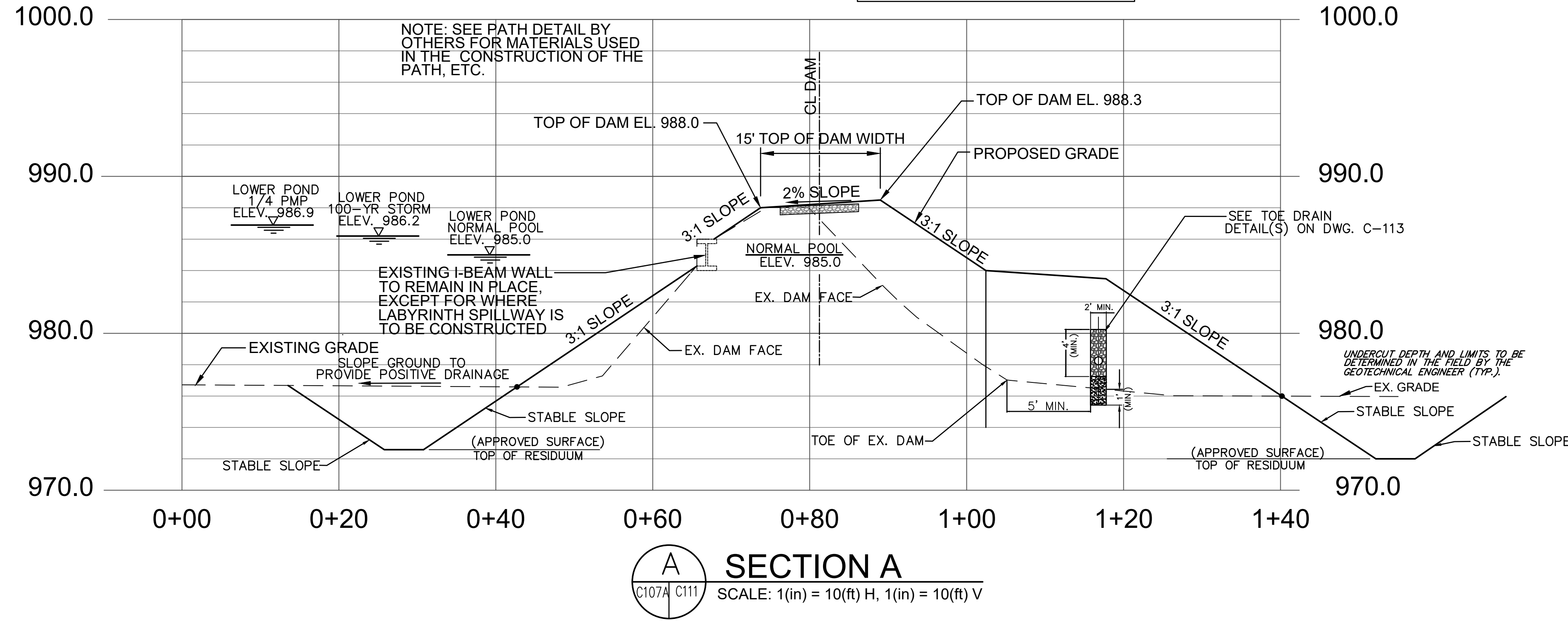
**CITY OF TUCKER**

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4201600 **42016- C-110** **0**



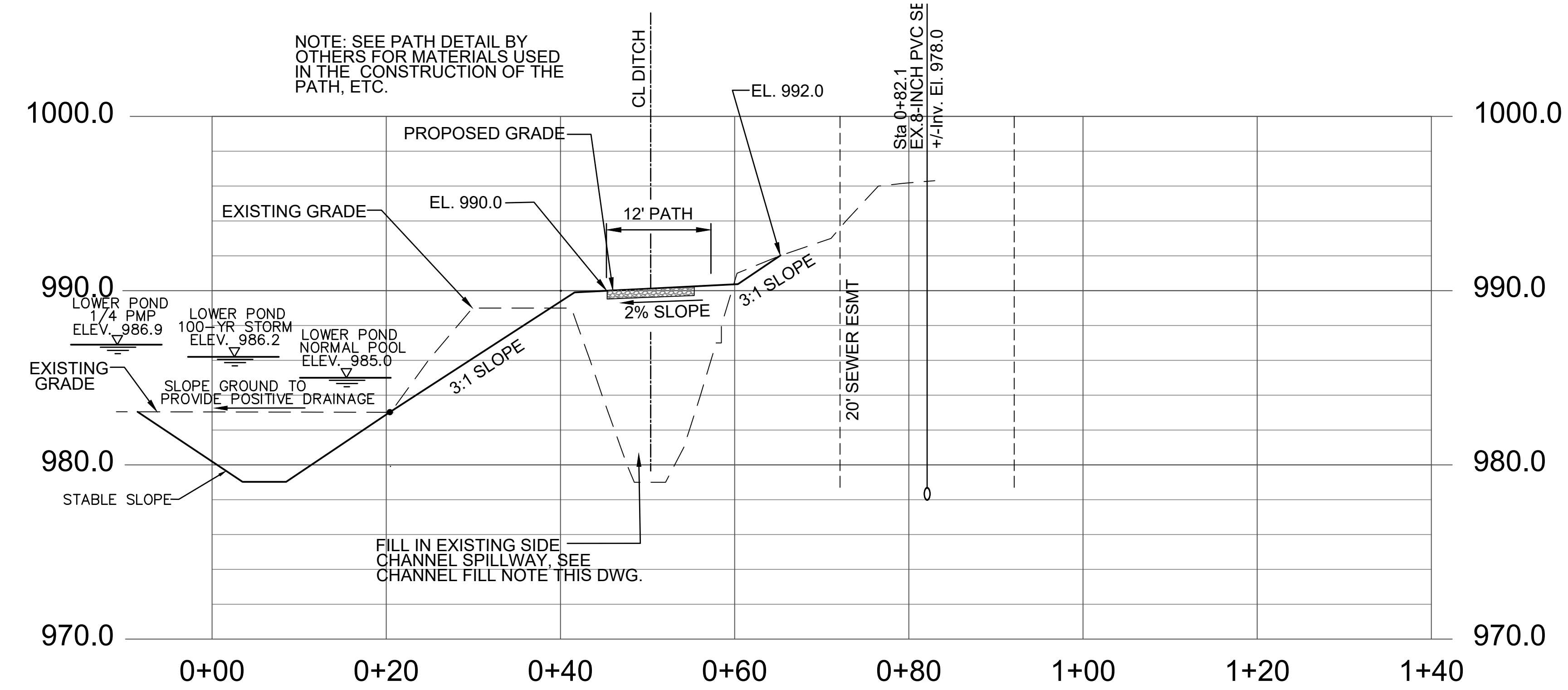
**FREEBOARD NOTE:**  
 TR56 METHOD SMOOTH SLOPE  
 REQUIRED FREEBOARD IS 1.02 FT.  
 THE 1/4 PMP EL (986.94) + 1.14'  
 MAKES THE MIN. TOP OF DAM  
 ELEVATION = 988.08. THE PROP.  
 TOP OF DAM ELEV. = 988.3



**A SECTION A**  
 C107A C111 SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V

**SIDE CHANNEL FILL NOTES**

1. A portion of the side channel spillway should be filled for dam safety and must be done while monitored by the Geotechnical Engineer. This portion is of monitored fill in the side channel spillway is from 50 feet downstream of the Upper Dam(s) centerline to 50 feet of the Lower Dam(s) centerline. This portion of the side channel spillway will become a part of the Upper and Lower Dams and must be constructed and filled as if they are a part of those portions of those dams.
2. The surface water must be stopped, and groundwater control implemented as discussed in the Geotechnical Report prepared for the two dams by Atlas Technical Consultants, LLC. The length(s) of channel should have the side slopes flattened as described on plan sheets C-110 (Section E) and C-112 (section D); and the bottoms need to be widened to allow operation of compaction equipment.
3. Approved select fill should be placed and compacted as described in Atlas' report's sections. New fill shall be benched into the sides of the channel, as shown on those plan sheets.
4. The rest of the channel's fill may come from onsite material or from material that would be considered "landscape fills" not suitable for use as dam select fill.



**B SECTION B**  
 C107A C111 SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V



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 4898 LAVISTA RD, TUCKER, GA 30084  
 CONTACT: RIP ROBERTSON, DIRECTOR  
 PHONE: (470) 481-0205  
 EMAIL: rrobertson@tuckerga.gov

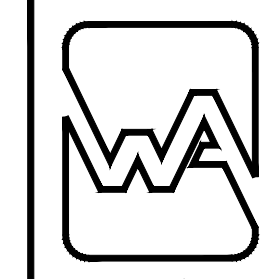
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DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE:	12/07/26

**JOHNS HOMESTEAD PARK**  
**DAM RENOVATION**  
**LOWER POND - SECTIONS**



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 CONSULTING ENGINEERS  
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 (770) 956 - 7879

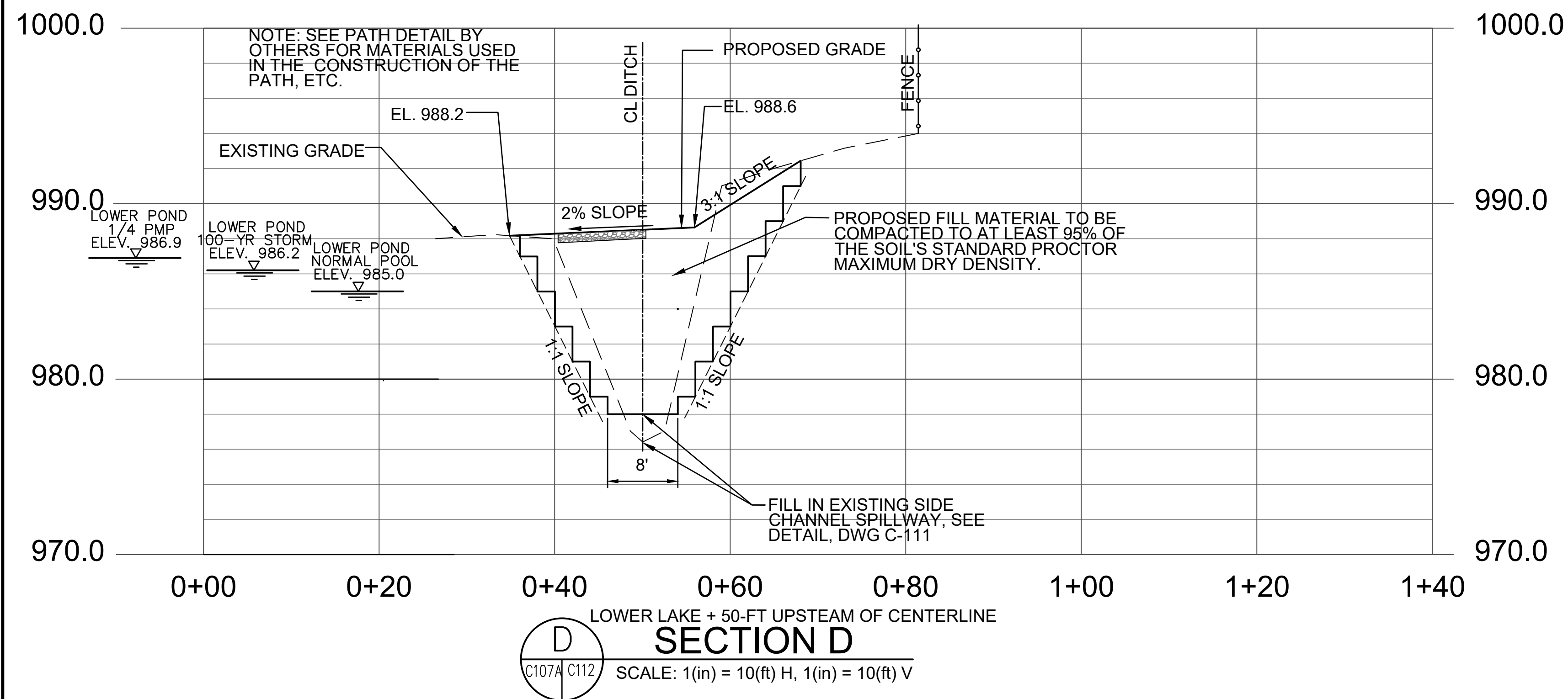
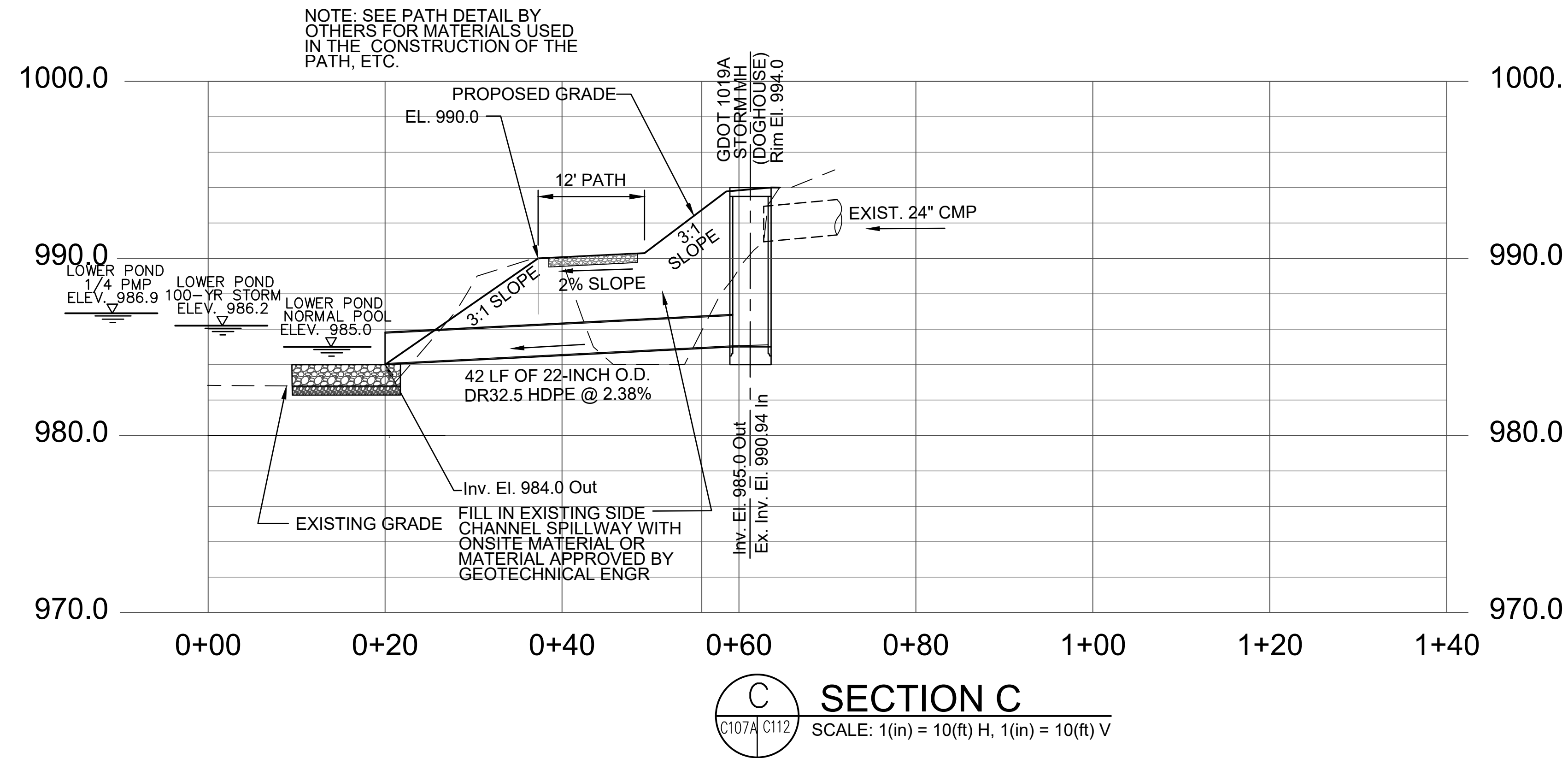
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 JASON F. RAPPLEAN

06/19/2024

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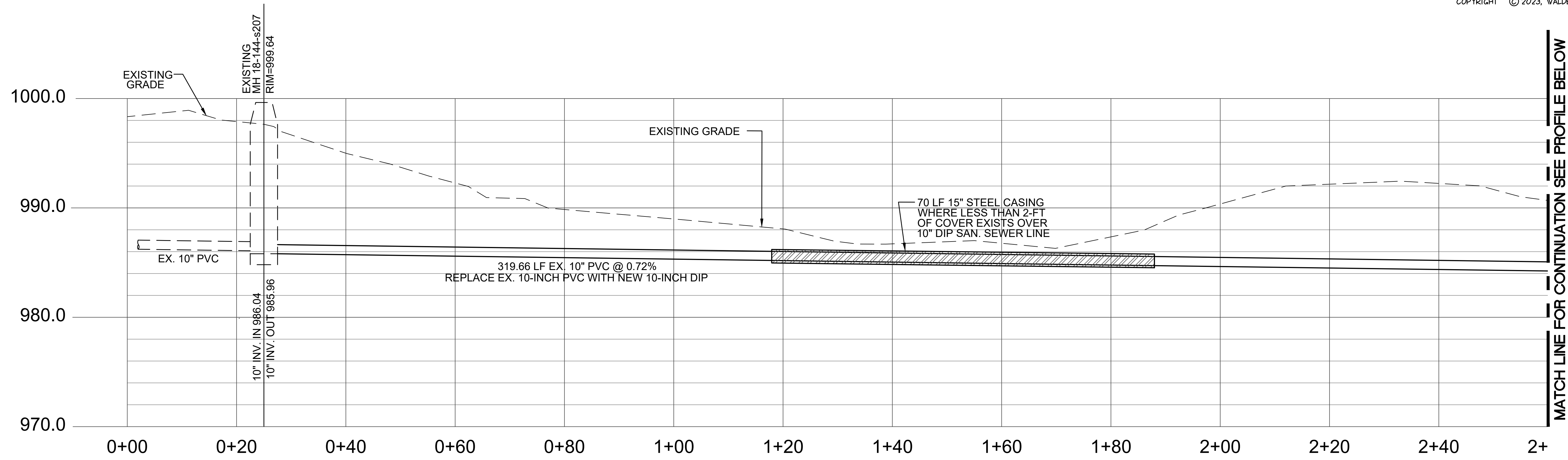
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 DAM RENOVATION  
 LOWER POND - SECTIONS AND DETAILS**

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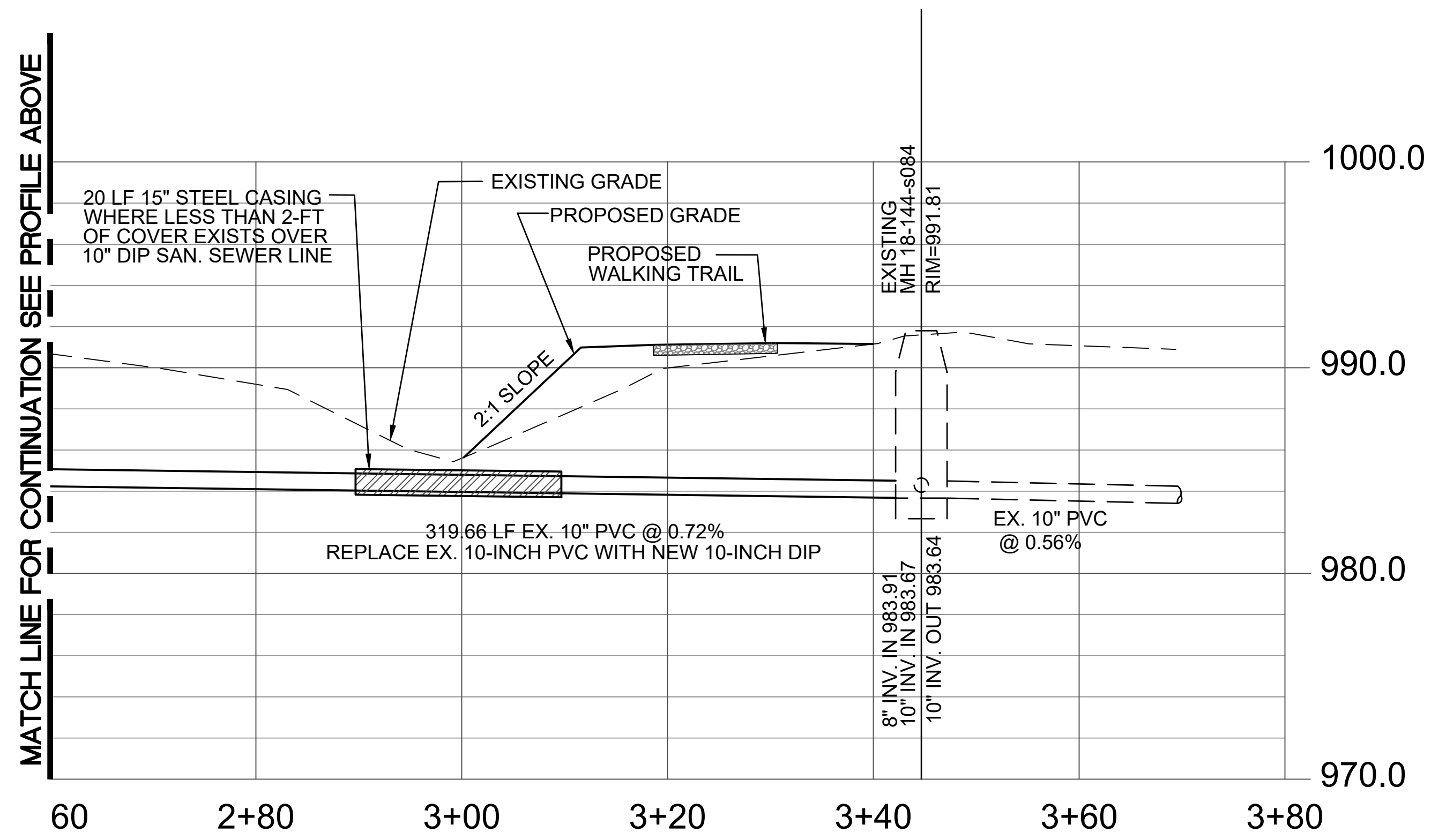
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**F** SECTION F - SAN. SEWER PROFILE  
 SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V



**F** SECTION F - SAN. SEWER PROFILE  
 SCALE: 1(in) = 10(ft) H, 1(in) = 10(ft) V



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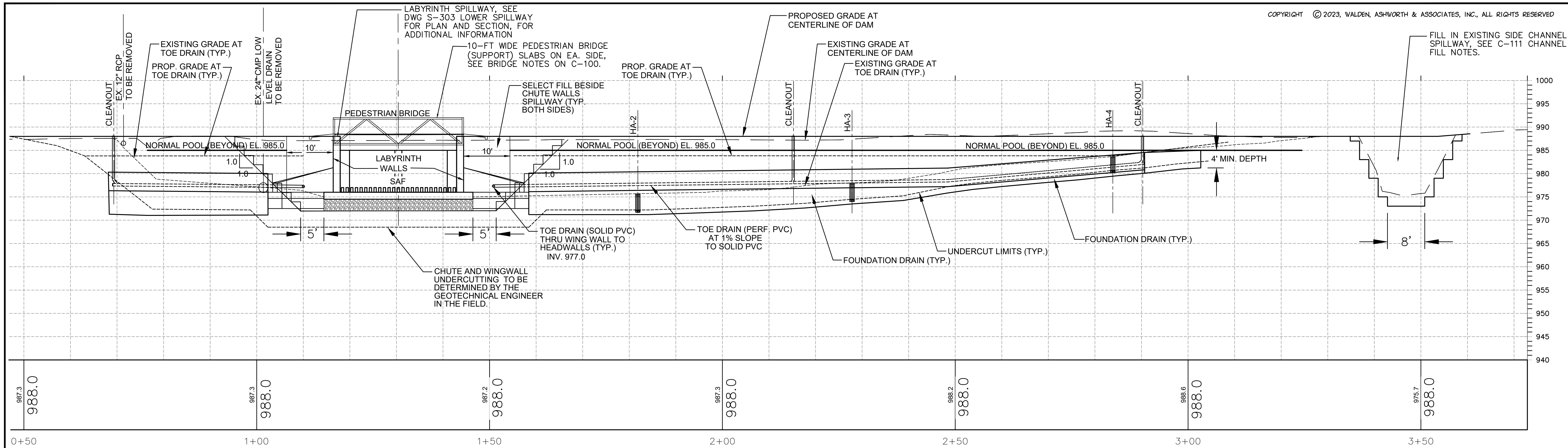
**JOHNS HOMESTEAD PARK  
 DAM RENOVATION  
 SANITARY SEWER PROFILE**

**CITY OF TUCKER**

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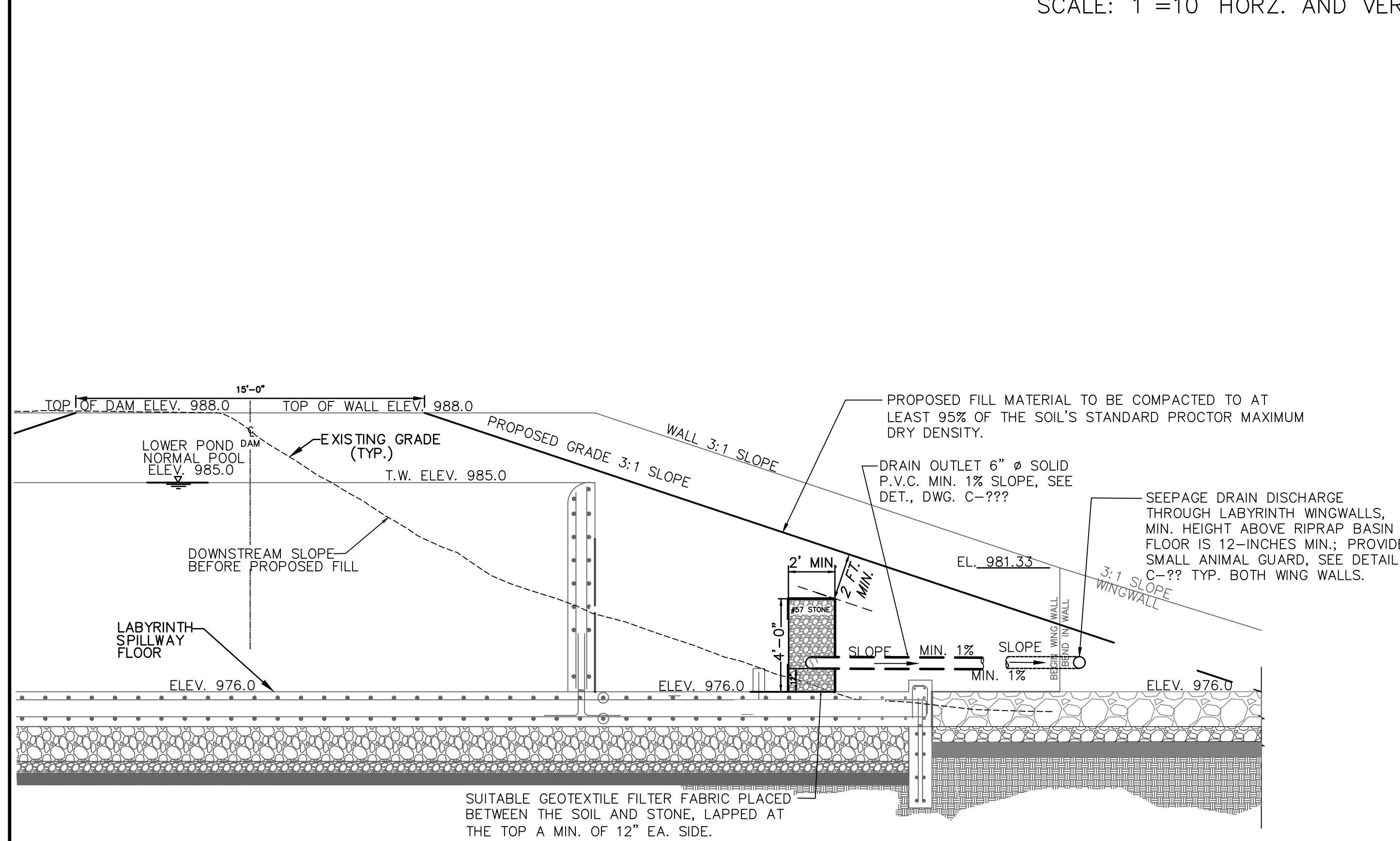
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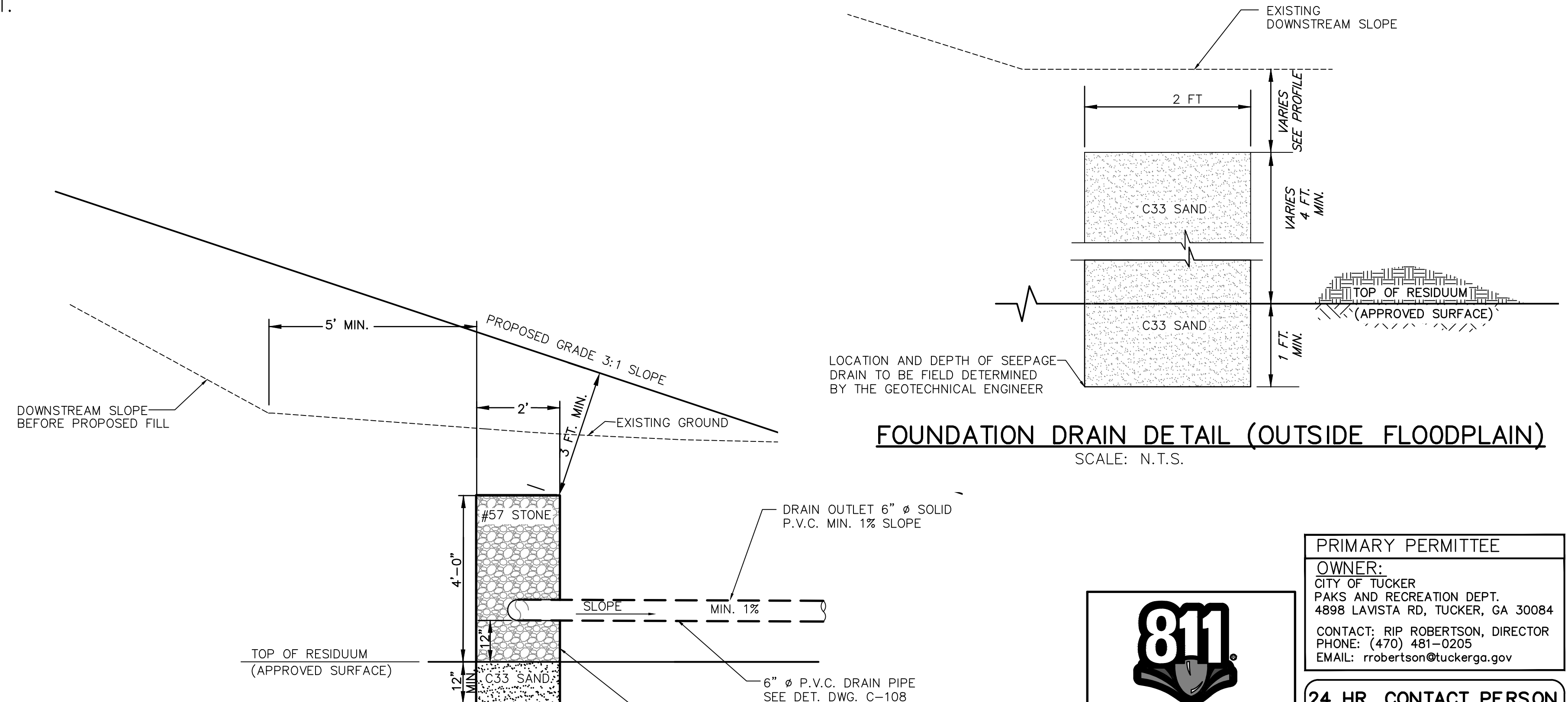
**TOE / FOUNDATION DRAIN PROFILE**

SCALE: 1"=10' HORZ. AND VERT.



**TYPICAL DRAIN DETAIL**

SCALE: 1/4"=1'-0"



**FOUNDATION DRAIN DETAIL (OUTSIDE FLOODPLAIN)**

SCALE: N.T.S.

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EXPIRATION DATE:	12/07/26

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DAM RENOVATION  
LOWER POND - SECTIONS AND DETAILS**

**CITY OF TUCKER**



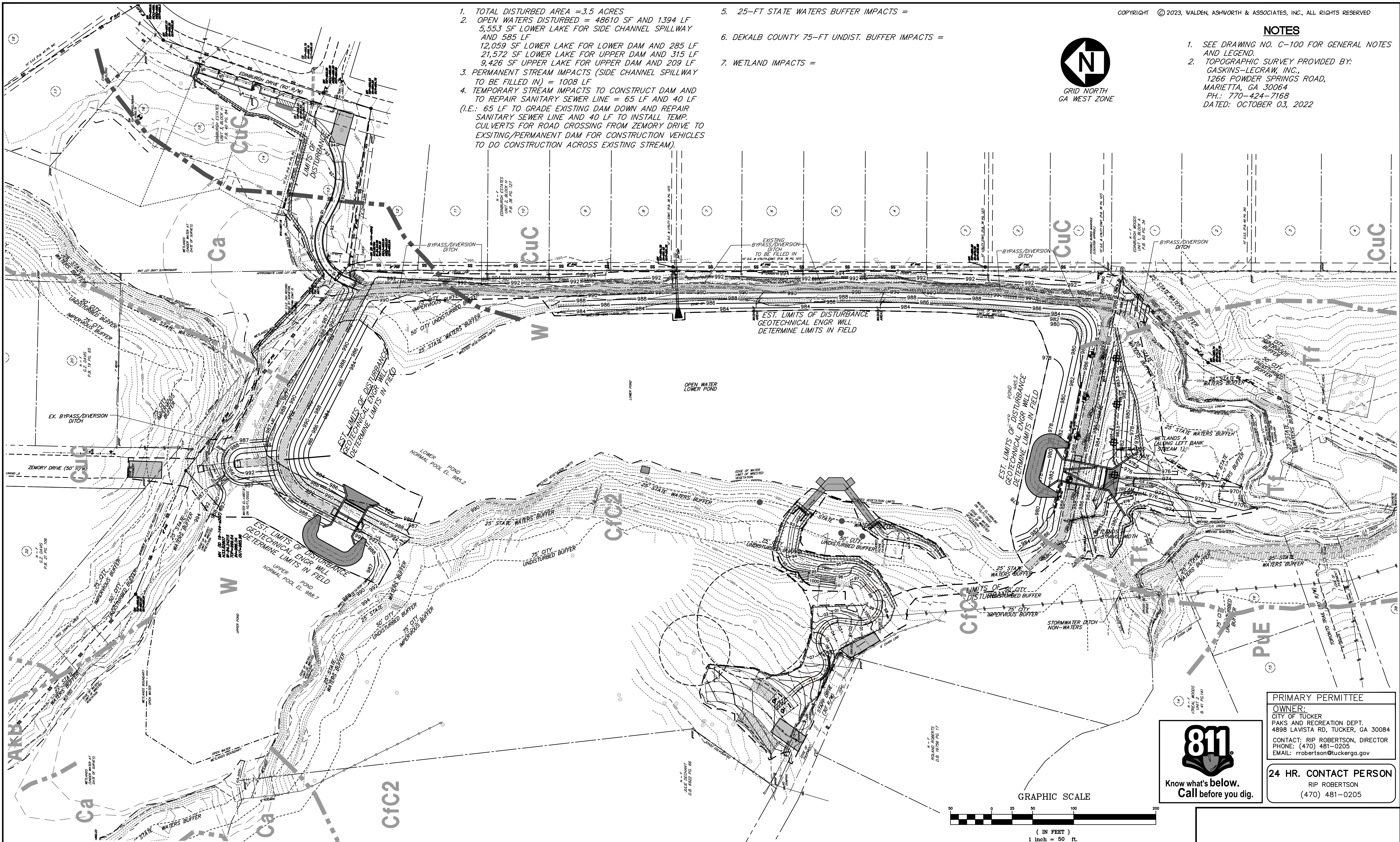
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1. TOTAL DISTURBED AREA = 3.5 ACRES
2. OPEN WATERS DISTURBED = 48610 SF AND 1394 LF  
5,553 SF LOWER LAKE FOR SIDE CHANNEL SPILLWAY AND 585 LF  
12,059 SF LOWER LAKE FOR LOWER DAM AND 285 LF  
21,572 SF LOWER LAKE FOR UPPER DAM AND 315 LF  
9,426 SF UPPER LAKE FOR UPPER DAM AND 209 LF
3. PERMANENT STREAM IMPACTS (SIDE CHANNEL SPILLWAY TO BE FILLED IN) = 1008 LF
4. TEMPORARY STREAM IMPACTS TO CONSTRUCT DAM AND TO REPAIR SANITARY SEWER LINE = 65 LF AND 40 LF  
(I.E.: 65 LF TO GRADE EXISTING DAM DOWN AND REPAIR SANITARY SEWER LINE AND 40 LF TO INSTALL TEMP. CULVERTS FOR ROAD CROSSING FROM ZEMORY DRIVE TO EXSITING/PERMANENT DAM FOR CONSTRUCTION VEHICLES TO DO CONSTRUCTION ACROSS EXISTING STREAM).
5. 25-FT STATE WATERS BUFFER IMPACTS =
6. DEKALB COUNTY 75-FT UNDIST. BUFFER IMPACTS =
7. WETLAND IMPACTS =



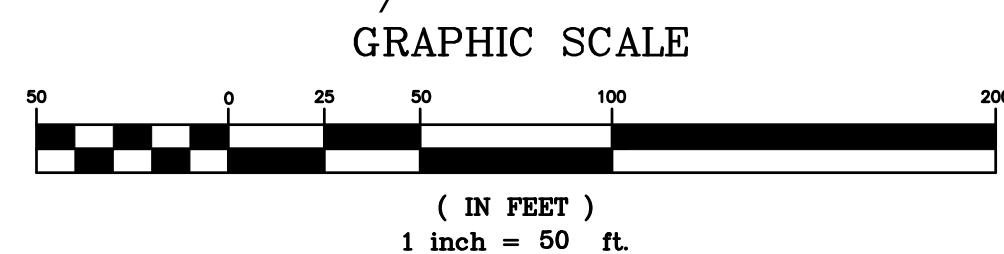
**NOTES**

1. SEE DRAWING NO. C-100 FOR GENERAL NOTES AND LEGEND.
2. TOPOGRAPHIC SURVEY PROVIDED BY: GASKINS-LECRAW, INC., 1266 POWDER SPRINGS ROAD, MARIETTA, GA 30064 PH.: 770-424-7168 DATED: OCTOBER 03, 2022

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JASON RAPPLEAN, PE, EDR	3091		
LEVEL 2 CERTIFICATION #	3091		
EXPIRATION DATE:	12/07/26		

**JOHNS HOMESTEAD PARK**  
**ENVIRONMENTAL EXHIBITS**  
**STATE WATERS AND COUNTY BUFFER VARIANCE EXHIBIT**

**CITY OF TUCKER AND DEKALB COUNTY**

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**EROSION, SEDIMENTATION & POLLUTION CONTROL INSPECTION NOTES:**

1. THE PRIMARY PERMITEE AND TERTIARY PERMITEE(S) MUST RETAIN THE DESIGN PROFESSIONAL WHO PREPARED THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN, EXCEPT WHEN THE PERMITEE HAS REQUESTED IN WRITING AND EPD HAS AGREED TO AN ALTERNATE DESIGN PROFESSIONAL, TO INSPECT THE INSTALLATION OF THE INITIAL SEDIMENT STORAGE REQUIREMENTS AND PERMETER CONTROL BMPs WHICH THE DESIGN PROFESSIONAL DESIGNED WITHIN SEVEN (7) DAYS AFTER INSTALLATION. THE DESIGN PROFESSIONAL SHALL DETERMINE IF THESE BMPs HAVE BEEN INSTALLED AND ARE BEING MAINTAINED AS DESIGNED. THE DESIGN PROFESSIONAL SHALL REPORT THE RESULTS OF THE INSPECTION TO THE PERMITEE WITHIN SEVEN (7) DAYS AND THE PERMITEE MUST CORRECT ALL DEFICIENCIES WITHIN TWO (2) BUSINESS DAYS OR RECEIPT OF THE INSPECTION REPORT FROM THE DESIGN PROFESSIONAL UNLESS WEATHER RELATED SITE CONDITIONS ARE SUCH THAT ADDITIONAL TIME IS REQUIRED.

2. THE PRIMARY, SECONDARY OR TERTIARY PERMITEES, AS APPLICABLE, SHALL AMEND THEIR PLANS WHENEVER THERE IS A CHANGE IN DESIGN, CONSTRUCTION, OPERATION, OR MAINTENANCE, WHICH HAS A SIGNIFICANT EFFECT ON BMPs WITH A HYDRAULIC COMPONENT. AMENDMENTS TO THE PLANS MUST BE CERTIFIED BY THE DESIGN PROFESSIONAL. ALL REVISIONS OR AMENDMENTS SHALL BE SUBMITTED TO THE LOCAL ISSUING AUTHORITY FOR REVIEW.

3. INSPECTIONS:  
A. PERMITEE REQUIREMENTS

(1). EACH DAY WHEN ANY TYPE OF CONSTRUCTION ACTIVITY HAS TAKEN PLACE AT A PRIMARY PERMITEE'S SITE, CERTIFIED PERSONNEL PROVIDED BY THE PRIMARY PERMITEE SHALL INSPECT: (A) ALL AREAS AT THE PRIMARY PERMITEE'S SITE WHERE PETROLEUM PRODUCTS ARE STORED, USED, OR HANDLED FOR SPILLS AND LEAKS FROM VEHICLES AND EQUIPMENT AND (B) ALL LOCATIONS AT THE PRIMARY PERMITEE'S SITE WHERE VEHICLES ENTER OR EXIT THE SITE FOR EVIDENCE OF OFF SITE SEDIMENT TRACKING. THESE INSPECTIONS MUST BE CONDUCTED UNTIL A NOTICE OF TERMINATION IS SUBMITTED.

(2). MEASURE AND RECORD RAINFALL WITHIN DISTURBED AREA OF THE SITE THAT HAVE NOT MET FINAL STABILIZATION ONCE EVERY 24 HOURS EXCEPT ANY NON-WORKING SUNDAY AND NON-WORKING FEDERAL HOLIDAY. THE DATA COLLECTED FOR THE PURPOSE OF COMPLIANCE WITH THIS PERMIT SHALL BE REPRESENTATIVE OF THE MONITORED ACTIVITY. MEASUREMENT OF RAINFALL MAY BE SUSPENDED IF ALL AREAS OF THE SITE HAVE UNDERGONE FINAL STABILIZATION OR ESTABLISHED A CROP OF ANNUAL VEGETATION AND A SEEDING OF TARGET PERENNIALS APPROPRIATE FOR THE REGION.

(3). CERTIFIED PERSONNEL (PROVIDED BY THE PRIMARY PERMITEE) SHALL INSPECT THE FOLLOWING AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF THE END OF A STORM THAT IS 0.5 INCHES RAINFALL OR GREATER (UNLESS SUCH STORM ENDS AFTER 5:00 PM ON ANY FRIDAY OR ON ANY NON-WORKING SATURDAY, NON-WORKING SUNDAY OR ANY NON-WORKING FEDERAL HOLIDAY IN WHICH CASE THE INSPECTION SHALL BE COMPLETED BY THE END OF THE NEXT BUSINESS DAY AND/OR WORKING DAY, WHICHEVER OCCURS FIRST): (A) DISTURBED AREAS OF THE PRIMARY PERMITEE'S CONSTRUCTION SITE ; (B) AREAS USED BY THE PRIMARY PERMITEE FOR STORAGE OF MATERIALS THAT ARE EXPOSED TO PRECIPITATION ; AND (C) STRUCTURAL CONTROL MEASURES, EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN APPLICABLE TO THE PRIMARY PERMITEE'S SITE SHALL BE OBSERVED TO ENSURE THAT THEY ARE OPERATING CORRECTLY. WHERE DISCHARGE LOCATIONS OR POINTS ARE ACCESSIBLE, THEY SHALL BE INSPECTED TO ASCERTAIN WHETHER EROSION CONTROL MEASURES ARE EFFECTIVE IN PREVENTING SIGNIFICANT IMPACTS TO RECEIVING WATER(S). FOR AREAS OF A SITE THAT HAVE UNDERGONE FINAL STABILIZATION OR ESTABLISHED A CROP OF ANNUAL VEGETATION AND A SEEDING OF TARGET PERENNIALS APPROPRIATE FOR THE REGION, THE PERMITEE MUST COMPLY WITH PART [V.D.4.A.(4)]. THESE INSPECTIONS MUST BE CONDUCTED UNTIL A NOTICE OF TERMINATION IS SUBMITTED.

(4). CERTIFIED PERSONNEL (PROVIDED BY THE PRIMARY PERMITEE) SHALL INSPECT AT LEAST ONCE PER MONTH DURING THE TERM OF THIS PERMIT (I.E., UNTIL A NOTICE OF TERMINATION HAS BEEN SUBMITTED) THE AREAS OF THE SITE THAT HAVE UNDERGONE FINAL STABILIZATION OR ESTABLISHED A CROP OF ANNUAL VEGETATION AND A SEEDING OF TARGET PERENNIALS APPROPRIATE FOR THE REGION. THESE AREAS SHALL BE INSPECTED FOR EVIDENCE OF OR THE POTENTIAL FOR POLLUTANTS ENTERING THE DRAINAGE SYSTEM AND THE RECEIVING WATER(S). EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN SHALL BE OBSERVED TO ENSURE THAT THEY ARE OPERATING CORRECTLY. WHERE DISCHARGE LOCATIONS OR POINTS ARE ACCESSIBLE, THEY SHALL BE INSPECTED TO ASCERTAIN WHETHER EROSION CONTROL MEASURES ARE EFFECTIVE IN PREVENTING SIGNIFICANT IMPACTS TO RECEIVING WATER(S).

(5). BASED ON THE RESULTS OF EACH INSPECTION, THE SITE DESCRIPTION AND THE POLLUTION PREVENTION AND CONTROL MEASURES IDENTIFIED IN THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN, THE PLAN SHALL BE REVISED AS APPROPRIATE NOT LATER THAN SEVEN (7) CALENDAR DAYS FOLLOWING EACH INSPECTION. IMPLEMENTATION OF SUCH CHANGES SHALL BE MADE AS SOON AS PRACTICAL BUT IN NO CASE LATER THAN SEVEN (7) CALENDAR DAYS FOLLOWING EACH INSPECTION.

(6). A REPORT OF EACH INSPECTION THAT INCLUDES THE NAME(S) OF CERTIFIED PERSONNEL MAKING EACH INSPECTION, THE DATE(S) OF EACH INSPECTION, CONSTRUCTION PHASE (I.E., INITIAL, INTERMEDIATE OR FINAL), MAJOR OBSERVATIONS RELATING TO THE IMPLEMENTATION OF THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN, AND ACTIONS TAKEN IN ACCORDANCE WITH PART [V.D.4.A.(5)]. OF THE PERMIT SHALL BE MADE AND RETAINED AT THE SITE OR BE READILY AVAILABLE AT A DESIGNATED ALTERNATE LOCATION UNTIL THE ENTIRE SITE OR THAT PORTION OF A CONSTRUCTION PROJECT THAT HAS BEEN PHASED HAS UNDERGONE FINAL STABILIZATION AND A NOTICE OF TERMINATION IS SUBMITTED TO EPD. SUCH REPORTS SHALL BE READILY AVAILABLE BY END OF THE SECOND BUSINESS DAY AND/OR WORKING DAY AND SHALL IDENTIFY ALL INCIDENTS OF BEST MANAGEMENT PRACTICES THAT HAVE NOT BEEN PROPERLY INSTALLED AND/OR MAINTAINED AS DESCRIBED IN THE PLAN. WHERE THE REPORT DOES NOT IDENTIFY ANY INCIDENTS, THE INSPECTION REPORT SHALL CONTAIN A CERTIFICATION THAT THE BEST MANAGEMENT PRACTICES ARE IN COMPLIANCE WITH THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN. THE REPORT SHALL BE SIGNED IN ACCORDANCE WITH PART V.G.2. OF THIS PERMIT.

5. MAINTENANCE:  
THE PLAN SHALL INCLUDE A DESCRIPTION OF PROCEDURES TO ENSURE THE TIMELY MAINTENANCE OF VEGETATION, EROSION AND SEDIMENT CONTROL MEASURES AND OTHER PROTECTIVE MEASURES IDENTIFIED IN THE SITE PLAN.

6. SAMPLING REQUIREMENTS:  
THIS PERMIT REQUIRES THE MONITORING OF NEPHELOMETRIC TURBIDITY IN RECEIVING WATER(S) OR OUTFALLS IN ACCORDANCE WITH THIS PERMIT. THIS PARAGRAPH SHALL NOT APPLY TO ANY LAND DISTURBANCE ASSOCIATED WITH THE CONSTRUCTION OF SINGLE-FAMILY HOMES WHICH ARE NOT PART OF A SUBDIVISION OR PLANNED COMMON DEVELOPMENT UNLESS FIVE (5) ACRES OR MORE WILL BE DISTURBED. THE FOLLOWING PROCEDURES CONSTITUTE EPD'S GUIDELINES FOR SAMPLING TURBIDITY.

A. SAMPLING REQUIREMENTS SHALL INCLUDE THE FOLLOWING:

(1) A USGS TOPOGRAPHIC MAP, A TOPOGRAPHIC MAP OR A DRAWING (REFERRED TO AS A TOPOGRAPHIC MAP) THAT IS A SCALE EQUAL TO OR MORE DETAILED THAN A 1:24000 MAP SHOWING THE LOCATION OF THE SITE OR THE STAND ALONE CONSTRUCTION; (A) THE LOCATION OF ALL PERENNIAL AND INTERMITTENT STREAMS AND OTHER WATER BODIES AS SHOWN ON A USGS TOPOGRAPHIC MAP, AND ALL OTHER PERENNIAL AND INTERMITTENT STREAMS AND OTHER WATER BODIES LOCATED DURING MANDATORY FIELD VERIFICATION, INTO WHICH THE STORM WATER IS DISCHARGED AND (B) THE RECEIVING WATER AND/OR OUTFALL SAMPLING LOCATIONS, WHEN THE PERMITEE HAS CHOSEN TO USE A USGS TOPOGRAPHIC MAP AND THE RECEIVING WATER(S) IS NOT SHOWN ON THE USGS TOPOGRAPHIC MAP, THE LOCATION OF THE RECEIVING WATER(S) MUST BE HAND-DRAWN ON THE USGS TOPOGRAPHIC MAP FROM WHERE THE STORM WATER(S) ENTERS THE RECEIVING WATER(S) TO THE POINT WHERE THE RECEIVING WATER(S) COMBINES WITH THE FIRST BLUE LINE STREAM SHOWN ON THE USGS TOPOGRAPHIC MAP.

(2). A WRITTEN NARRATIVE OF SITE SPECIFIC ANALYTICAL METHODS USED TO COLLECT, HANDLE AND ANALYZE THE SAMPLES INCLUDING QUALITY CONTROL/QUALITY ASSURANCE PROCEDURES. THIS NARRATIVE MUST INCLUDE PRECISE SAMPLING METHODOLOGY FOR EACH SAMPLING LOCATION;

(3). WHEN THE PERMITEE HAS DETERMINED THAT SOME OR ALL OUTFALLS WILL BE SAMPLED, A RATIONALE MUST BE INCLUDED ON THE PLAN FOR THE NTU LIMIT(S) SELECTED FROM APPENDIX B. THIS RATIONALE MUST INCLUDE THE SIZE OF THE CONSTRUCTION SITE, THE CALCULATION OF THE SIZE OF THE SURFACE WATER DRAINAGE AREA, AND THE TYPE OF RECEIVING WATER(S) (I.E., TROUT STREAM OR SUPPORTING WARM WATER FISHERIES); AND

(4). ANY ADDITIONAL INFORMATION EPD DETERMINES NECESSARY TO BE PART OF THE PLAN. EPD WILL PROVIDE WRITTEN NOTICE TO THE PERMITEE OF THE INFORMATION NECESSARY AND THE TIME LINE FOR SUBMITTAL.

B. SAMPLE TYPE. ALL SAMPLING SHALL BE COLLECTED BY GRAB SAMPLES\* AND THE ANALYSIS OF THESE SAMPLES MUST BE CONDUCTED IN ACCORDANCE WITH METHODOLOGY AND TEST PROCEDURES ESTABLISHED BY 40 CFR PART 136 (UNLESS OTHER TEST PROCEDURES HAVE BEEN APPROVED); THE GUIDANCE DOCUMENT TITLED "NPDES STORM WATER SAMPLING GUIDANCE DOCUMENT, EPA 833-B-92 001" AND GUIDANCE DOCUMENTS THAT MAY BE PREPARED BY THE EPD.

(1 ). SAMPLE CONTAINERS SHOULD BE LABELED PRIOR TO COLLECTING THE SAMPLES.

(2). SAMPLES SHOULD BE WELL MIXED BEFORE TRANSFERRING TO A SECONDARY CONTAINER.

(3). LARGE MOUTH, WELL CLEANED AND RINSED GLASS OR PLASTIC JARS SHOULD BE USED FOR COLLECTING SAMPLES. THE JARS SHOULD BE CLEANED THOROUGHLY TO AVOID CONTAMINATION.

(4). MANUAL, AUTOMATIC OR RISING STAGE SAMPLING MAY BE UTILIZED. SAMPLES REQUIRED BY THIS PERMIT SHOULD BE ANALYZED IMMEDIATELY, BUT IN NO CASE LATER THAN 48 HOURS AFTER COLLECTION. HOWEVER, SAMPLES FROM AUTOMATIC SAMPLERS MUST BE COLLECTED NO LATER THAN THE NEXT BUSINESS DAY AFTER THEIR ACCUMULATION, UNLESS FLOW THROUGH AUTOMATED ANALYSIS IS UTILIZED. IF AUTOMATIC SAMPLING IS UTILIZED AND THE AUTOMATIC SAMPLER IS NOT ACTIVATED DURING THE QUALIFYING EVENT, THE PERMITEE MUST UTILIZE MANUAL SAMPLING OR RISING STAGE SAMPLING DURING THE NEXT QUALIFYING EVENT. DILUTION OF SAMPLES IS NOT REQUIRED. SAMPLES MAY BE ANALYZED DIRECTLY WITH A PROPERLY CALIBRATED TURBIDIMETER. SAMPLES ARE NOT REQUIRED TO BE COOLED.

(5). SAMPLING AND ANALYSIS OF THE RECEIVING WATER(S) OR OUTFALLS BEYOND THE MINIMUM FREQUENCY STATED IN THIS PERMIT MUST BE REPORTED TO EPD AS SPECIFIED IN PART IV.E.

C. SAMPLING POINTS

(1). FOR CONSTRUCTION ACTIVITIES THE PRIMARY PERMITEE MUST SAMPLE ALL RECEIVING WATER(S), OR ALL OUTFALL(S), OR A COMBINATION OF RECEIVING WATER(S) AND OUTFALL(S). SAMPLES TAKEN FOR THE PURPOSE OF COMPLIANCE WITH THIS PERMIT SHALL BE REPRESENTATIVE OF THE MONITORED ACTIVITY AND REPRESENTATIVE OF THE WATER QUALITY OF THE RECEIVING WATER(S) AND/OR THE STORM WATER OUTFALLS USING THE FOLLOWING MINIMUM GUIDELINES:

(A). THE UPSTREAM SAMPLE FOR EACH RECEIVING WATER(S) MUST BE TAKEN IMMEDIATELY UPSTREAM OF THE CONFLUENCE OF THE FIRST STORM WATER DISCHARGE FROM THE PERMITTED ACTIVITY (I.E., THE DISCHARGE FARTHEST UPSTREAM AT THE SITE) BUT DOWNSTREAM OF ANY OTHER STORM WATER DISCHARGES NOT ASSOCIATED WITH THE PERMITTED ACTIVITY. WHERE APPROPRIATE, SEVERAL UPSTREAM SAMPLES FROM ACROSS THE RECEIVING WATER(S) MAY NEED TO BE TAKEN AND THE ARITHMETIC AVERAGE OF THE TURBIDITY OF THESE SAMPLES USED FOR THE UPSTREAM TURBIDITY VALUE.

(B). THE DOWNSTREAM SAMPLE FOR EACH RECEIVING WATER(S) MUST BE TAKEN DOWNSTREAM OF THE CONFLUENCE OF THE LAST STORM WATER DISCHARGE FROM THE PERMITTED ACTIVITY (I.E., THE DISCHARGE FARTHEST DOWNSTREAM AT THE SITE) BUT UPSTREAM OF ANY OTHER STORM WATER DISCHARGE NOT ASSOCIATED WITH THE PERMITTED ACTIVITY. WHERE APPROPRIATE, SEVERAL DOWNSTREAM SAMPLES FROM ACROSS THE RECEIVING WATER(S) MAY NEED TO BE TAKEN AND THE ARITHMETIC AVERAGE OF THE TURBIDITY OF THESE SAMPLES USED FOR THE DOWNSTREAM TURBIDITY VALUE.

(C). IDEALLY THE SAMPLES SHOULD BE TAKEN FROM THE HORIZONTAL AND VERTICAL CENTER OF THE RECEIVING WATER(S) OR THE STORM WATER OUTFALL CHANNEL(S).

(D). CARE SHOULD BE TAKEN TO AVOID STIRRING THE BOTTOM SEDIMENTS IN THE RECEIVING WATER(S) OR IN THE OUTFALL STORM WATER CHANNEL.

(E). THE SAMPLING CONTAINER SHOULD BE HELD SO THAT THE OPENING FACES UPSTREAM.

(F). THE SAMPLES SHOULD BE KEPT FREE FROM FLOATING DEBRIS.

(G). PERMITEES DO NOT HAVE TO SAMPLE SHEET FLOW THAT FLOWS ONTO UNDISTURBED NATURAL AREAS OR AREAS STABILIZED BY THE PROJECT FOR PURPOSES OF THIS SECTION, STABILIZED SHALL MEAN, FOR UNPAVED AREAS AND AREAS NOT COVERED BY PERMANENT STRUCTURES AND AREAS LOCATED OUTSIDE THE WASTE DISPOSAL LIMITS OF A LANDFILL CELL THAT HAS BEEN CERTIFIED BY EPD FOR WASTE DISPOSAL, 100% OF THE SOIL SURFACE IS UNIFORMLY COVERED IN PERMANENT VEGETATION WITH A DENSITY OF 70% OR GREATER, OR LANDSCAPED ACCORDING TO THE PLAN (UNIFORMLY COVERED WITH LANDSCAPING MATERIALS IN PLANNED LANDSCAPED AREAS), OR EQUIVALENT PERMANENT STABILIZATION MEASURES AS DEFINED IN THE MANUAL (EXCLUDING A CROP'S OF ANNUAL VEGETATION AND A SEEDING OF TARGET CROP PERENNIALS APPROPRIATE FOR THE REGION).

(H). ALL SAMPLING PURSUANT TO THIS PERMIT MUST BE DONE IN SUCH A WAY (INCLUDING GENERALLY ACCEPTED SAMPLING METHODS, LOCATIONS, TIMING, AND FREQUENCY) AS TO ACCURATELY REFLECT WHETHER STORM WATER RUNOFF FROM THE CONSTRUCTION SITE IS IN COMPLIANCE WITH THE STANDARD SET FORTH IN PARTS III.D.3. OR III.D.4., WHICHEVER IS APPLICABLE.

D. SAMPLING FREQUENCY:

(1). THE PRIMARY PERMITEE MUST SAMPLE IN ACCORDANCE WITH THE PLAN AT LEAST ONCE FOR EACH RAINFALL EVENT DESCRIBED BELOW. FOR A QUALIFYING EVENT, THE PERMITEE SHALL SAMPLE AT THE BEGINNING OF ANY STORM WATER DISCHARGE TO A MONITORED RECEIVING WATER AND/OR FROM A MONITORED OUTFALL LOCATION WITHIN IN FORTY-FIVE (45) MINUTES OR AS SOON AS POSSIBLE.

(2). HOWEVER, WHERE MANUAL AND AUTOMATIC SAMPLING ARE IMPOSSIBLE (AS DEFINED IN THIS PERMIT), OR ARE BEYOND THE PERMITEE'S CONTROL, THE PERMITEE SHALL TAKE SAMPLES AS SOON AS POSSIBLE, BUT IN NO CASE MORE THAN TWELVE (12) HOURS AFTER THE BEGINNING OF THE STORM WATER DISCHARGE.

(3). SAMPLING BY THE PERMITEE SHALL OCCUR FOR THE FOLLOWING QUALIFYING EVENTS:

(A). FOR EACH AREA OF THE SITE THAT DISCHARGES TO A RECEIVING WATER OR FROM AN OUTFALL, THE FIRST RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH WITH A STORM WATER DISCHARGE THAT OCCURS DURING NORMAL BUSINESS HOURS AS DEFINED IN THIS PERMIT AFTER ALL CLEARING AND GRUBBING OPERATIONS HAVE BEEN COMPLETED, BUT PRIOR TO COMPLETION OF MASS GRADING OPERATIONS, IN THE DRAINAGE AREA OF THE LOCATION SELECTED AS THE SAMPLING LOCATION;

(B). IN ADDITION TO (A) ABOVE, FOR EACH AREA OF THE SITE THAT DISCHARGES TO A RECEIVING WATER OR FROM AN OUTFALL, THE FIRST RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH WITH A STORM WATER DISCHARGE THAT OCCURS DURING NORMAL BUSINESS HOURS AS DEFINED IN THIS PERMIT EITHER 90 DAYS AFTER THE FIRST SAMPLING EVENT OR AFTER ALL MASS GRADING OPERATIONS HAVE BEEN COMPLETED, BUT PRIOR TO SUBMITTAL OF A NOT, IN THE DRAINAGE AREA OF THE LOCATION SELECTED AS THE SAMPLING LOCATION, WHICHEVER COMES FIRST.

(C). AT THE TIME OF SAMPLING PERFORMED PURSUANT TO (A) AND (B) ABOVE, IF BMPs IN ANY AREA OF THE SITE THAT DISCHARGES TO A RECEIVING WATER OR FROM AN OUTFALL ARE NOT PROPERLY DESIGNED, INSTALLED AND MAINTAINED, CORRECTIVE ACTION SHALL BE DEFINED AND IMPLEMENTED WITHIN TWO (2) BUSINESS DAYS, AND TURBIDITY SAMPLES SHALL BE TAKEN FROM DISCHARGES FROM THAT AREA OF THE SITE FOR EACH SUBSEQUENT RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH DURING NORMAL BUSINESS HOURS\* UNTIL THE SELECTED TURBIDITY STANDARD IS ATTAINED, OR UNTIL POST-STORM EVENT INSPECTIONS DETERMINE THAT BMPs ARE PROPERLY DESIGNED, INSTALLED AND MAINTAINED;

(D). WHERE SAMPLING PURSUANT TO (A), (B) OR (C) ABOVE IS REQUIRED BUT NOT POSSIBLE (OR NOT REQUIRED BECAUSE THERE WAS NO DISCHARGE), THE PERMITEE, IN ACCORDANCE WITH PART [V.D.4.A.(6)], MUST INCLUDE A WRITTEN JUSTIFICATION IN THE INSPECTION REPORT OF WHY SAMPLING WAS NOT PERFORMED. PROVIDING THIS JUSTIFICATION DOES NOT RELIEVE THE PERMITEE OF ANY SUBSEQUENT SAMPLING OBLIGATIONS UNDER (A), (B) OR (C) ABOVE; AND

(E). EXISTING CONSTRUCTION ACTIVITIES, I.E., THOSE THAT ARE OCCURRING ON OR BEFORE THE EFFECTIVE DATE OF THIS PERMIT, THAT HAVE MET THE SAMPLING REQUIRED BY (A) ABOVE SHALL SAMPLE IN ACCORDANCE WITH (B). THOSE EXISTING CONSTRUCTION ACTIVITIES THAT HAVE MET THE SAMPLING REQUIRED BY (B) ABOVE SHALL NOT BE REQUIRED TO CONDUCT ADDITIONAL SAMPLING OTHER THAN AS REQUIRED BY (C) ABOVE.

\* NOTE THAT THE PERMITEE MAY CHOOSE TO MEET THE REQUIREMENTS OF (A) AND (B) ABOVE BY COLLECTING TURBIDITY SAMPLES FROM ANY RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH AND ALLOWS FOR SAMPLING AT ANY TIME OF THE DAY OR WEEK.

E. REPORTING:

1. THE APPLICABLE PERMITEES ARE REQUIRED TO SUBMIT THE SAMPLING RESULTS TO THE EPD AT THE ADDRESS SHOWN IN PART II.C. BY THE FIFTEENTH DAY OF THE MONTH FOLLOWING THE REPORTING PERIOD. REPORTING PERIODS ARE MONTHS DURING WHICH SAMPLES ARE TAKEN IN

ACCORDANCE WITH THIS PERMIT. SAMPLING RESULTS SHALL BE IN A CLEARLY LEGIBLE FORMAT, UPON WRITTEN NOTIFICATION, EPD MAY REQUIRE THE APPLICABLE PERMITEE TO SUBMIT THE SAMPLING RESULTS ON A MORE FREQUENT BASIS. SAMPLING AND ANALYSIS OF ANY STORM WATER DISCHARGE(S) OR THE RECEIVING WATER(S) BEYOND THE MINIMUM FREQUENCY STATED IN THIS PERMIT MUST BE REPORTED IN A SIMILAR MANNER TO THE EPD. THE SAMPLING REPORTS MUST BE SIGNED IN ACCORDANCE WITH PART V.G.2. SAMPLING REPORTS MUST BE SUBMITTED TO EPD USING THE ELECTRONIC SUBMITTAL SERVICE PROVIDED BY EPD. SAMPLING REPORTS MUST BE SUBMITTED TO EPD UNTIL SUCH TIME AS A NOT IS SUBMITTED IN ACCORDANCE WITH PART VI.

2. ALL SAMPLING REPORTS SHALL INCLUDE THE FOLLOWING INFORMATION:

- A. THE RAINFALL AMOUNT, DATE, EXACT PLACE AND TIME OF SAMPLING OR MEASUREMENTS;
B. THE NAME(S) OF THE CERTIFIED PERSONNEL WHO PERFORMED THE SAMPLING AND MEASUREMENTS;
C. THE DATE(S) ANALYSES WERE PERFORMED;
D. THE TIME(S) ANALYSES WERE INITIATED;
E. THE NAME(S) OF THE CERTIFIED PERSONNEL WHO PERFORMED THE ANALYSES;
F. REFERENCES AND WRITTEN PROCEDURES, WHEN AVAILABLE, FOR THE ANALYTICAL TECHNIQUES OR METHODS USED;
G. THE RESULTS OF SUCH ANALYSES, INCLUDING THE BENCH SHEETS, INSTRUMENT READOUTS, COMPUTER DISKS OR TAPES, ETC., USED TO DETERMINE THESE RESULTS;
H. RESULTS WHICH EXCEED 1000 NTU SHALL BE REPORTED AS "EXCEEDS 1000 NTU;" AND
I. CERTIFICATION STATEMENT THAT SAMPLING WAS CONDUCTED AS PER THE PLAN.

3. ALL WRITTEN CORRESPONDENCE REQUIRED BY THIS PERMIT SHALL BE SUBMITTED BY RETURN RECEIPT CERTIFIED MAIL (OR SIMILAR SERVICE) TO THE APPROPRIATE DISTRICT OFFICE OF THE EPD ACCORDING TO THE SCHEDULE IN APPENDIX A OF THIS PERMIT. THE PERMITEE SHALL RETAIN A COPY OF THE PROOF OF SUBMITTAL AT THE CONSTRUCTION SITE OR THE PROOF OF SUBMITTAL SHALL BE READILY AVAILABLE AT A DESIGNATED LOCATION FROM COMMENCEMENT OF CONSTRUCTION UNTIL SUCH TIME AS A NOT IS SUBMITTED IN ACCORDANCE WITH PART VI.

IF AN ELECTRONIC SUBMITTAL IS PROVIDED BY EPD THEN THE WRITTEN CORRESPONDENCE MAY BE SUBMITTED ELECTRONICALLY; IF REQUIRED, A PAPER COPY MUST ALSO BE SUBMITTED BY RETURN RECEIPT CERTIFIED MAIL OR SIMILAR SERVICE.

F. RETENTION OF RECORDS:

1. THE PRIMARY PERMITEE SHALL RETAIN THE FOLLOWING RECORDS AT THE CONSTRUCTION SITE OR THE RECORDS SHALL BE READILY AVAILABLE AT A DESIGNATED ALTERNATE LOCATION FROM COMMENCEMENT OF CONSTRUCTION UNTIL SUCH TIME AS A NOT IS SUBMITTED IN ACCORDANCE WITH PART VI:

- A. A COPY OF ALL NOTICES OF INTENT SUBMITTED TO EPD;
B. A COPY OF THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN REQUIRED BY THIS PERMIT;
C. THE DESIGN PROFESSIONALS REPORT OF THE RESULTS OF THE INSPECTION CONDUCTED IN ACCORDANCE WITH PART LV.A.5. OF THIS PERMIT;
D. A COPY OF ALL SAMPLING INFORMATION, RESULTS, AND REPORTS REQUIRED BY THIS PERMIT;
E. A COPY OF ALL INSPECTION REPORTS GENERATED IN ACCORDANCE WITH PART LV.D.4.A. OF THIS PERMIT;
F. A COPY OF ALL VIOLATION SUMMARIES AND VIOLATION SUMMARY REPORTS GENERATED IN ACCORDANCE WITH PART LLD.2. OF THIS PERMIT; AND
G. DAILY RAINFALL INFORMATION COLLECTED IN ACCORDANCE WITH PART [V.D.4.A.(2)]. OF THIS PERMIT.

2. COPIES OF ALL NOTICES OF INTENT, NOTICES OF TERMINATION, INSPECTION REPORTS, SAMPLING REPORTS (INCLUDING ALL CALIBRATION AND MAINTENANCE RECORDS AND ALL ORIGINAL STRIP CHART RECORDINGS FOR CONTINUOUS MONITORING INSTRUMENTATION) OR OTHER REPORTS REQUESTED BY THE EPD, EROSION, SEDIMENTATION AND POLLUTION CONTROL PLANS, RECORDS OF ALL DATA USED TO COMPLETE THE NOTICE OF INTENT TO BE COVERED BY THIS PERMIT AND ALL OTHER RECORDS REQUIRED BY THIS PERMIT SHALL BE RETAINED BY THE PERMITEE WHO EITHER PRODUCED OR USED IT FOR A PERIOD OF AT LEAST THREE YEARS FROM THE DATE THAT THE NOT IS SUBMITTED IN ACCORDANCE WITH PART VI. OF THIS PERMIT. THESE RECORDS MUST BE MAINTAINED AT THE PERMITEE'S PRIMARY PLACE OF BUSINESS OR AT A DESIGNATED ALTERNATIVE LOCATION ONCE THE CONSTRUCTION ACTIVITY HAS CEASED AT THE PERMITTED SITE. THIS PERIOD MAY BE EXTENDED BY REQUEST OF THE EPD AT ANY TIME UPON WRITTEN NOTIFICATION TO THE PERMITEE.

THE ALLOWABLE TURBIDITY AT THE OUTFALL SAMPLING POINT FOR THIS PROJECT IS 25 NTU, SEE APPENDIX B, THIS DWG FOR INFORMATION.

THE ALLOWABLE INCREASE IN TURBIDITY BETWEEN THE DOWNSTREAM AND UPSTREAM SAMPLING POINTS IN THE RECEIVING WATERS, WHICH ARE CLASSIFIED AS WARM WATER IS 25 NTU.

I certify that the permittee's Erosion, Sedimentation and Pollution Control Plan provides for the monitoring of: (a) all perennial and intermittent streams and other water bodies shown on the USGS topographic map and all other fieldverified perennial and intermittent streams and other water bodies, or (b) where any such specific identified perennial or intermittent stream and other water body is not proposed to be sampled, I have determined in my professional judgment, utilizing the factors required in the General NPDES Permit No. GAR100002, that the increase in the turbidity of each specific identified sampled receiving water will be representative of the increase in the turbidity of a specific identified un-sampled receiving water.

WALDEN, ASHWORTH & ASSOCIATES, INC.

SIGNATURE: Jason Rapplean CERTIFICATION NO.: 3031 EXPIRATION DATE: 12/07/26

JASON RAPPLEAN, P.E., VICE PRESIDENT

IMPARED STREAM STATEMENT (CHECKLIST #2 & 3)
The South Fork Peachtree Creek TMDL GAR 031300011207 for Headwaters to Peachtree Creek segment of Chattahoochee River Basin of Atlanta, DeKalb County and Fulton County, Georgia. This segment is 15 miles long and is primarily used for fishing. A TMDL was completed in 2003 for FC and was revised in 2008. In 2018 another TMDL was created for BIO F and BIO M with a Status of No. DeKalb County and Fulton County have a Stream Monitoring Program that maintain monitoring sites along the segment for assessing water quality every quarter, including fecal coliform.
The Two Counties have programs in place for investigating potential sources of pollution. These programs are described below:
1. General urban runoff is fecal tested by the Counties Water System's Stream monitoring program and sampling is done for all pertinent biological and chemical data including fecal coliform for all streams at sites per quarter and by NPDES Fecal Coliform Monitoring Program at 8 sites per quarter. The Counties Board's of Health regulates septic tanks and maintains a nuisance ordinance against unregulated waste. The National Resources Conservation Service maintains incentives for the restoration of fencing to protect stream buffers, thereby enhancing urban runoff water quality.
2. Monitoring for sanitary sewer leaks is by the aforementioned by the Counties Water System's Stream Monitoring Program. Water Engineering's Inflow and Infiltration department's stream walks at sewer stream crossings and manholes, and through the aforementioned Fecal Coliform Monitoring Program. Water Protection group also maintains a restaurant grease trap program, prohibiting all county restaurants from discharging grease to septic tanks, and requiring all county restaurants to pump their traps quarterly so as to keep sewer line grease at a minimal and less of a factor in blocking lines and causing sanitary sewer overflows. Each County's Water's Engineering Dept also maintains a manhole raising program in low lying areas in order to place sewer caps above the latest FEMA flood plain levels, curtailing overflow contamination. Water's System maintenance also maintains a foam root control program for sewer lines.
3. Monitoring for illicit connections and illegal dumping is through the efforts of Stream Monitoring Program, Water Quality Section, and the NPDES Fecal Coliform monitoring, and County. Each County has an Illicit Discharge ordinance prohibits illicit/illegal discharges to the storm drainage system with monitoring by all sections of the water systems.
4. Animal waste from farm animals, birds, and pets is regulated through restrictive stream buffers enforced by each county's Community Development and by Board of Health's nuisance ordinance, as well as addressed through the USDA's NRCS's incentives for maintaining fences and buffers. The USDA also sponsors a program in cooperation with Stormwater to remove beavers from areas where their dams raise water levels to sanitary sewer cap manholes. Monitoring is accomplished under general fecal monitoring through the aforementioned stream monitoring and NPDES fecal monitoring programs.
5. Land disturbing activities contribution is addressed through Each County's Community Development's Erosion and Sediment Control restrictions, regulatory BMPs and buffer ordinance as well as the NRCS buffer incentives.

PRIMARY PERMITEE
OWNER:
CITY OF TUCKER
PAKS AND RECREATION DEPT.
4898 LAVISTA RD, TUCKER, GA 30084
CONTACT: RIP ROBERTSON, DIRECTOR
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EMAIL: rrobertson@tuckerga.gov
24 HR. CONTACT PERSON
RIP ROBERTSON
(470) 481-0205

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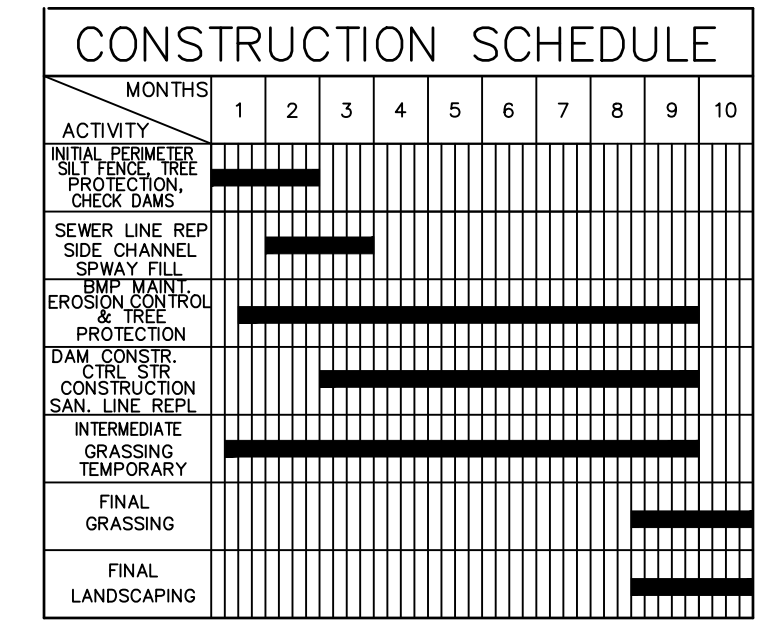


Table with columns: NO., REVISIONS, MADE, CND., DATE, DRAWING NO., REFERENCE DRAWINGS. Includes a professional seal for Jason Rapplean, No. 30441, Professional Engineer, State of Georgia, dated 06/19/2024.

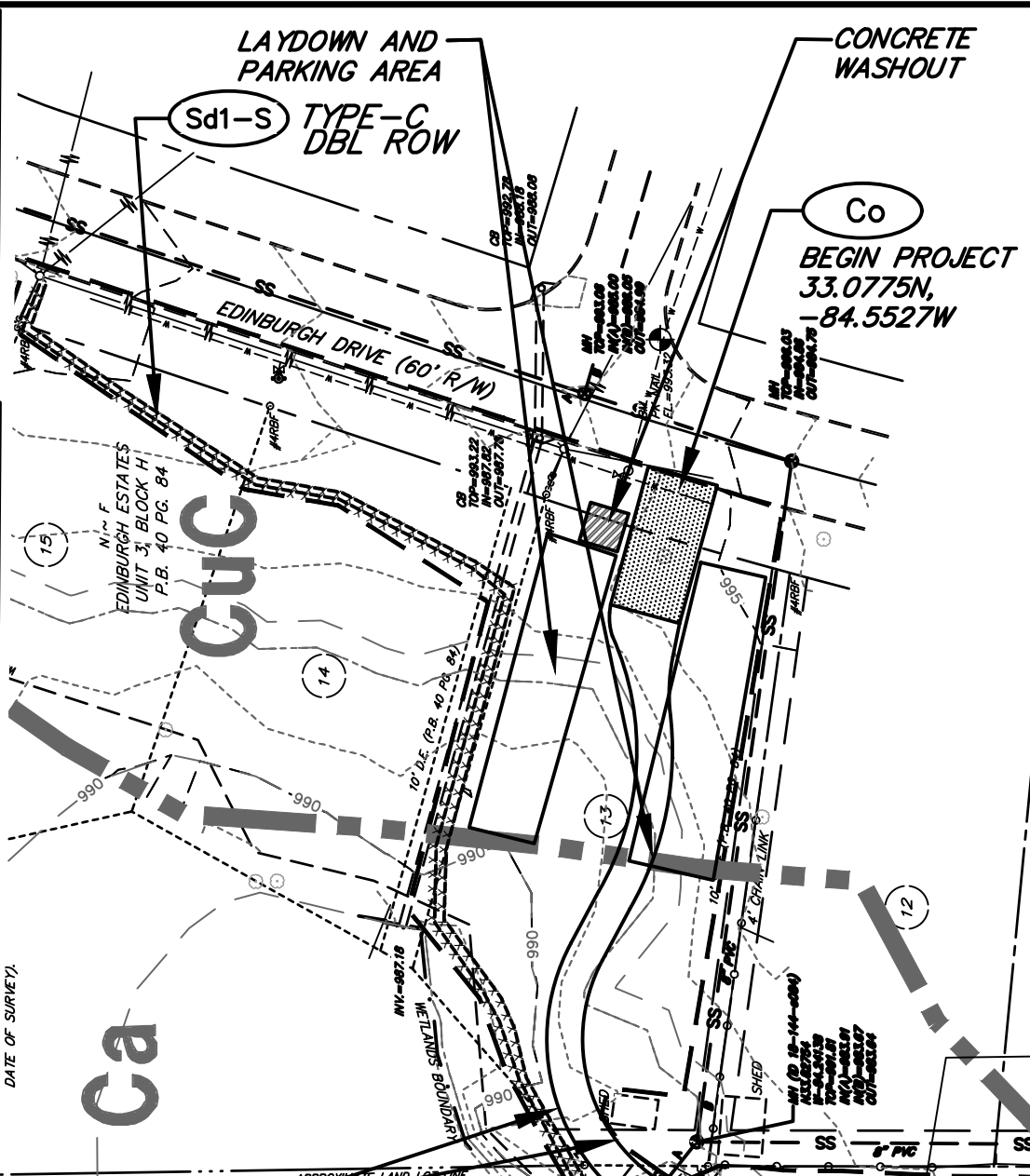
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DRAWN BY: JH/EC 01/23
CHECKED BY: JR 01/23
CDA# PEF 000707 EXP: 06/30/2024
JASON RAPPLEAN, PE, EDR
LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE: 12/07/26

JOHNS HOMESTEAD PARK
EROSION CONTROL
MONITORING NOTES
WALDEN, ASHWORTH & ASSOCIATES, INC.
CONSULTING ENGINEERS
P.O. BOX 6462
MARIETTA, GEORGIA 30065
(770) 956 - 7879
CITY OF TUCKER AND DEKALB COUNTY
4201600
42016-C-201 0



NOTE: PLEASE SEE THE COVER SHEET FOR THE VICINITY MAP AND THE FEMA FLOODPLAIN MAP FOR THE SITE AND SURROUNDING AREA. DUE TO THE SIZE OF THE SITE AND THE INFORMATION NEEDED TO BE SHOWN ON THE TWO MAPS, THERE IS NOT ENOUGH ROOM ON THIS SHEET FOR THOSE TWO MAPS AND ALL OF THE OTHER REQUIRED INFORMATION TO BE SHOWN TOGETHER.

NOTE: PLEASE SEE THE COVER SHEET FOR THE APPROVAL INFORMATION FOR A STATE WATERS BUFFER VARIANCE FOR THIS PROJECT DATED: NOV. 15, 2023; AS WELL AS A U.S. ARMY CORPS OF ENGINEERS APPROVAL FOR A PRE-CONSTRUCTION NOTIFICATION (PCN) FOR NWP No. 3 FOR THE PARK AND DAM IMPROVEMENTS NECESSARY TO CONSTRUCT THIS PROJECT.



THE PROJECT IS A RENOVATION OF AN EXISTING DAM. CONSTRUCTION ACTIVITIES WILL BE LOCATED WITHIN STATE WATERS (DRAINED DURING CONSTR.) AND/OR WITHIN WETLANDS. THE FOLLOWING TEMPORARY AND PERMANENT MEASURES WILL BE UTILIZED, IN CONJUNCTION TO THE BMP'S DENOTED ON THE EROSION CONTROL PLANS TO PREVENT THE ESCAPE OF SEDIMENT FROM ENTERING THE STREAM BELOW THE DAM.

TEMPORARY POLLUTION CONTROL MEASURES DURING CONSTRUCTION:

- PERIMETER SEDIMENT CONTROL MEASURES WILL BE INSTALLED AND ROCK-FILTER DAMS WILL BE INSTALLED AT INTAKES.
- RIPRAP BASIN (ENERGY DISSIPATORS) AT OUTLETS.
- EROSION PREVENTED WITH TEMPORARY GRASSING AND/OR MULCHING.
- CHECK DAMS OR ENERGY DISSIPATORS AT ALL DISCHARGE POINTS.

PERMANENT POLLUTION CONTROL MEASURES AFTER CONSTRUCTION IS COMPLETE:

- THE SITE STABILIZED WITH VEGETATIVE MEASURES.
- NEW STORMWATER CONTROL STRUCTURES.
- SLOPES PROTECTED WITH PERMANENT BANK STABILIZATION MATTING.
- PERMANENT ENERGY DISSIPATOR AND/OR RIPRAP PLUNGE POOLS.

SEDIMENT AND EROSION CONTROL MEASURES AND PRACTICES TO BE INSPECTED DAILY. ALL DISTURBED AREAS TO BE GRASSED AS SOON AS CONSTRUCTION PHASES PERMIT.

WEEKLY EROSION AND SEDIMENT CONTROL REPORTS SHALL BE SUBMITTED TO THE DEVELOPMENT DEPARTMENT STARTING WITH THE ISSUANCE OF THE DEVELOPMENT PERMIT AND ENDING WHEN THE PROJECT IS RELEASED BY THE INSPECTOR.

**CONSTRUCTION EXIT NOTE:**  
A STABILIZED CONSTRUCTION EXIT WILL BE PROVIDED AT THE SITE ENTRANCE AND/OR AT THE EXIT POINT OF CONSTRUCTION ACTIVITIES. THE (PUBLIC) DRIVEWAY AND STREET WILL BE INSPECTED DAILY FOR TRACKING OF MUD, DIRT, AND ROCK. DUMP TRUCKS Hauling MATERIAL FROM THE CONSTRUCTION SITE WILL BE COVERED WITH A TARPULIN.

NOTE: THE CONTRACTOR SHALL INSTALL A DOUBLE ROW OF SILT FENCE TYPE SENSITIVE NEAR WETLANDS AND STATE AND/OR COUNTY WATER BUFFERS.

NOTE: THE CRITICAL AREA OF THIS SITE IS THE TRIBUTARY OF SOUTH FORK PEACHTREE CREEK LOCATED DOWNSTREAM OF THE SITE. AN SAF STILLING BASIN WITH RIPRAP OUTLET PROTECTION AND A ROCK DAM WILL BE UTILIZED TO PREVENT SEDIMENT LEAVING THE WORK AREA AND ENTERING THE ENDANGERED STREAM SEGMENT.

NOTE: THE TRIBUTARY OF SOUTH FORK PEACHTREE CREEK IS LISTED AS AN IMPAIRED STREAM BASED ON THE LATEST 303(d) LIST ON GA EPD'S 2022 INTEGRATED LIST.

NOTE: BMP FOR CONCRETE WASHDOWN OF TOOLS, CONCRETE MIXER CHUTES, HOPPERS AND THE REAR OF THE VEHICLES IS PROVIDED. WASHOUT OF THE DRUM AT THE CONSTRUCTION SITE IS PROHIBITED.

AMENDMENTS/REVISIONS TO THE ES&PC PLAN WHICH HAVE A SIGNIFICANT EFFECT ON BMP'S WITH A HYDRAULIC COMPONENT MUST BE CERTIFIED BY THE DESIGN PROFESSIONAL.

NOTE: THE CONTRACTOR MAY ELECT TO UTILIZE A SR-B TEMPORARY BRIDGE CROSSING INSTEAD OF THE SR-C CALLED FOR ON THE PLANS IF THEY CHOOSE TO. THE BRIDGE SHOULD BE SPAN THE ENTIRE CHANNEL WIDTH.

**EROSION CONTROL (BMP) LEGEND**

LIMITS OF DISTURBANCE	SOIL DELINEATION	STABILIZATION	PROTECTION	CONSTRUCTION	STABILIZATION	TACKIFIERS AND BINDERS	FLOCCULANTS AND COAGULANTS	STORMDRAIN OUTLET PROTECTION	DIVERSION	FILTER RING	ROCK FILTER DAM	CHECKDAM
Ds1	Ds2	Ds3	Ss	Tr	Co	Fi-Ca	Fi-Ca	Di	Fr	Rd	Co	

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**SOILS LEGEND**

AKB	ALMUSTA FINE SANDY LOAM, 2-6 PERCENT SLOPES
Ca	CARTECAY SILT LOAM, FREQUENTLY FLOODED
CaB	CECIL SANDY LOAM 2-6 PERCENT SLOPES
CC2	CECIL-URBAN LAND COMPLEX, 2-10 PERCENT SLOPES
PD	PACOLET-URBAN LAND COMPLEX, 10-25 PERCENT SLOPES
Tf	TACCOGA SANDY LOAM, 0-2 PERCENT SLOPES, FREQUENTLY FLOODED
W	WATER

STATE WATERS EXISTS WITHIN 200 FEET OF THE SITE. THE RECEIVING WATERS FROM THIS SITE IS A TRIBUTARY OF SOUTH FORK PEACHTREE CREEK.

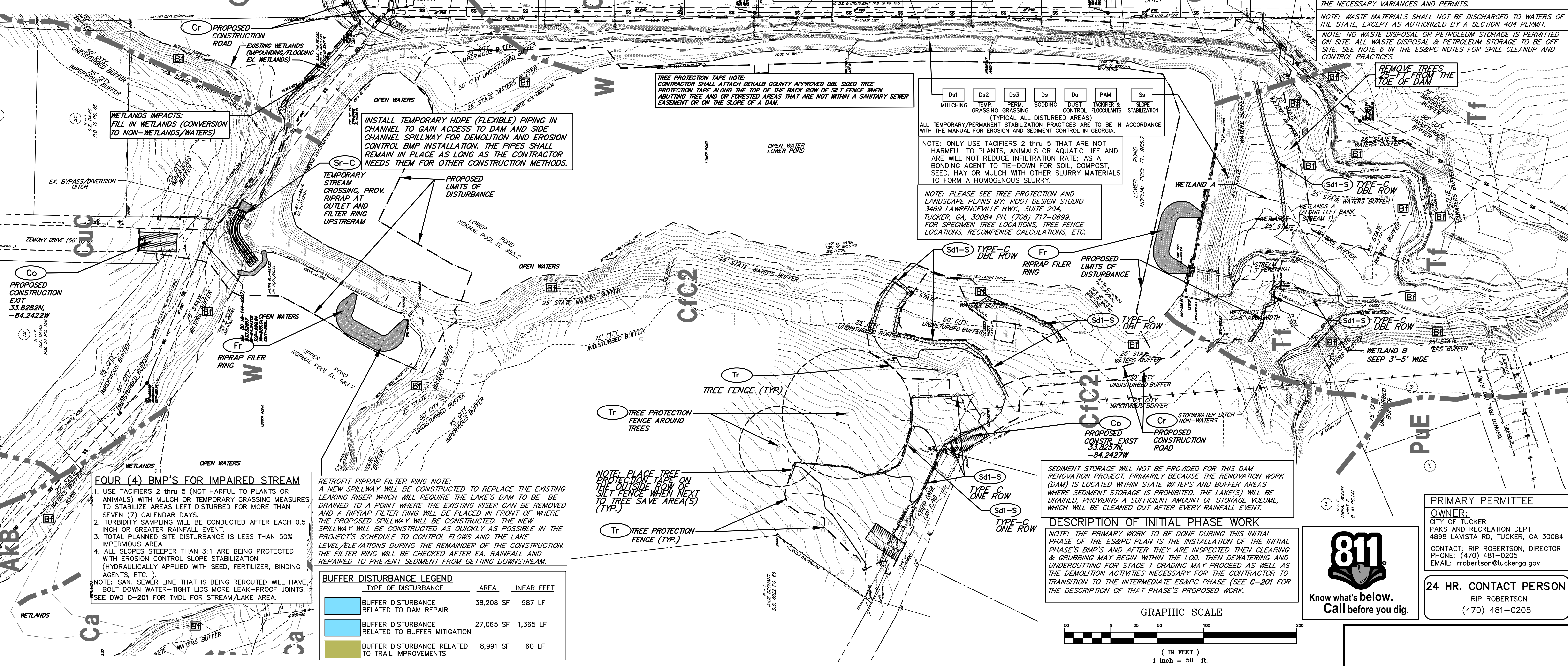
THE ESCAPE OF SEDIMENT FROM THE SITE SHALL BE PREVENTED BY THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND PRACTICES PRIOR TO LAND-DISTURBING ACTIVITIES. EROSION CONTROL MEASURES WILL BE MAINTAINED AT ALL TIMES. IF FULL IMPLEMENTATION OF THE APPROVED PLAN DOES NOT PROVIDE FOR EFFECTIVE EROSION CONTROL, ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED TO CONTROL OR TREAT THE SEDIMENT SOURCE, OR AS DIRECTED BY THE EROSION CONTROL INSPECTOR.

ANY DISTURBED AREA LEFT EXPOSED FOR A PERIOD GREATER THAN 14 DAYS SHALL BE STABILIZED WITH MULCH OR TEMPORARY SEEDING.

NOTE: ALL NON-EXEMPT ACTIVITIES SHALL NOT BE CONDUCTED WITHIN THE 25 OR 50' UNDISTURBED STREAM BUFFERS AS MEASURED FROM THE POINT OF WRESTED VEGETATION OR WITHIN 25'-FT OF THE COASTAL MARSHLAND BUFFER AS MEASURED FROM THE JURISDICTION DETERMINATION LINE, WITHOUT FIRST ACQUIRING THE NECESSARY VARIANCES AND PERMITS.

NOTE: WASTE MATERIALS SHALL NOT BE DISCHARGED TO WATERS OF THE STATE, EXCEPT AS AUTHORIZED BY A SECTION 404 PERMIT.

NOTE: NO WASTE DISPOSAL OR PETROLEUM STORAGE IS PERMITTED ON SITE. ALL WASTE DISPOSAL & PETROLEUM STORAGE TO BE OFF SITE. SEE NOTE 6 IN THE ES&PC NOTES FOR SPILL CLEANUP AND CONTROL PRACTICES.



WETLANDS IMPACTS:  
FILL IN WETLANDS (CONVERSION TO NON-WETLANDS/WATERS)

INSTALL TEMPORARY HDPE (FLEXIBLE) PIPING IN CHANNEL TO GAIN ACCESS TO DAM AND SIDE CHANNEL SPILLWAY FOR DEMOLITION AND EROSION CONTROL BMP INSTALLATION. THE PIPES SHALL REMAIN IN PLACE AS LONG AS THE CONTRACTOR NEEDS THEM FOR OTHER CONSTRUCTION METHODS.

TREE PROTECTION TAPE NOTE:  
CONTRACTOR SHALL ATTACH DEKALB COUNTY APPROVED DBL SIDED TREE PROTECTION TAPE ALONG THE TOP OF THE BACK ROW OF SILT FENCE WHEN ABUTTING TREE AND OR FORESTED AREAS THAT ARE NOT WITHIN A SANITARY SEWER EASEMENT OR ON THE SLOPE OF A DAM.

NOTE: ONLY USE TACKIFIERS 2 thru 5 THAT ARE NOT HARMFUL TO PLANTS, ANIMALS OR AQUATIC LIFE AND ARE WILL NOT REDUCE INFILTRATION RATE; AS A BONDING AGENT TO TIE-DOWN FOR SOIL, COMPOST, SEED, HAY OR MULCH WITH OTHER SLURRY MATERIALS TO FORM A HOMOGENOUS SLURRY.

NOTE: PLEASE SEE TREE PROTECTION AND LANDSCAPE PLANS BY: ROOT DESIGN STUDIO 3469 LAWRENCEVILLE HWY., SUITE 204, TUCKER, GA, 30084 PH. (706) 717-0699. FOR SPECIFIC TREE LOCATIONS, TREE FENCE LOCATIONS, RECOMPENSE CALCULATIONS, ETC.

REMOVE TREES 10' OF DAM

**FOUR (4) BMP'S FOR IMPAIRED STREAM**

- USE TACKIFIERS 2 thru 5 (NOT HARMFUL TO PLANTS OR ANIMALS) WITH MULCH OR TEMPORARY GRASSING MEASURES TO STABILIZE AREAS LEFT DISTURBED FOR MORE THAN SEVEN (7) CALENDAR DAYS.
- TURBIDITY SAMPLING WILL BE CONDUCTED AFTER EACH 0.5 INCH OR GREATER RAINFALL EVENT.
- TOTAL PLANNED SITE DISTURBANCE IS LESS THAN 50% IMPERVIOUS AREA.
- ALL SLOPES STEEPER THAN 3:1 ARE BEING PROTECTED WITH EROSION CONTROL SLOPE STABILIZATION (HYDRAULICALLY APPLIED WITH SEED, FERTILIZER, BINDING AGENTS, ETC.).

NOTE: SAN. SEWER LINE THAT IS BEING REROUTED WILL HAVE BOLT DOWN WATER-TIGHT LIDS MORE LEAK-PROOF JOINTS. SEE DWG C-201 FOR TMDL FOR STREAM/LAKE AREA.

**RETROFIT RIPRAP FILTER RING NOTE:**  
A NEW SPILLWAY WILL BE CONSTRUCTED TO REPLACE THE EXISTING LEAKING RISER WHICH WILL REQUIRE THE LAKE'S DAM TO BE DRAINED TO A POINT WHERE THE EXISTING RISER CAN BE REMOVED AND A RIPRAP FILTER RING WILL BE PLACED IN FRONT OF WHERE THE PROPOSED SPILLWAY WILL BE CONSTRUCTED. THE NEW SPILLWAY WILL BE CONSTRUCTED AS QUICKLY AS POSSIBLE IN THE PROJECT'S SCHEDULE TO CONTROL FLOWS AND THE LAKE LEVEL ELEVATIONS DURING THE REMAINDER OF THE CONSTRUCTION. THE FILTER RING WILL BE CHECKED AFTER EACH RAINFALL AND REPAIRED TO PREVENT SEDIMENT FROM GETTING DOWNSTREAM.

NOTE: PLACE TREE PROTECTION TAPE ON THE OUTSIDE ROW OF SILT FENCE WHEN NEXT TO TREE SAVE AREA(S) (TYP.)

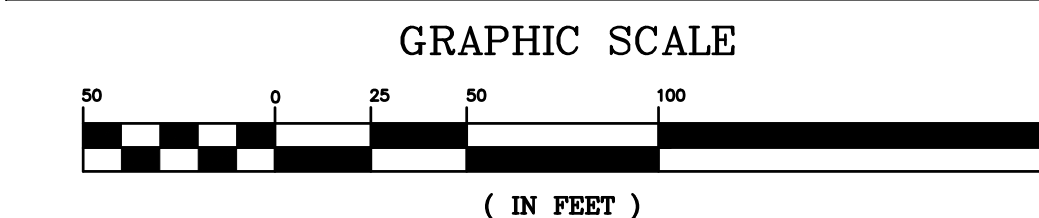
**BUFFER DISTURBANCE LEGEND**

TYPE OF DISTURBANCE	AREA	LINEAR FEET
BUFFER DISTURBANCE RELATED TO DAM REPAIR	38,208 SF	987 LF
BUFFER DISTURBANCE RELATED TO BUFFER MITIGATION	27,065 SF	1,365 LF
BUFFER DISTURBANCE RELATED TO TRAIL IMPROVEMENTS	8,991 SF	60 LF

SEDIMENT STORAGE WILL NOT BE PROVIDED FOR THIS DAM RENOVATION PROJECT, PRIMARILY BECAUSE THE RENOVATION WORK (DAM) IS LOCATED WITHIN STATE WATERS AND BUFFER AREAS WHERE SEDIMENT STORAGE IS PROHIBITED. THE LAKE(S) WILL BE DRAINED, PROVIDING A SUFFICIENT AMOUNT OF STORAGE VOLUME, WHICH WILL BE CLEANED OUT AFTER EVERY RAINFALL EVENT.

**DESCRIPTION OF INITIAL PHASE WORK**

NOTE: THE PRIMARY WORK TO BE DONE DURING THIS INITIAL PHASE OF THE ES&PC PLAN IS THE INSTALLATION OF THE INITIAL PHASE'S BMP'S AND AFTER THEY ARE INSPECTED THEN CLEARING & GRUBBING MAY BEGIN WITHIN THE LOD. THEN DOWATERING AND UNDERCUTTING FOR STAGE 1 GRADING MAY PROCEED AS WELL AS THE DEMOLITION ACTIVITIES NECESSARY FOR THE CONTRACTOR TO TRANSITION TO THE INTERMEDIATE ES&PC PHASE (SEE C-201 FOR THE DESCRIPTION OF THAT PHASE'S PROPOSED WORK).



**PRIMARY PERMITTEE**

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CITY OF TUCKER  
PAKS AND RECREATION DEPT.  
4998 LAVISTA RD, TUCKER, GA 30084

CONTACT: RIP ROBERTSON, DIRECTOR  
PHONE: (470) 481-0205  
EMAIL: rrobertson@tuckerga.gov

**24 HR. CONTACT PERSON**

RIP ROBERTSON  
(470) 481-0205

NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

SCALE: 1"=50'

DESIGN BY: JR/JH 01/23

DRAWN BY: JH/EC 01/23

CHECKED BY: JR 01/23

CD# PEF 000707 EXP. 06/30/2024

JASON RAPPLEAN, PE, EDR  
LEVEL 2 CERTIFICATION # 3091  
EXPIRATION DATE: 12/07/26

06/19/2024

**JOHNS HOMESTEAD PARK**

**EROSION CONTROL**

**INITIAL PLAN**

**CITY OF TUCKER AND DEKALB COUNTY**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**

CONSULTING ENGINEERS  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
(770) 956 - 7879

4201600 **42016-C-202A** 0



**DESCRIPTION OF FINAL PHASE WORK**

*NOTE: THE PRIMARY WORK TO BE DONE DURING THIS FINAL PHASE OF THE ES&PC PLAN IS TO VERIFY THAT THE SITE IS AT FINAL GRADE, EITHER OVER THE ENTIRE SITE OR WITHIN AREAS TO BE PERMANENTLY STABILIZED. AREAS AND/OR THE SITE THAT IS STABILIZED MAY HAVE THEIR TEMP. STRUCTURAL BMP'S REMOVED AND THEIR PERMANENT VEGETATIVE GRASSING PLACED AS DENOTED ON THE TABLES AND NOTES ON C-200. ALL AREAS TO HAVE SLOPE STABILIZATION SHALL BE CHECKED AND IF THEY HAVE NOT STARTED VEGETATING, THEN THEY SHALL BE HYDRAULICALLY APPLIED. TREE FENCING SHALL BE REMOVED IF HEAVY EQUIPMENT IS NO LONGER A THREAT TO THE TREES. THE CONCRETE WASHOUT MAY BE REMOVED IF CONCRETE IS NO LONGER BEING POURED. AN NOT MAY BE FILED WHEN THE SITE IS FULLY STABILIZED IN ACCORDANCE WITH THE NPDES PERMIT REQUIREMENTS.*

**FINAL PHASE NOTES:**

THE CONTRACTOR SHALL MAINTAIN THE SEDIMENT POND IF APPLICABLE UNTIL PERMANENT GROUND COVER IS ESTABLISHED. SEDIMENT SHALL BE CLEANED OUT OF THE POND WHEN IT REACHES ONE THIRD OF THE DEPTH OF THE BASIN.

ALL ROADWAY AND PARKING SHOULDERS SHOULD BE GRASSED AS SOON AS FINAL GRADE IS ACHIEVED.

SEDIMENT AND EROSION CONTROL MEASURES MUST BE CHECKED AFTER EACH RAIN EVENT. EACH DEVICE IS TO BE MAINTAINED OR REPLACED IF SEDIMENT ACCUMULATION HAS REACHED HALF THE CAPACITY OF THE DEVICE. ADDITIONAL DEVICES MUST BE INSTALLED IF NEW CHANNELS HAVE DEVELOPED.

FAILURE TO INSTALL, OPERATE OR MAINTAIN ALL EROSION CONTROL MEASURES WILL RESULT IN ALL CONSTRUCTION BEING STOPPED ON THE SITE UNTIL SUCH MEASURES ARE CORRECTED BACK TO THE APPROVED PLANS.

UPON COMPLETION OF THE PROJECT AND RECEIPT OF THE CERTIFICATE OF COMPLETION, THE CONTRACTOR SHALL REMOVE ALL TEMPORARY EROSION CONTROL MEASURES AND DISPOSE OF THEM UNLESS NOTED OTHERWISE ON PLANS.

FOR ADDITIONAL EROSION CONTROL NOTES FOR EACH ES&PC PHASE, SEE EROSION CONTROL NOTES DRAWINGS.

**GRADING PHASE NOTES:**

DURING CONSTRUCTION, THE CONTRACTOR SHALL MAINTAIN CAREFUL SCHEDULING AND PERFORMANCE TO ENSURE THAT LAND STRIPPED OF ITS NATURAL GROUND COVER IS EXPOSED ONLY IN SMALL QUANTITIES, AND THEREFORE, LIMITED DURATIONS BEFORE PERMANENT EROSION PROTECTION IS ESTABLISHED.

EARTHWORK OPERATIONS IN THE VICINITY OF STREAM BUFFERS SHALL BE CAREFULLY CONTROLLED TO AVOID DUMPING OR SLOUGHING INTO THE BUFFER AREAS.

EROSION CONTROL DEVICES SHALL BE INSTALLED IMMEDIATELY AFTER GROUND DISTURBANCE OCCURS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ACCOMPLISH EROSION CONTROL FOR ALL DRAINAGE PATTERNS CREATED AT VARIOUS STAGES DURING CONSTRUCTION, AND ALTER THE LOCATION OF EROSION CONTROL DEVICES ACCORDINGLY. ANY DIFFICULTY IN CONTROLLING EROSION DURING ANY PHASE OF CONSTRUCTION SHALL BE REPORTED TO THE DESIGN PROFESSIONAL IMMEDIATELY.

THE CONTRACTOR SHALL ESTABLISH BARRIERS AT THE TOP OF ALL SLOPES UNDER CONSTRUCTION. CUT AND FILL SLOPES SHALL NOT EXCEED 3:1 UNLESS SPECIFICALLY DENOTED ON THE PLANS.

STORM DRAIN OUTLET PROTECTION SHALL BE PLACED AT ALL OUTLET HEADWALLS AS SOON AS THE HEADWALL IS CONSTRUCTED.

ALL DRAINAGE SWALES AND GRADED AREAS SHALL BE APPLIED WITH VEGETATIVE COVER AS SOON AS FINAL GRADE IS ACHIEVED. MULCH OR TEMPORARY GRASSING SHALL BE APPLIED TO ALL EXPOSED AREAS WITHIN 7 DAYS OF LAND DISTURBANCE. ALL DISTURBED AREAS LEFT MULCHED FOR MORE THAN 30 DAYS SHALL BE STABILIZED WITH TEMPORARY GRASSING.

THE CONTRACTOR SHALL MAINTAIN THE SEDIMENT POND IF APPLICABLE UNTIL PERMANENT GROUND COVER IS ESTABLISHED. SEDIMENT SHALL BE CLEAN OUT OF THE POND WHEN IT REACHES ONE THIRD OF THE DEPTH OF THE BASIN.

MULCH OR TEMPORARY GRASSING SHALL BE APPLIED TO ALL EXPOSED AREAS WITHIN 7 DAYS OF LAND DISTURBANCE. ALL DISTURBED AREAS LEFT MULCHED FOR MORE THAN 30 DAYS SHALL BE STABILIZED WITH TEMPORARY GRASSING.

SEDIMENT AND EROSION CONTROL MEASURES MUST BE CHECKED AFTER EACH RAIN EVENT. EACH DEVICE IS TO BE MAINTAINED OR REPLACED IF SEDIMENT ACCUMULATION HAS REACHED HALF THE CAPACITY OF THE DEVICE. ADDITIONAL DEVICES MUST BE INSTALLED IF NEW CHANNELS HAVE DEVELOPED.

CONTRACTOR SHALL INSPECT CONTROL MEASURES AT THE END OF EACH WORKING DAY TO ENSURE MEASURES ARE FUNCTIONING PROPERLY.

THE CONSTRUCTION EXIT SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACK OR FLOW OF MUD ONTO PUBLIC RIGHT-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH 1-3" OF STONE, AS CONDITIONS DEMAND. ALL MATERIALS SPILLED, DROPPED WASHED OR TRACKED FROM A VEHICLE ONTO PUBLIC ROADWAY OR INTO STORM DRAIN MUST BE REMOVED IMMEDIATELY.

FAILURE TO INSTALL, OPERATE OR MAINTAIN ALL EROSION CONTROL MEASURES WILL RESULT IN ALL CONSTRUCTION BEING STOPPED ON THE JOB UNTIL SUCH MEASURES ARE CORRECTED BACK TO THE APPROVED EROSION CONTROL PLANS.

FOR ADDITIONAL EROSION CONTROL NOTES FOR EACH ES&PC PHASE, SEE EROSION CONTROL NOTES DRAWINGS.

**CLEARING PHASE NOTES:**

PRIOR TO LAND DISTURBING ACTIVITY, THE CONTRACTOR SHALL SCHEDULE A PRECONSTRUCTION MEETING WITH THE AREA SITE DEVELOPMENT INSPECTOR(S), THE ENGINEER(S), AND THE CITY, COUNTY AND STATE OFFICIALS.

THE CONTRACTOR SHALL OBSERVE THE PROJECT SEQUENCE SHOWN ON THE PLANS. THE CONTRACTOR SHALL MAINTAIN CAREFUL SCHEDULING AND PERFORMANCE TO ENSURE THAT LAND STRIPPED OF ITS NATURAL COVER IS EXPOSED ONLY IN SMALL QUANTITIES.

THE OWNER AGREES TO PROVIDE AND MAINTAIN OFF-STREET PARKING ON THE SUBJECT PROPERTY DURING THE ENTIRE CONSTRUCTION PERIOD.

NO STAGING AREAS, MATERIAL STORAGE, CONCRETE WASH OUT AREAS, OR DEBRIS BURNING AND BURIAL HOLES SHALL BE LOCATED WITHIN 500 FEET OF DESIGNATED TREE PROTECTION AREAS.

A COPY OF THE APPROVED LAND DISTURBANCE PLAN AND PERMIT SHALL BE PRESENT ON THE SITE AT ALL TIMES.

PRIOR TO COMMENCING LAND DISTURBANCE ACTIVITY, LIMITS OF LAND DISTURBANCE SHALL CLEARLY AND ACCURATELY BE DEMARCATED WITH STAKES, RIBBONS OR OTHER APPROPRIATE MEANS, AND SHALL BE DEMARCATED FOR THE DURATION OF THE CONSTRUCTION ACTIVITY. NO LAND DISTURBANCE SHALL OCCUR OUTSIDE THE LIMITS INDICATED ON THE APPROVED PLANS.

PRIOR TO ANY OTHER CONSTRUCTION, A STABILIZED CONSTRUCTION ENTRANCE SHALL BE CONSTRUCTED AT EACH POINT OF ENTRY TO OR EXIT FROM THE SITE OR ONTO ANY PUBLIC ROADWAY.

THE FOLLOWING INITIAL EROSION CONTROL MEASURES SHALL BE IMPLEMENTED PRIOR TO ANY OTHER CONSTRUCTION ACTIVITY:

1. THE CONSTRUCTION EXIT(S) SHALL BE PLACED AS SHOWN ON THE PLANS.
2. IMMEDIATELY AFTER THE ESTABLISHMENT OF CONSTRUCTION EXIT(S), ALL PERIMETER EROSION CONTROL AND STORMWATER MANAGEMENT DEVICES SHALL BE INSTALLED AS SHOWN ON THE CLEARING PHASE EROSION CONTROL PLAN.
3. TREE PROTECTION FENCING SHALL BE INSTALLED PRIOR TO THE START OF ANY LAND DISTURBING ACTIVITY (SEE PLAN(S) BY ROOT DESIGN GROUP).

WITHIN SEVEN (7) DAYS AFTER INSTALLATION OF INITIAL EROSION CONTROL MEASURES, THE SITE CONTRACTOR SHALL SCHEDULE AN INSPECTION BY THE PROJECT DESIGN PROFESSIONAL. NO OTHER CONSTRUCTION ACTIVITIES SHALL OCCUR UNTIL THE PROJECT PROFESSIONAL APPROVES THE INSTALLATION OF SAID EROSION CONTROL MEASURES. IF UNFORESEEN CONDITIONS EXIST IN THE FIELD THAT WARRANT ADDITIONAL EROSION CONTROL MEASURES, THE CONTRACTOR MUST CONSTRUCT ANY ADDITIONAL EROSION CONTROL DEVICES DEEMED NECESSARY BY THE PROJECT PROFESSIONAL DURING THE SITE INSPECTION.

AFTER APPROVAL OF INITIAL EROSION CONTROL INSTALLATION, THE CONTRACTOR MAY PROCEED WITH CLEARING AND GRUBBING ACTIVITIES. AS CLEARING PERMITS, THE CONTRACTOR SHALL CONSTRUCT SEDIMENT PONDS AS SHOWN ON PLANS.

THE CONTRACTOR CAN UTILIZE CLEARED TREES AS BARRIER BRUSH SEDIMENT CONTROL WHERE INITIAL GRADING ACTIVITIES WILL NOT OCCUR.

NO BURN OR BURY PITS SHALL BE PERMITTED ON THE CONSTRUCTION SITE WITHOUT WRITTEN PERMISSION BY THE OWNER AND/OR THE ENGINEER OF RECORD.

ALL SILT FENCES MUST MEET THE REQUIREMENTS OF SECTION 171-TEMPORARY SILT FENCE FOR THE DEPARTMENT OF TRANSPORTATION, STATE OF GEORGIA, STANDARD SPECIFICATIONS, 1983 EDITION.

MULCH OR TEMPORARY GRASSING SHALL BE APPLIED TO ALL EXPOSED AREAS WITHIN 7 DAYS OF LAND DISTURBANCE. ALL DISTURBED AREAS LEFT MULCHED MORE THAN 30 DAYS SHALL BE STABILIZED WITH TEMPORARY VEGETATION.

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CONTRACTOR SHALL INSPECT EROSION CONTROL MEASURES AT THE END OF EACH WORKING DAY TO ENSURE PROPER FUNCTIONING. FAILURE TO INSTALL, OPERATE OR MAINTAIN ALL EROSION CONTROL MEASURES WILL RESULT IN ALL CONSTRUCTION BEING STOPPED ON THE SITE UNTIL SUCH MEASURES ARE CORRECTED BACK TO THE APPROVED PLANS.

FOR ADDITIONAL EROSION CONTROL NOTES FOR EACH ES&PC PHASE, SEE CONTROL NOTES DRAWINGS.

**PRIMARY PERMITTEE**

**OWNER:**  
CITY OF TUCKER  
PAKS AND RECREATION DEPT.  
4898 LAVISTA RD, TUCKER, GA 30084  
CONTACT: RIP ROBERTSON, DIRECTOR  
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**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
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JASON RAPPLEAN, PE, EDR LEVEL 2 CERTIFICATION # 3031 EXPIRATION DATE: 12/07/26	

**JOHNS HOMESTEAD PARK  
EROSION CONTROL  
INITIAL, INTERMEDIATE AND FINAL NOTES PLAN**

**CITY OF TUCKER AND DEKALB COUNTY**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**

CONSULTING ENGINEERS  
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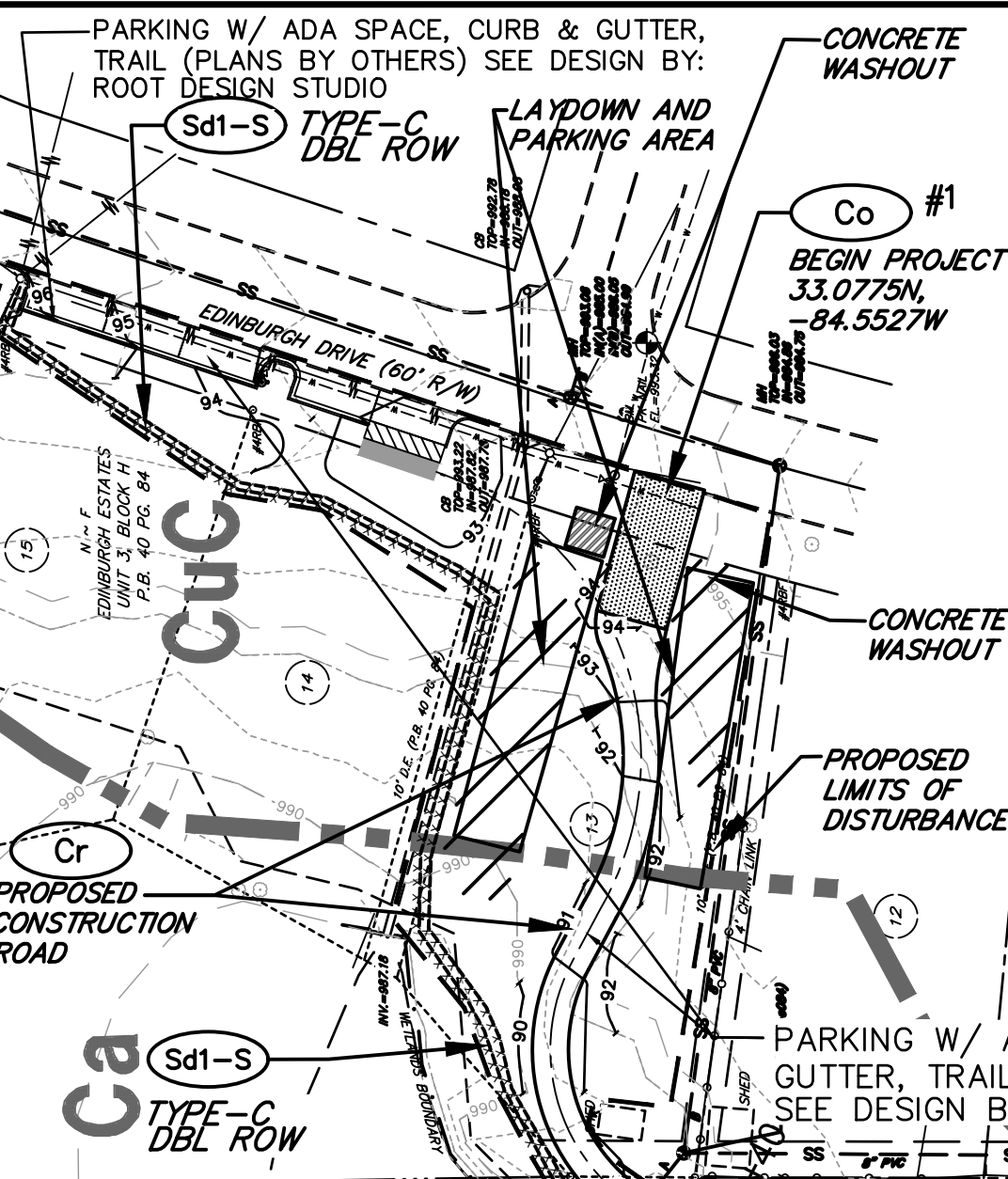


**NOTE: PLEASE SEE THE COVER SHEET FOR THE VICINITY MAP AND THE FEMA FLOODPLAIN MAP FOR THE SITE AND SURROUNDING AREA. DUE TO THE SIZE OF THE SITE AND THE INFORMATION NEEDED TO BE SHOWN ON THE TWO MAPS, THERE IS NOT ENOUGH ROOM ON THIS SHEET FOR THOSE TWO MAPS AND ALL OF THE OTHER REQUIRED INFORMATION TO BE SHOWN TOGETHER.**

**STATE WATERS BUFFER STATEMENT**

NOTE: THIS PROJECT IS BEING CARRIED OUT UNDER THE SUPERVISION OF THE CITY OF TUCKER AND DEKALB COUNTY PUBLIC WORKS. THERE ARE THREE PORTIONS OF WORK INVOLVED WITH THIS BUFFER VARIANCE REQUEST:

1. IS TO MOVE THE UPPER DAM SO THAT THE SEWER LINE NO LONGER IS WITHIN THE DAM, WHICH CREATES A HAZARD FOR THE UPSTREAM WETLANDS AND THE DOWNSTREAM OPEN WATERS.
2. THE SIDE CHANNEL SPILLWAY FOR THE UPPER DAM RUNS ALONG A SANITARY SEWER LINE AT APPROXIMATELY THE SAME ELEVATION AND IT IS EXEMPT FROM THE GEORGIA EROSION AND PRESENTMENTS NOT ONLY A CONTAMINATION HAZARD BUT A HYDRAULIC ONE AS WELL, SO WE PROPOSE TO CLOSE THE SPILLWAY WITH THE CONSTRUCTION OF A NEW SPILLWAY THAT IS CAPABLE OF HANDLING THE DESIGN FLOW.
3. THE LOWER DAM HAS A SEEPAGE ISSUE AND EVEN WITH THE SIDE CHANNEL SPILLWAY, IS AT RISK OF FAILURE DUE TO OVERTOPPING. SO WE PROPOSE TO CONSTRUCT A NEW LABYRINTH SPILLWAY WITH A SEEPAGE DRAIN SYSTEM.



**THE PROJECT IS A RENOVATION OF AN EXISTING DAM. CONSTRUCTION ACTIVITIES WILL BE LOCATED WITHIN STATE WATERS (DRAINED DURING CONSTR.) AND/OR WITHIN WETLAND AREAS. THE FOLLOWING TEMPORARY AND PERMANENT MEASURES WILL BE UTILIZED, IN CONJUNCTION TO THE BMP'S DENOTED ON THE EROSION CONTROL PLANS TO PREVENT THE ESCAPE OF SEDIMENT FROM ENTERING THE STREAM BELOW THE DAM.**

- TEMPORARY POLLUTION CONTROL MEASURES DURING CONSTRUCTION:**
1. PERIMETER SEDIMENT CONTROL MEASURES WILL BE INSTALLED AND ROCK-FILTER DAMS WILL BE INSTALLED AT INTAKES.
  2. RIPRAP BASIN (ENERGY DISSIPATORS) AT OUTLETS.
  3. EROSION PREVENTED WITH TEMPORARY GRASSING AND/OR MULCHING.
  4. CHECK DAMS OR ENERGY DISSIPATORS AT ALL DISCHARGE POINTS.

**PERMANENT POLLUTION CONTROL MEASURES AFTER CONSTRUCTION IS COMPLETE:**

1. THE SITE STABILIZED WITH VEGETATIVE MEASURES.
2. NEW STORMWATER CONTROL STRUCTURES.
3. SLOPES PROTECTED WITH PERMANENT BANK STABILIZATION MATTING.
4. PERMANENT ENERGY DISSIPATOR AND/OR RIPRAP PLUNGE POOLS.

**SEDIMENT AND EROSION CONTROL MEASURES AND PRACTICES TO BE INSPECTED DAILY. ALL DISTURBED AREAS TO BE GRASSED AS SOON AS CONSTRUCTION PHASES PERMIT.**

**WEEKLY EROSION AND SEDIMENT CONTROL REPORTS SHALL BE SUBMITTED TO THE DEVELOPMENT DEPARTMENT STARTING WITH THE ISSUANCE OF THE DEVELOPMENT PERMIT AND ENDING WHEN THE PROJECT IS RELEASED BY THE INSPECTOR.**

**CONSTRUCTION EXIT NOTE:**

A STABILIZED CONSTRUCTION EXIT WILL BE PROVIDED AT THE SITE ENTRANCE AND/OR AT THE EXIT POINT OF CONSTRUCTION ACTIVITIES. THE (PUBLIC) DRIVEWAY AND STREET WILL BE INSPECTED DAILY FOR TRACKING OF MUD, DIRT, AND ROCK. DUMP TRUCKS Hauling MATERIAL FROM THE CONSTRUCTION SITE WILL BE COVERED WITH A TARPAULIN.

NOTE: THE CONTRACTOR SHALL INSTALL A DOUBLE ROW OF SILT FENCE TYPE SENSITIVE NEAR WETLANDS AND STATE AND/OR COUNTY WATER BUFFERS.

NOTE: THE CRITICAL AREA OF THIS SITE IS THE TRIBUTARY OF SOUTH FORK PEACHTREE CREEK LOCATED DOWNSTREAM OF THE SITE. AN SAF STILLING BASIN WITH RIPRAP OUTLET PROTECTION AND A ROCK DAM WILL BE UTILIZED TO PREVENT SEDIMENT LEAVING THE WORK AREA AND ENTERING THE ENDANGERED STREAM SEGMENT.

NOTE: THE TRIBUTARY OF SOUTH FORK PEACHTREE CREEK IS LISTED AS AN IMPAIRED STREAM BASED ON THE LATEST 303(d) LIST ON GA EPD'S 2022 INTEGRATED LIST.

NOTE: BMP FOR CONCRETE WASHDOWN OF TOOLS, CONCRETE MIXER CHUTES, HOPPERS AND THE REAR OF THE VEHICLES IS PROVIDED. WASHDOWN OF THE DRUM AT THE CONSTRUCTION SITE IS PROHIBITED.

AMENDMENTS/REVISIONS TO THE ES&PC PLAN WHICH HAVE A SIGNIFICANT EFFECT ON BMP'S WITH A HYDRAULIC COMPONENT MUST BE CERTIFIED BY THE DESIGN PROFESSIONAL.

**EROSION CONTROL (BMP) LEGEND**

LIMITS OF DISTURBANCE	SOIL DELINEATION	STABILIZATION	PROTECTION	CONSTRUCTION	SLOPE STABILIZATION	TACKIFIERS AND BINDERS	FLOCCULANTS AND COAGULANTS	STORMWATER OUTLET PROTECTION	DIVERSION	FILTER RING	ROCK FILTER DAM	CHECKDAM
Ds1	Ds2	Ss	Tr	Co	Ss	Ta	Fl-Co	St	Di	Fr	Rd	Co

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**SOILS LEGEND**

AK	ALTIMATA FINE SANDY LOAM, 2-6 PERCENT SLOPES
Ca	CARTECAY SILT LOAM, FREQUENTLY FLOODED
CaB	CECIL SANDY LOAM 2-6 PERCENT SLOPES
CC2	CECIL-URBAN LAND COMPLEX, 2-10 PERCENT SLOPES
PD	PACOLET-URBAN LAND COMPLEX, 10-25 PERCENT SLOPES
Tf	TACCOGA SANDY LOAM, 0-2 PERCENT SLOPES, FREQUENTLY FLOODED
W	WATER

**STATE WATERS EXISTS WITHIN 200 FEET OF THE SITE. THE RECEIVING WATERS FROM THIS SITE IS A TRIBUTARY OF SOUTH FORK PEACHTREE CREEK.**

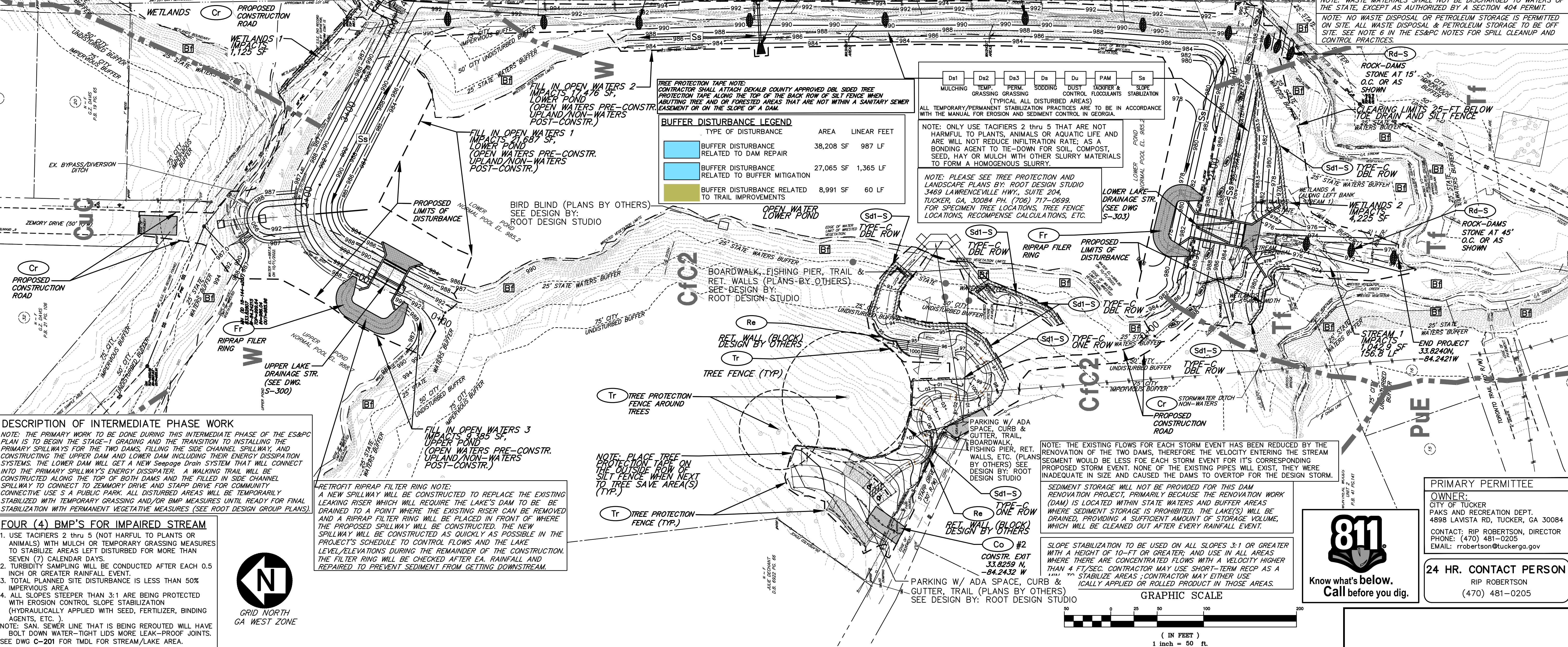
**THE ESCAPE OF SEDIMENT FROM THE SITE SHALL BE PREVENTED BY THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND PRACTICES PRIOR TO LAND-DISTURBING ACTIVITIES. EROSION CONTROL MEASURES WILL BE MAINTAINED AT ALL TIMES. IF FULL IMPLEMENTATION OF THE APPROVED PLAN DOES NOT PROVIDE FOR EFFECTIVE EROSION CONTROL, ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED TO CONTROL OR TREAT THE SEDIMENT SOURCE, OR AS DIRECTED BY THE EROSION CONTROL INSPECTOR.**

**ANY DISTURBED AREA LEFT EXPOSED FOR A PERIOD GREATER THAN 14 DAYS SHALL BE STABILIZED WITH MULCH OR TEMPORARY SEEDING.**

NOTE: ALL NON-EXEMPT ACTIVITIES SHALL NOT BE CONDUCTED WITHIN THE 25 OR 50' UNDISTURBED STREAM BUFFERS AS MEASURED FROM THE POINT OF WRESTED VEGETATION OR WITHIN 25-FT OF THE COASTAL MARSHLAND BUFFER AS MEASURED FROM THE JURISDICTIONAL DETERMINATION LINE, WITHOUT FIRST ACQUIRING THE NECESSARY VARIANCES AND PERMITS.

NOTE: WASTE MATERIALS SHALL NOT BE DISCHARGED TO WATERS OF THE STATE, EXCEPT AS AUTHORIZED BY A SECTION 404 PERMIT.

NOTE: NO WASTE DISPOSAL OR PETROLEUM STORAGE IS PERMITTED ON SITE. ALL WASTE DISPOSAL & PETROLEUM STORAGE TO BE OFF SITE. SEE NOTE 6 IN THE ES&PC NOTES FOR SPILL CLEANUP AND CONTROL PRACTICES.



**BUFFER DISTURBANCE LEGEND**

TYPE OF DISTURBANCE	AREA	LINEAR FEET
Buffer disturbance related to dam repair	38,208 SF	987 LF
Buffer disturbance related to buffer mitigation	27,065 SF	1,365 LF
Buffer disturbance related to trail improvements	8,991 SF	60 LF

- MULCHING TEMP:** Ds1, Ds2, Ds3, Ds4, Ds5, PAM, Ss
- GRASSING TEMP:** Ss
- PERM. SODDING:** Ss
- DUST TACKIFIER:** Ta
- CONTROL FLOCCULANTS:** Fl-Co
- STABILIZATION:** Ss

NOTE: ONLY USE TACKIFIERS 2 thru 5 THAT ARE NOT HARMFUL TO PLANTS, ANIMALS OR AQUATIC LIFE AND ARE WILL NOT REDUCE INFILTRATION RATE; AS A BONDING AGENT TO TIE-DOWN FOR SOIL, COMPOST, SEED, HAY OR MULCH WITH OTHER SLURRY MATERIALS TO FORM A HOMOGENEOUS SLURRY.

NOTE: PLEASE SEE TREE PROTECTION AND LANDSCAPE PLANS BY: ROOT DESIGN STUDIO 3469 LAWRENCEVILLE HWY., SUITE 204, TUCKER, GA, 30084 PH. (706) 717-0699 FOR SPECIMEN TREE LOCATIONS, TREE FENCE LOCATIONS, RECOMPENSE CALCULATIONS, ETC.

**DESCRIPTION OF INTERMEDIATE PHASE WORK**

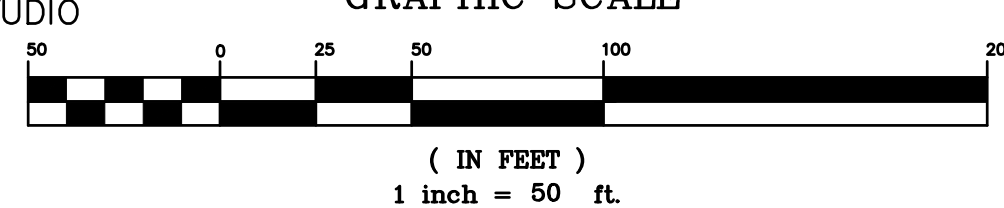
NOTE: THE PRIMARY WORK TO BE DONE DURING THIS INTERMEDIATE PHASE OF THE ES&PC PLAN IS TO BEGIN THE STAGE-1 GRADING AND THE TRANSITION TO INSTALLING THE PRIMARY SPILLWAYS FOR THE TWO DAMS, FILLING THE SIDE CHANNEL SPILLWAY, AND CONSTRUCTING THE UPPER DAM AND LOWER DAM INCLUDING THEIR ENERGY DISSIPATION SYSTEMS. THE LOWER DAM WILL GET A NEW Seepage Drain System THAT WILL CONNECT INTO THE PRIMARY SPILLWAY'S ENERGY DISSIPATOR. A WALKING TRAIL WILL BE CONSTRUCTED ALONG THE TOP OF BOTH DAMS AND THE FILLED IN SIDE CHANNEL SPILLWAY TO CONNECT TO ZEMORY DRIVE AND STAFF DRIVE FOR COMMUNITY CONNECTIVE USE AS A PUBLIC PARK. ALL DISTURBED AREAS WILL BE TEMPORARILY STABILIZED WITH TEMPORARY GRASSING AND/OR BMP MEASURES UNTIL READY FOR FINAL STABILIZATION WITH PERMANENT VEGETATIVE MEASURES (SEE ROOT DESIGN GROUP PLANS).

**FOUR (4) BMP'S FOR IMPAIRED STREAM**

1. USE TACKIFIERS 2 thru 5 (NOT HARMFUL TO PLANTS OR ANIMALS) WITH MULCH OR TEMPORARY GRASSING MEASURES TO STABILIZE AREAS LEFT DISTURBED FOR MORE THAN SEVEN (7) CALENDAR DAYS.
  2. TURBIDITY SAMPLING WILL BE CONDUCTED AFTER EACH 0.5 INCH OR GREATER RAINFALL EVENT.
  3. TOTAL PLANNED SITE DISTURBANCE IS LESS THAN 50% IMPERVIOUS AREA.
  4. ALL SLOPES STEEPER THAN 3:1 ARE BEING PROTECTED WITH EROSION CONTROL SLOPE STABILIZATION (HYDRAULICALLY APPLIED WITH SEED, FERTILIZER, BINDING AGENTS, ETC.).
- NOTE: SAN. SEWER LINE THAT IS BEING REROUTED WILL HAVE BOLT DOWN WATER-TIGHT UDS MORE LEAK-PROOF JOINTS. SEE DWG C-201 FOR TMDL FOR STREAM/LAKE AREA.

**RETROFIT RIPRAP FILTER RING NOTE:**  
A NEW SPILLWAY WILL BE CONSTRUCTED TO REPLACE THE EXISTING LEAKING RISER WHICH WILL REQUIRE THE LAKE'S DAM TO BE DRAINED TO A POINT WHERE THE EXISTING RISER CAN BE REMOVED AND A RIPRAP FILTER RING WILL BE PLACED IN FRONT OF WHERE THE PROPOSED SPILLWAY WILL BE CONSTRUCTED. THE NEW SPILLWAY WILL BE CONSTRUCTED AS QUICKLY AS POSSIBLE IN THE PROJECT'S SCHEDULE TO CONTROL FLOWS AND THE LAKE LEVEL/ELEVATIONS DURING THE REMAINDER OF THE CONSTRUCTION. THE FILTER RING WILL BE CHECKED AFTER EA. RAINFALL AND REPAIRED TO PREVENT SEDIMENT FROM GETTING DOWNSTREAM.

**NOTE: PLACE TREE PROTECTION TAPE ON THE OUTSIDE ROW OF SILT FENCE WHEN NEXT TO TREE SAVE AREA(S) (TYP.)**



**PRIMARY PERMITTEE**

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4998 LAVISTA RD, TUCKER, GA 30084

**CONTACT:** RIP ROBERTSON, DIRECTOR  
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EMAIL: rrobertson@tuckerga.gov

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**EROSION CONTROL INTERMEDIATE PLAN**

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4201600 **42016-C-203** 0



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- 3.

**CONSTRUCTION EXIT, STAGING AREA, LAYDOWN AREA, TEMPORARY PARKING AND CONSTRUCTION ACCESS DRIVE RESTORED TO NATURAL CONDITIONS AND VEGETATED WITH PERMANENT GRASSING.**

**THE PROJECT IS A RENOVATION OF AN EXISTING DAM. CONSTRUCTION ACTIVITIES WILL BE LOCATED WITHIN STATE WATERS (DRAINED DURING CONSTR.) AND/OR WITHIN WETLAND AREAS. THE FOLLOWING TEMPORARY AND PERMANENT MEASURES WILL BE UTILIZED, IN CONJUNCTION TO THE BMP'S DENOTED ON THE EROSION CONTROL PLANS TO PREVENT THE ESCAPE OF SEDIMENT FROM ENTERING THE STREAM BELOW THE DAM.**

- TEMPORARY POLLUTION CONTROL MEASURES DURING CONSTRUCTION:**
1. PERIMETER SEDIMENT CONTROL MEASURES WILL BE INSTALLED AND ROCK-FILTER DAMS WILL BE INSTALLED AT INTAKES.
  2. RIPRAP BASIN (ENERGY DISSIPATORS) AT OUTLETS.
  3. EROSION PREVENTED WITH TEMPORARY GRASSING AND/OR MULCHING.
  4. CHECK DAMS OR ENERGY DISSIPATORS AT ALL DISCHARGE POINTS.

**PERMANENT POLLUTION CONTROL MEASURES AFTER CONSTRUCTION IS COMPLETE:**

1. THE SITE STABILIZED WITH VEGETATIVE MEASURES.
2. NEW STORMWATER CONTROL STRUCTURES.
3. SLOPES PROTECTED WITH PERMANENT BANK STABILIZATION MATTING.
4. PERMANENT ENERGY DISSIPATOR AND/OR RIPRAP PLUNGE POOLS.

**SEDIMENT AND EROSION CONTROL MEASURES AND PRACTICES TO BE INSPECTED DAILY. ALL DISTURBED AREAS TO BE GRASSED AS SOON AS CONSTRUCTION PHASES PERMIT.**

**WEEKLY EROSION AND SEDIMENT CONTROL REPORTS SHALL BE SUBMITTED TO THE DEVELOPMENT DEPARTMENT STARTING WITH THE ISSUANCE OF THE DEVELOPMENT PERMIT AND ENDING WHEN THE PROJECT IS RELEASED BY THE INSPECTOR.**

**CONSTRUCTION EXIT NOTE:**

A STABILIZED CONSTRUCTION EXIT WILL BE PROVIDED AT THE SITE ENTRANCE AND/OR AT THE EXIT POINT OF CONSTRUCTION ACTIVITIES. THE (PUBLIC) DRIVEWAY AND STREET WILL BE INSPECTED DAILY FOR TRACKING OF MUD, DIRT, AND ROCK. DUMP TRUCKS Hauling MATERIAL FROM THE CONSTRUCTION SITE WILL BE COVERED WITH A TARPAULIN.

**NOTE:** THE CONTRACTOR SHALL INSTALL A DOUBLE ROW OF SILT FENCE TYPE SENSITIVE NEAR WETLANDS AND STATE AND/OR COUNTY WATER BUFFERS.

**NOTE:** THE CRITICAL AREA OF THIS SITE IS THE TRIBUTARY OF SOUTH FORK PEACHTREE CREEK LOCATED DOWNSTREAM OF THE SITE. AN SAF STILLING BASIN WITH RIPRAP OUTLET PROTECTION AND A ROCK DAM WILL BE UTILIZED TO PREVENT SEDIMENT LEAVING THE WORK AREA AND ENTERING THE ENDANGERED STREAM SEGMENT.

**NOTE:** THE TRIBUTARY OF SOUTH FORK PEACHTREE CREEK IS LISTED AS AN IMPAIRED STREAM BASED ON THE LATEST 303(d) LIST ON GA EPD'S 2022 INTEGRATED LIST.

**NOTE:** BMP FOR CONCRETE WASHDOWN OF TOOLS, CONCRETE MIXER CHUTES, HOPPERS AND THE REAR OF THE VEHICLES IS PROVIDED. WASHOUT OF THE DRUM AT THE CONSTRUCTION SITE IS PROHIBITED.

**AMENDMENTS/REVISIONS TO THE ES&PC PLAN WHICH HAVE A SIGNIFICANT EFFECT ON BMP'S WITH A HYDRAULIC COMPONENT MUST BE CERTIFIED BY THE DESIGN PROFESSIONAL.**

**EROSION CONTROL (BMP) LEGEND**

<b>SOIL DELINEATION</b>	<b>BT BUFFER ZONE</b>
<b>Ds1</b> DISTURBED AREA STABILIZATION WITH MULCHING ONLY	<b>Ds2</b> DISTURBED AREA STABILIZATION WITH TEMP. SEEDING
<b>Ds3</b> STABILIZED AREA (PERM. SEEDING)	<b>Ds4</b> STABILIZED AREA (PERM. SEEDING)
<b>Ss</b> SLOPE STABILIZATION	<b>Tr</b> TREE PROTECTION
<b>Co</b> CONSTRUCTION EXIT	<b>St</b> STORM DRAIN OUTLET PROTECTION
<b>Ss</b> SLOPE STABILIZATION	<b>Fl-Co</b> FLOCCULANTS AND COAGULANTS
<b>Ta</b> TACKIFIERS AND BINDERS	<b>St</b> STORM DRAIN OUTLET PROTECTION
<b>Fl-Co</b> FLOCCULANTS AND COAGULANTS	<b>Di</b> DIVERSION
<b>St</b> STORM DRAIN OUTLET PROTECTION	<b>Ft</b> FILTER RING
<b>Di</b> DIVERSION	<b>Rf</b> ROCK FILTER DAM
<b>Ft</b> FILTER RING	<b>Ch</b> CHECKDAM
<b>Rf</b> ROCK FILTER DAM	
<b>Ch</b> CHECKDAM	

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**SOILS LEGEND**

AKB	ALTAVISTA FINE SANDY LOAM, 2-6 PERCENT SLOPES
Ca	CARTECAY SILT LOAM, FREQUENTLY FLOODED
CeB	CECIL SANDY LOAM 2-6 PERCENT SLOPES
CC2	CECIL-URBAN LAND COMPLEX, 2-10 PERCENT SLOPES
PD	PACOLET-URBAN LAND COMPLEX, 10-25 PERCENT SLOPES
Tf	TACCOGA SANDY LOAM, 0-2 PERCENT SLOPES, FREQUENTLY FLOODED
W	WATER

**STATE WATERS EXISTS WITHIN 200 FEET OF THE SITE. THE RECEIVING WATERS FROM THIS SITE IS A TRIBUTARY OF SOUTH FORK PEACHTREE CREEK.**

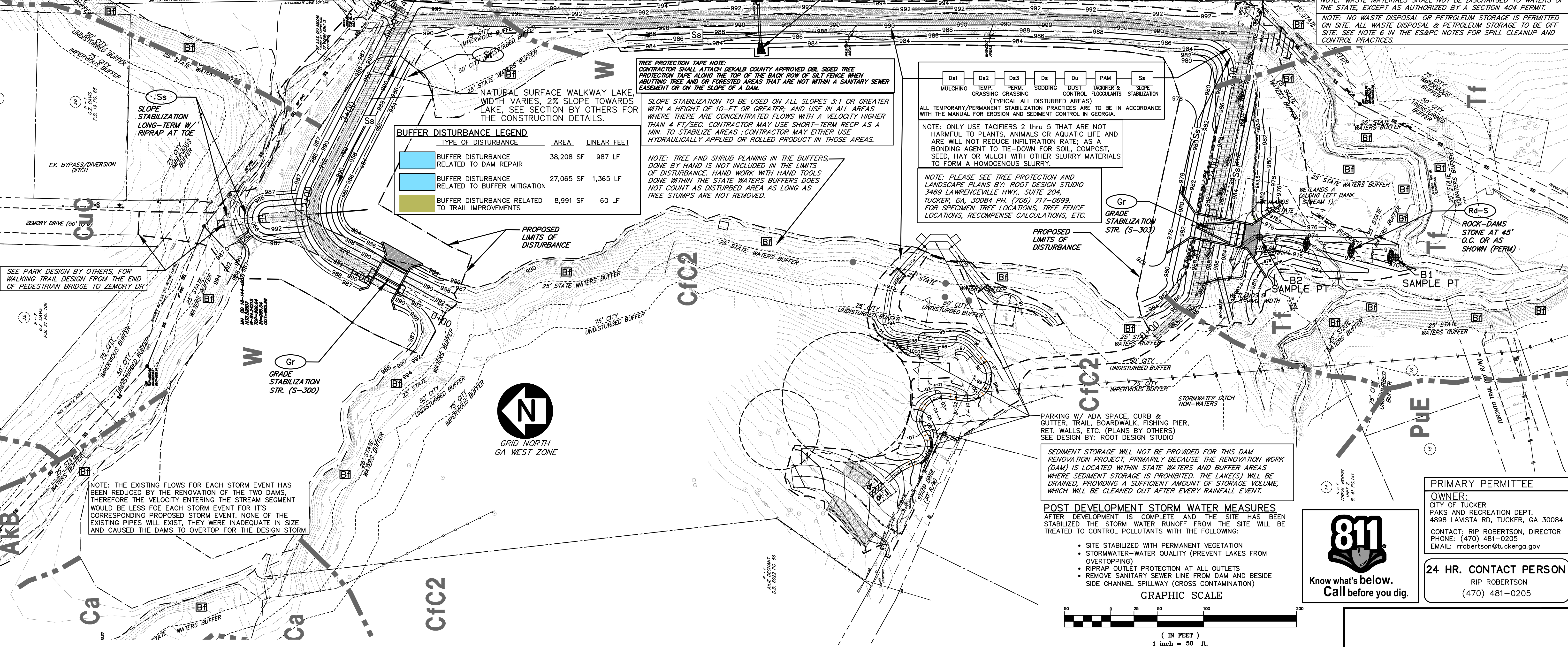
**THE ESCAPE OF SEDIMENT FROM THE SITE SHALL BE PREVENTED BY THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND PRACTICES PRIOR TO LAND-DISTURBING ACTIVITIES. EROSION CONTROL MEASURES WILL BE MAINTAINED AT ALL TIMES. IF FULL IMPLEMENTATION OF THE APPROVED PLAN DOES NOT PROVIDE FOR EFFECTIVE EROSION CONTROL, ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED TO CONTROL OR TREAT THE SEDIMENT SOURCE, OR AS DIRECTED BY THE EROSION CONTROL INSPECTOR.**

**ANY DISTURBED AREA LEFT EXPOSED FOR A PERIOD GREATER THAN 14 DAYS SHALL BE STABILIZED WITH MULCH OR TEMPORARY SEEDING.**

**NOTE:** ALL NON-EXEMPT ACTIVITIES SHALL NOT BE CONDUCTED WITHIN THE 25 OR 50' UNDISTURBED STREAM BUFFERS AS MEASURED FROM THE POINT OF WRESTED VEGETATION OR WITHIN 25-FT OF THE COASTAL MARSHLAND BUFFER AS MEASURED FROM THE JURISDICTIONAL DETERMINATION LINE, WITHOUT FIRST ACQUIRING THE NECESSARY VARIANCES AND PERMITS.

**NOTE:** WASTE MATERIALS SHALL NOT BE DISCHARGED TO WATERS OF THE STATE, EXCEPT AS AUTHORIZED BY A SECTION 404 PERMIT.

**NOTE:** NO WASTE DISPOSAL OR PETROLEUM STORAGE IS PERMITTED ON SITE. ALL WASTE DISPOSAL & PETROLEUM STORAGE TO BE OFF SITE. SEE NOTE 6 IN THE ES&PC NOTES FOR SPILL CLEANUP AND CONTROL PRACTICES.



**BUFFER DISTURBANCE LEGEND**

TYPE OF DISTURBANCE	AREA	LINEAR FEET
BUFFER DISTURBANCE RELATED TO DAM REPAIR	38,208 SF	987 LF
BUFFER DISTURBANCE RELATED TO BUFFER MITIGATION	27,065 SF	1,365 LF
BUFFER DISTURBANCE RELATED TO TRAIL IMPROVEMENTS	8,991 SF	60 LF

**TREE PROTECTION TAPE NOTE:** CONTRACTOR SHALL ATTACH DEKALB COUNTY APPROVED DBL SIDED TREE PROTECTION TAPE ALONG THE TOP OF THE BACK ROW OF SILT FENCE WHEN ADJUTING TREE AND/OR FORESTED AREAS THAT ARE NOT WITHIN A SANITARY SEWER EASEMENT OR ON THE SLOPE OF A DAM.

**SLOPE STABILIZATION TO BE USED ON ALL SLOPES 3:1 OR GREATER WITH A HEIGHT OF 10-FT OR GREATER, AND USE IN ALL AREAS WHERE THERE ARE CONCENTRATED FLOWS WITH A VELOCITY HIGHER THAN 4 FT/SEC. CONTRACTOR MAY USE SHORT-TERM RECP AS A MIN. TO STABILIZE AREAS; CONTRACTOR MAY EITHER USE HYDRAULICALLY APPLIED OR ROLLED PRODUCT IN THOSE AREAS.**

**NOTE:** TREE AND SHRUB PLANING IN THE BUFFERS, DONE BY HAND IS NOT INCLUDED IN THE LIMITS OF DISTURBANCE. HAND WORK WITH HAND TOOLS DONE WITHIN THE STATE WATERS BUFFERS DOES NOT COUNT AS DISTURBED AREA AS LONG AS TREE STUMPS ARE NOT REMOVED.

Ds1	MULCHING
Ds2	TEMP. GRASSING
Ds3	PERM. GRASSING
Ds4	SODDING
Du	DUST TACKIFIER
PAM	PERM. MATTING
Ss	SLOPE STABILIZATION

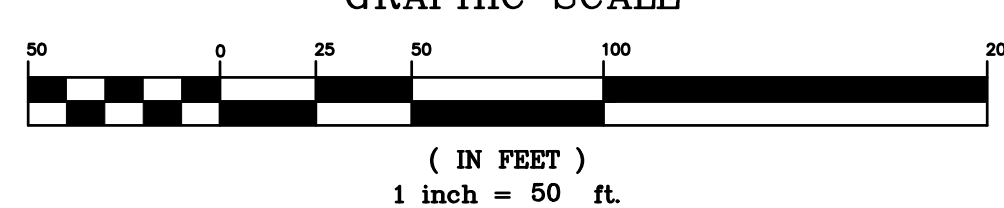
**NOTE:** ONLY USE TACKIFIERS 2 thru 4 THAT ARE NOT HARMFUL TO PLANTS, ANIMALS OR AQUATIC LIFE AND ARE WILL NOT REDUCE INFILTRATION RATE; AS A BONDING AGENT TO TIE-DOWN FOR SOIL, COMPOST, SEED, HAY OR MULCH WITH OTHER SLURRY MATERIALS TO FORM A HOMOGENOUS SLURRY.

**NOTE:** PLEASE SEE TREE PROTECTION AND LANDSCAPE PLANS BY: ROOT DESIGN STUDIO 3469 LAWRENCEVILLE HWY., SUITE 204, TUCKER, GA, 30084 PH. (706) 717-0699 FOR SPECIMEN TREE LOCATIONS, TREE FENCE LOCATIONS, RECOMPENSE CALCULATIONS, ETC.

**SEDIMENT STORAGE WILL NOT BE PROVIDED FOR THIS DAM RENOVATION PROJECT, PRIMARILY BECAUSE THE RENOVATION WORK (DAM) IS LOCATED WITHIN STATE WATERS AND BUFFER AREAS WHERE SEDIMENT STORAGE IS PROHIBITED. THE LAKE(S) WILL BE DRAINED, PROVIDING A SUFFICIENT AMOUNT OF STORAGE VOLUME, WHICH WILL BE CLEANED OUT AFTER EVERY RAINFALL EVENT.**

**POST DEVELOPMENT STORM WATER MEASURES**  
AFTER DEVELOPMENT IS COMPLETE AND THE SITE HAS BEEN STABILIZED THE STORM WATER RUNOFF FROM THE SITE WILL BE TREATED TO CONTROL POLLUTANTS WITH THE FOLLOWING:

- SITE STABILIZED WITH PERMANENT VEGETATION
- STORMWATER-WATER QUALITY (PREVENT LAKES FROM OVERTOPPING)
- RIPRAP OUTLET PROTECTION AT ALL OUTLETS
- REMOVE SANITARY SEWER LINE FROM DAM AND BESIDE SIDE CHANNEL SPILLWAY (CROSS CONTAMINATION)



**PRIMARY PERMITTEE**

**OWNER:**  
CITY OF TUCKER  
PARKS AND RECREATION DEPT.  
4998 LAVISTA RD, TUCKER, GA 30084  
**CONTACT:** RIP ROBERTSON, DIRECTOR  
PHONE: (470) 481-0205  
EMAIL: rrobertson@tuckerga.gov

**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205

NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

06/19/2024

SCALE:	1"=50'	DATE:	01/23
DESIGN BY:	JR/JH	DATE:	01/23
DRAWN BY:	JH/EC	DATE:	01/23
CHECKED BY:	JR	DATE:	01/23
CD#:	PEF 000707	EXP:	06/30/2024
JASON RAPPLEAN, PE, EDR	No. 30441	LEVEL 2 CERTIFICATION #	28931
EXPIRATION DATE:	12/07/26		

**JOHNS HOMESTEAD PARK**  
**EROSION CONTROL**  
**FINAL PLAN**

**CITY OF TUCKER AND DEKALB COUNTY**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
CONSULTING ENGINEERS  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
(770) 956 - 7879

4201600 **42016-C-204** **0**



SPECIES	1000/FT2 ACRES		MTS.-L-STONE	PIEDMONT	COASTAL	REMARKS
	LB	LBS.				
BAHIA WILMINGTON (PASPALUM NOTATUM) ALONE OR TEMP. COVER WITH OTHER PERENNIALS	1.4 0.7	60 30	1/1 - 12/31	1/1 - 12/31		166,000 SEED PER POUND (P.L.S.). LOW GROWING. SOD FORMING. SLOW TO ESTABLISH. PLANT WITH A COMPANION CROP. WILL SPREAD INTO BERMUDA PASTURES AND LAWNS.
BERMUDA, COMMON (CYNODON DACTYLON) UNMULLED SEED WITH OTHER PERENNIALS	0.2 0.1	10 5	5/1 - 6/30	2/15 - 6/30		1,787,000 SEED PER POUND (P.L.S.). QUICK COVER. LOW GROWING AND SOD FORMING. FULL SUN. GOOD FOR ATHLETIC FIELDS.
BERMUDA, COMMON (CYNODON DACTYLON) UNMULLED SEED WITH OTHER PERENNIALS	0.2 0.1	10 5	10/1 - 2/28	11/1 - 1/31		PLANT WITH WINTER ANNUALS.
CENTPEDE, (EREMOCHLOA OPHIURODES)	BLOCK SOD ONLY		4/1 - 6/30	4/1 - 6/30		DROUGHT TOLERANT. FULL SUN OR PARTIAL SHADE. EFFECTIVE ADJACENT TO CONCRETE AND IN CONCENTRATED FLOW AREAS. IRRIGATION IS NEEDED UNTIL FULLY ESTABLISHED. DO NOT PLANT NEAR PASTURES. WINTERHARDY AS FAR NORTH AS ATHENS AND ATLANTA.
FESCUE, TALL (FESTUCA ARUNDINACEA) WITH OTHER PERENNIALS	1.1 0.7	50 30	8/1 - 10/31	3/1 - 4/15	8/15 - 10/31	227,000 SEED PER POUND (P.L.S.). USE ALONE ONLY ON BETTER SITES. NOT FOR DROUGHT SOILS MIX WITH PERENNIAL OR CROWNVEICH. APPLY TOPDRESSING IN SPRING FOLLOWING FALL PLANTINGS. NOT FOR HEAVY USE AREAS OR ATHLETIC FIELDS.

M-L REPRESENTS THE MOUNTAIN; BLUE RIDGE; AND RIDGES AND VALLEYS MLRAS  
 P REPRESENTS THE SOUTHERN PIEDMONT MLRA  
 C REPRESENTS SOUTHERN COASTAL PLAIN; SAND HILLS; BLACK LANDS; AND ATLANTIC COAST FLATWOODS MLRAS

**DEFINITION**  
 THE PLANTING OF PERENNIAL VEGETATION SUCH AS TREES, SHRUBS, VINES, GRASSES, OR LEGUMES ON EXPOSED AREAS FOR FINAL PERMANENT STABILIZATION. PERMANENT PERENNIAL VEGETATION SHALL BE USED TO ACHIEVE FINAL STABILIZATION.

**CONDITIONS**  
 PERMANENT PERENNIAL VEGETATION IS USED TO PROVIDE A PROTECTIVE COVER FOR EXPOSED AREAS INCLUDING CUTS, FILLS, DAMS, AND OTHER DENUDED AREAS.

**GRADING AND SHAPING**  
 GRADING AND SHAPING MAY NOT BE REQUIRED WHERE HYDRAULIC SEEDING AND FERTILIZING EQUIPMENT IS TO BE USED. VERTICAL BANKS SHALL BE SLOPED TO ENABLE PLANT ESTABLISHMENT.

WHEN CONVENTIONAL SEEDING AND FERTILIZING ARE TO BE DONE, GRADE AND SHAPE WHERE FEASIBLE AND PRACTICAL, SO THAT EQUIPMENT CAN BE USED SAFELY AND EFFICIENTLY DURING SEEDBED PREPARATION, SEEDING, MULCHING AND MAINTENANCE OF THE VEGETATION.

CONCENTRATIONS OF WATER THAT WILL CAUSE EXCESSIVE SOIL EROSION SHALL BE DIVERTED TO A SAFE OUTLET. DIVERSIONS AND OTHER TREATMENT PRACTICES SHALL CONFORM WITH THE APPROPRIATE STANDARDS AND SPECIFICATIONS.

**SEEDBED PREPARATION**  
 SEEDBED PREPARATION MAY NOT BE REQUIRED WHERE HYDRAULIC SEEDING AND FERTILIZING EQUIPMENT IS TO BE USED. WHEN CONVENTIONAL SEEDING IS TO BE USED, SEEDBED PREPARATION WILL BE DONE AS FOLLOWS:

**BROADCAST PLANTINGS**

- TILLAGE AT A MINIMUM, SHALL ADEQUATELY LOOSEN THE SOIL TO A DEPTH OF 4 TO 6 INCHES; ALLEVIATE COMPACTION; INCORPORATE LIME AND FERTILIZER; SMOOTH AND FIRM THE SOIL; ALLOW FOR THE PROPER PLACEMENT OF SEED, SPRIGS, OR PLANTS; AND ALLOW FOR THE ANCHORING OF STRAW OR HAY MULCH IF A DISK IS TO BE USED.
- TILLAGE MAY BE DONE WITH ANY SUITABLE EQUIPMENT.
- TILLAGE SHOULD BE DONE ON THE CONTOUR WHERE FEASIBLE.
- ON SLOPES TOO STEEP FOR THE SAFE OPERATION OF TILLAGE EQUIPMENT, THE SOIL SURFACE SHALL BE PITTED OR TRENCHED ACROSS THE SLOPE WITH APPROPRIATE HAND TOOLS TO PROVIDE TWO PLACES 6 TO 8 INCHES APART IN WHICH SEED MAY LODGE AND GERMINATE. HYDRAULIC SEEDING MAY ALSO BE USED.

**INDIVIDUAL PLANTS**

- WHERE INDIVIDUAL PLANTS ARE TO BE SET, THE SOIL SHALL BE PREPARED BY EXCAVATING HOLES, OPENING FURROWS, OR DIBBLE PLANTING.
- FOR NURSERY STOCK PLANTS, HOLES SHALL BE LARGE ENOUGH TO ACCOMMODATE ROOTS WITHOUT CROWDING.
- WHERE PINE SEEDLINGS ARE TO BE PLANTED, SUBSOIL UNDER THE ROW 36 INCHES DEEP ON THE CONTOUR FOUR TO SIX MONTHS PRIOR TO PLANTING. SUBSOILING SHOULD BE DONE WHEN THE SOIL IS DRY, PREFERABLY IN AUGUST OR SEPTEMBER.

**MULCHING**

**HYDRAULIC SEEDING**  
 MIX THE SEED (INOCULATED IF NEEDED), FERTILIZER, AND WOOD CELLULOSE OR WOOD PULP FIBER MULCH WITH WATER AND APPLY IN A SLURRY UNIFORMLY OVER THE AREA TO BE TREATED. APPLY WITHIN ONE HOUR AFTER THE MIXTURE IS MADE.

**CONVENTIONAL SEEDING**  
 SEEDING WILL BE DONE ON A FRESHLY PREPARED AND FIRMED SEEDBED. FOR BROADCAST PLANTING, USE A CULTIPACKER SEEDER, DRILL, ROTARY SEEDER, OTHER MECHANICAL SEEDER, OR HAND SEEDING TO DISTRIBUTE THE SEED UNIFORMLY OVER THE AREA TO BE TREATED. COVER THE SEED LIGHTLY WITH 1/2 TO 1/4 INCH OF SOIL FOR SMALL SEED AND 1/2 TO 1 INCH FOR LARGE SEED WHEN USING A CULTIPACKER OR OTHER SUITABLE EQUIPMENT.

**NO-TILL SEEDING**  
 NO-TILL SEEDING IS PERMISSIBLE INTO ANNUAL COVER CROPS WHEN PLANTING IS DONE FOLLOWING MATURITY OF THE COVER CROP OR IF THE TEMPORARY COVER STAND IS SPARSE ENOUGH TO ALLOW ADEQUATE GROWTH OF PERMANENT (PERENNIAL) SPECIES. NO-TILL SEEDING SHALL BE DONE WITH APPROPRIATE NO-TILL SEEDING EQUIPMENT. THE SEED MUST BE UNIFORMLY DISTRIBUTED AND PLANTED AT THE PROPER DEPTH.

**INDIVIDUAL PLANTS**  
 SHRUBS, VINES AND SPRIGS MAY BE PLANTED WITH APPROPRIATE PLANTERS OR HAND TOOLS. PINE TREES SHALL BE PLANTED MANUALLY IN THE SUBSOIL FURROW. EACH PLANT SHALL BE SET IN A MANNER THAT WILL AVOID CROWDING THE ROOTS. NURSERY STOCK PLANTS SHALL BE PLANTED AT THE SAME DEPTH OR SLIGHTLY DEEPER THAN THEY GREW AT THE NURSERY. THE TIPS OF VINES AND SPRIGS MUST BE AT OR SLIGHTLY ABOVE THE GROUND SURFACE WHERE INDIVIDUAL HOLES ARE DUG. FERTILIZER SHALL BE PLACED IN THE BOTTOM OF THE HOLE, TWO INCHES OF SOIL SHALL BE ADDED AND THE PLANT SHALL BE SET IN THE HOLE.

**MULCHING**

MULCH IS REQUIRED FOR ALL PERMANENT VEGETATION APPLICATIONS. MULCH APPLIED TO SEEDED AREAS SHALL ACHIEVE 75% SOIL COVER. SELECT THE MULCHING MATERIAL FROM THE FOLLOWING AND APPLY AS INDICATED:

- DRY STRAW OR DRY HAY OF GOOD QUALITY AND FREE OF WEED SEEDS CAN BE USED. DRY STRAW SHALL BE APPLIED AT THE RATE OF 2 TONS PER ACRE. DRY HAY SHALL BE APPLIED AT A RATE OF 2 1/2 TONS PER ACRE.
- WOOD CELLULOSE MULCH OR WOOD PULP FIBER SHALL BE USED WITH HYDRAULIC SEEDING. IT SHALL BE APPLIED AT THE RATE OF 500 POUNDS PER ACRE. DRY STRAW OR DRY HAY SHALL BE APPLIED AT THE RATE INDICATED ABOVE AFTER HYDRAULIC SEEDING.
- ONE THOUSAND POUNDS OF WOOD CELLULOSE OR WOOD PULP FIBER; WHICH INCLUDES A TACKIFIER, SHALL BE USED WITH HYDRAULIC SEEDING ON SLOPES 1/4 OR STEEPER.
- SERICEA LESPEDeza HAY CONTAINING MATURE SEED SHALL BE APPLIED AT A RATE OF THREE TONS PER ACRE.
- PINE STRAW OR PINE BARK SHALL BE APPLIED AT A THICKNESS OF THREE INCHES FOR BEDDING PURPOSES. OTHER SUITABLE MATERIALS IN SUFFICIENT QUANTITY MAY BE USED WHERE ORNAMENTALS OR OTHER GROUND COVERS ARE PLANTED. THIS IS NOT APPROPRIATE FOR SEEDED AREAS.
- WHEN USING TEMPORARY EROSION CONTROL BLANKETS OR BLOCK SOD, MULCH IS NOT REQUIRED.
- BITUMINOUS TREATED ROVING MAY BE APPLIED ON PLANTED AREAS ON SLOPES, IN DITCHES OR DRY WATERWAYS TO PREVENT EROSION. BITUMINOUS TREATED ROVING SHALL BE APPLIED WITHIN 24 HOURS AFTER AN AREA HAS BEEN PLANTED. APPLICATION RATES AND MATERIALS MUST MEET GEORGIA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS.

WOOD CELLULOSE AND WOOD PULP FIBERS SHALL NOT CONTAIN GERMINATION OR GROWTH INHIBITING FACTORS. THEY SHALL BE EVENLY DISPERSED WHEN AGITATED IN WATER. THE FIBERS SHALL CONTAIN A DYE TO ALLOW VISUAL METERING AND AID IN UNIFORM APPLICATION DURING SEEDING.

**ANCHORING MULCH**  
 ANCHOR STRAW OR HAY MULCH IMMEDIATELY AFTER APPLICATION BY ONE OF THE FOLLOWING METHODS:  
 1. EMULSIFIED ASPHALT CAN BE (A) SPRAYED UNIFORMLY ONTO THE MULCH AS IT IS EJECTED FROM THE BLOWER MACHINE OR (B) SPRAYED ON THE MULCH IMMEDIATELY FOLLOWING MULCH APPLICATION WHEN STRAW OR HAY IS SPREAD BY METHODS OTHER THAN SPECIAL BLOWER EQUIPMENT.

THE COMBINATION OF ASPHALT EMULSION AND WATER SHALL CONSIST OF A HOMOGENEOUS MIXTURE SATISFACTORY OF SPRAYING. THE MIXTURE SHALL CONSIST OF 100 GALLONS OF GRADE SS-1H OR CSS-1H EMULSIFIED ASPHALT AND 100 GALLONS OF WATER PER TON OF MULCH. CARE SHALL BE TAKEN AT ALL TIMES TO PROTECT STATE WATERS, THE PUBLIC, ADJACENT PROPERTY, PAVEMENTS, CURBS, SIDEWALKS, AND ALL OTHER STRUCTURES FROM ASPHALT DISCOLORATION.

2. HAY AND STRAW MULCH SHALL BE PRESSED INTO THE SOIL IMMEDIATELY AFTER THE MULCH IS SPREAD. A SPECIAL "PACKER DISK" OR DISK HARROW WITH THE DISKS SET STRAIGHT MAY BE USED. THE DISKS MAY BE SMOOTH OR SERRATED AND SHOULD BE 20 INCHES OR MORE IN DIAMETER AND 8 TO 12 INCHES APART. THE EDGES OF THE DISKS SHALL BE DULL ENOUGH TO PRESS THE MULCH INTO THE GROUND WITHOUT CUTTING IT, LEAVING MUCH OF IT IN AN UPRIGHT POSITION. MULCH SHALL NOT BE PLOWED INTO THE SOIL.

3. SYNTHETIC TACKIFIERS OR BINDERS APPROVED BY GOVT SHALL BE APPLIED WITH OR IMMEDIATELY AFTER THE MULCH IS SPREAD. SYNTHETIC TACKIFIERS SHALL BE MIXED AND APPLIED ACCORDING TO MANUFACTURER'S SPECIFICATIONS. REFER TO Td-TACKIFIERS AND BINDERS.

4. RYE AND WHEAT CAN BE INCLUDED WITH FALL AND WINTER PLANTINGS TO STABILIZE THE MULCH. THEY SHALL BE APPLIED AT A RATE OF ONE-QUARTER TO ONE-HALF BUSHEL PER ACRE.

5. PLASTIC MESH OR NETTING WITH MESH NO LARGER THAN ONE INCH BY ONE INCH MAY BE NEEDED TO ANCHOR STRAW OR HAY MULCH ON UNSTABLE SOILS AND CONCENTRATED FLOW AREAS. THESE MATERIALS SHALL BE INSTALLED AND ANCHORED ACCORDING TO MANUFACTURER'S SPECIFICATIONS.

**IRRIGATION**  
 IRRIGATION SHALL BE APPLIED AT A RATE THAT WILL NOT CAUSE RUNOFF.

NOTE:  
 CONTRACTOR SHALL COMPLY WITH CONSTRUCTION SPECIFICATION 26 TOP SOILING.

**Ds3 DISTURBED AREA STABILIZATION (WITH PERMANENT VEGETATION)**

SPECIES	1000 FT2 ACRES		MTS.-L-STONE	PIEDMONT	COASTAL	REMARKS
	LB	LBS.				
BARLEY (HORDEUM VULGARE) ALONE IN MIXTURES	3.3 0.6	3 BU (144 LBS) 1/2 BU (24 LBS)	8/15-11/15	8/15-12/15	9/1-12/31	14,000 SEED PER POUND (P.L.S.). WINTERHARDY. USE ON PRODUCTIVE SOILS.
MILLET, BROWNTOP (PANICUM FASCICULATUM) ALONE IN MIXTURES	0.9 0.2	40 10	4/1-7/30	4/1-7/20	4/1-7/15	137,000 SEED PER POUND (P.L.S.). DENSE COVER, WILL PROVIDE TOO MUCH COMPETITION IN MIXTURES
MILLET, PEARL (PENNESETUM GLAUCUM) ALONE	1.1	50	5/1-7/31	4/15-8/31	4/1-8/31	DENSE COVER. MAY REACH 6 FEET IN HEIGHT. NOT RECOMMENDED FOR MIXTURES.
OATS (AVENA SATIVA) ALONE IN MIXTURES	2.9 0.7	4 BU (128 LBS) 1 BU (32 LBS)	9/1-11/30	9/1-11/30	9/1-11/30	13,000 SEED PER POUND (P.L.S.). USE ON PRODUCTIVE SOILS. NOT AS WINTER HARDY AS RYE OR BARLEY.
RYE (SECALE CEREALE) ALONE IN MIXTURES	3.9 0.6	3 BU (168 LBS) 1/2 BU (28 LBS)	7/15-11/30	8/15-12/31	9/1-2/28	18,000 SEED PER POUND. QUICK COVER. DROUGHT TOLERANT AND WINTERHARDY.
RYEGRASS, ANNUAL (LOLIUM TEMULENTUM) ALONE	0.9	40	8/1-4/30	8/1-4/15	8/15-3/31	227,000 SEED PER POUND (P.L.S.). DENSE COVER. VERY COMPETITIVE AND IS NOT TO BE USED IN MIXTURES.
SUDANGRASS (SORGHUM SUDANESE) ALONE	1.4	60	4/1-8/31	4/1-8/31	3/1-7/31	55,000 SEED PER POUND (P.L.S.). GOOD ON DROUGHTY SITES. NOT RECOMMENDED FOR MIXTURES.
TRITICALE (X-TRITICOSECALE) ALONE IN MIXTURES	3.3 0.6	3 BU (144 LBS) 1/2 BU (24 LBS)		9/15-1/31		USE ON LOWER PART OF SOUTHERN COASTAL PLAIN AND IN ATLANTIC COASTAL FLATWOODS ONLY.
WHEAT (TRITICUM AESTIVUM) ALONE IN MIXTURES	4.1 0.7	3 BU (180 LBS) 1/2 BU (30 LBS)	9/1-12/31	9/1-12/31	9/15-1/31	15,000 SEED PER POUND (P.L.S.). WINTERHARDY.

GRASSING TABLE

**DEFINITION**  
 THE ESTABLISHMENT OF TEMPORARY VEGETATIVE COVER WITH FAST GROWING SEEDINGS FOR SEASONAL PROTECTION ON DISTURBED OR DENUDED AREAS.

**CONDITIONS**  
 TEMPORARY GRASSING INSTEAD OF MULCH, CAN BE APPLIED TO ROUGH GRADED AREAS THAT WILL BE EXPOSED FOR LESS THAN SIX MONTHS. TEMPORARY VEGETATIVE MEASURES SHOULD BE COORDINATED WITH PERMANENT MEASURES TO ASSURE ECONOMIC AND EFFECTIVE STABILIZATION. MOST TYPES OF TEMPORARY VEGETATION ARE IDEAL TO USE AS COMPANION CROPS UNTIL THE PERMANENT VEGETATION IS ESTABLISHED.

- TEMPORARY COVER CROPS ARE VERY COMPETITIVE AND WILL CROWD OUT PERENNIALS IF SEEDED TOO HEAVILY.
- REDUCE SEEDING RATES BY 50% WHEN DRILLED.
- PLS IS AN ABBREVIATION FOR PURE LIVE SEED.
- M-L REPRESENTS THE MOUNTAIN; BLUE RIDGE; AND RIDGES AND VALLEYS MLRAS  
 P REPRESENTS THE SOUTHERN PIEDMONT MLRA  
 C REPRESENTS SOUTHERN COASTAL PLAIN; SAND HILLS; BLACK LANDS; AND ATLANTIC COAST FLATWOODS MLRAS

**GRADING AND SHAPING**

- EXCESSIVE WATER RUN-OFF SHALL BE REDUCED BY PROPERLY DESIGNED AND INSTALLED EROSION CONTROL PRACTICES SUCH AS CLOSED DRAINS, DITCHES, DIKES, DIVERSIONS, SEDIMENT BARRIERS AND OTHERS.
- NO SHAPING OR GRADING IS REQUIRED IF SLOPES CAN BE STABILIZED BY HAND-SEEDED VEGETATION OR IF HYDRAULIC SEEDING EQUIPMENT IS TO BE USED.

**SEEDBED PREPARATION**

- WHEN A HYDRAULIC SEEDER IS USED, SEEDBED PREPARATION IS NOT REQUIRED. WHEN USING CONVENTIONAL OR HAND-SEEDING, SEEDBED PREPARATION IS NOT REQUIRED IF THE SOIL MATERIAL IS LOOSE AND NOT SEALED BY RAINFALL.
- WHEN SOIL HAS BEEN SEALED BY RAINFALL OR CON-SISTS OF SMOOTH CUT SLOPES, THE SOIL SHALL BE PITTED, TRENCHED OR OTHERWISE SCARIFIED TO PROVIDE A PLACE FOR SEED TO LODGE AND GERMINATE.

**LIME AND FERTILIZER**

AGRICULTURAL LIME IS REQUIRED UNLESS SOIL TESTS INDICATE OTHERWISE. APPLY AGRICULTURAL LIME AT A RATE OF ONE TON PER ACRE. GRADED AREAS REQUIRE LIME APPLICATION. SOILS CAN BE TESTED TO DETERMINE IF FERTILIZER IS NEEDED. ON REASONABLY FERTILE SOILS OR SOIL MATERIAL, FERTILIZER IS NOT REQUIRED. FOR SOILS WITH VERY LOW FERTILITY, 500 TO 700 POUNDS OF 10-10-10 FERTILIZER OR THE EQUIVALENT PER ACRE (12-16 LBS./1000 SQ. FT.) SHALL BE APPLIED. FERTILIZER SHOULD BE APPLIED BEFORE LAND PREPARATION AND INCORPORATED WITH A DISK, RIPPER OR CHISEL.

**SEEDING**

SELECT A GRASS OR GRASS-LEGUME MIXTURE SUITABLE TO THE AREA AND SEASON OF THE YEAR. SEED SHALL BE APPLIED UNIFORMLY BY HAND, CYCLONE SEEDER, DRILL, CULTIPACKER-SEEDER, OR HYDRAULIC SEEDER (SLURRY INCLUDING SEED AND FERTILIZER). DRILL OR CULTIPACKER SEEDERS SHOULD NORMALLY PLACE SEED ONE-QUARTER TO ONE-HALF INCH DEEP. APPROPRIATE DEPTH OF PLANTING IS TEN TIMES THE SEED DIAMETER. SOIL SHOULD BE RAKED LIGHTLY TO COVER SEED WITH SOIL IF SEEDING BY HAND.

**MULCHING**

TEMPORARY VEGETATION CAN, IN MOST CASES, BE ESTABLISHED WITHOUT THE USE OF MULCH. MULCH WITHOUT SEEDING SHOULD BE CONSIDERED FOR SHORT TERM PROTECTION. SEE DS1- DISTURBED AREA STABILIZATION, (WITH MULCHING ONLY).

**IRRIGATION**

DURING TIMES OF DROUGHT, WATER SHALL BE APPLIED AT A RATE NOT CAUSING RUNOFF AND EROSION. THE SOIL SHALL BE THOROUGHLY WETTED TO A DEPTH THAT WILL INSURE GERMINATION OF THE SEED. SUBSEQUENT APPLICATIONS SHOULD BE MADE WHEN NEEDED.

**Ds2 DISTURBED AREA STABILIZATION (WITH TEMPORARY VEGETATION)**

GRASSING NOTES

**Tac TACKIFIERS**

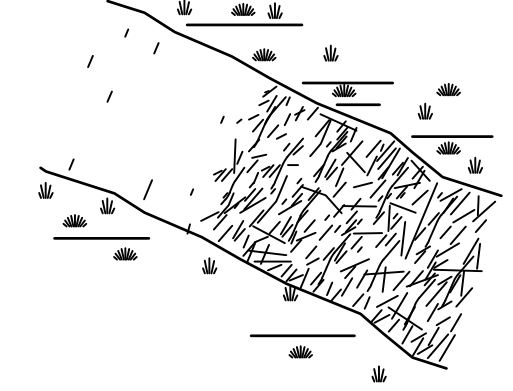
Tackifiers are used as a tie-down for soil, compost, seed, straw, hay or mulch. Tackifiers hydrate in water and readily blend with other slurry materials to form a homogenous slurry.

This practice is intended for direct soil surface application to sites where the timely establishment of vegetation may not be feasible or where vegetation cover is absent or inadequate. Such areas include construction areas, where plant residues are inadequate to protect the soil surface and where land disturbing activities prevent the establishment or maintenance of a vegetative cover.

- Tac-1 Tackifiers: Synthetic Polymers ← DO NOT USE (HARM FULL TO ANIMALS)
  - Tac-2 Tackifiers: Organic Polymers
  - Tac-3 Tackifiers: Synthetic/Organic Polymers Blends
  - Tac-4 Tackifiers: Organic Tackifiers with Synthetic Fibers
  - Tac-5 Tackifiers: Synthetic/Organic Blends with Synthetic Fibers
- Tackified areas should be checked after every rain event. Periodic inspections and required maintenance must be provided per manufacturer's recommendations.

ESTABLISHING A TEMPORARY PROTECTION FOR DISTURBED AREAS USING SPECIFIC MULCH MATERIALS.

- MULCH MATERIALS SHALL CONSIST OF DRY STRAW OR HAY AT 2.5 TONS PER ACRE, WOOD CHIPS AT 6 TO 9 TONS PER ACRE, EROSION CONTROL MATTING OR NETTING, OR POLYETHYLENE FILM.
- THIS STANDARD APPLIED TO GRADES OR CLEARED AREAS WHICH MAY BE SUBJECT TO EROSION CONTROL FOR 6 MONTHS OR LESS, AND CAN BE STABILIZED WITH A MULCH COVER.



**Ds1 DISTURBED AREA STABILIZATION (WITH MULCHING)**

**FI-Co**

FLOCCULANTS COAGULANTS

**Planning Considerations** Since settling of flocculated soil particles requires very slow moving (still) water, chemical additives should never be introduced into an outfall BMP where water leaves the property or enters state waters. In all cases where chemical additives are used to reduce turbidity, it is essential to include a sediment basin or sediment trap unless using a "pump and treat" treatment system.

**CRITERIA** Application rates shall conform to manufacturer's guidelines for application. Only anionic forms of FI-Co shall be used.

Following are examples of FI-Co applications within construction storm water ditches or drainageways that feed into sediment basins or other BMPs:

- \*FI-Co Bags or Socks that are installed directly in a ditch, pipe or culvert.
- \*FI-Co treated ditch checks (i.e. fiber rolls, wattles, or compost logs inoculated or used in conjunction with FI-Co).
- \*Granulated FI-Co treated rock ditch checks.
- \*Ditch checks with attached FI-Co Bags or Socks.
- \*Addition of granular FI-Co directly into a ditch.
- \*Erosion control blankets and turf reinforcement mats that have been inoculated with a FI-Co.
- \*"Pump and Treat" systems that use mechanical mixing with a chemical treatment of a FI-Co.

**IRRIGATION** THIS IS GENERALLY DONE AS AN EMERGENCY TREATMENT. SITE IS SPRINKLED WITH WATER UNTIL THE SURFACE IS WET. REPEAT AS NEEDED.

**BARRIERS** SOLID BOARD FENCES, SNOW FENCES, BURLAP FENCES, CRATE WALLS, BALES OF HAY AND SIMILAR MATERIAL CAN BE USED TO CONTROL AIR CURRENTS AND SOIL BLOWING. BARRIERS PLACED AT RIGHT ANGLES TO PREVAILING CURRENTS AT INTERVALS OF ABOUT 15 TIMES THEIR HEIGHT ARE EFFECTIVE IN CONTROLLING WIND EROSION.

CALCIUM CHLORIDE. APPLY AT RATE THAT WILL KEEP SURFACE MOIST. MAY NEED RETREATMENT.

**B. PERMANENT METHODS**

**PERMANENT VEGETATION** SEE STANDARD Ds3-DISTURBED AREA STABILIZATION (WITH PERMANENT VEGETATION). EXISTING TREES AND LARGE SHRUBS MAY AFFORD VALUABLE PROTECTION IF LEFT IN PLACE.

**TOPSOILING** THIS ENTAILS COVERING THE SURFACE WITH LESS EROSION SOIL MATERIAL. SEE STANDARD Tp-TOPSOILING.

**STONE** COVER SURFACE WITH CRUSHED STONE OR COARSE GRAVEL. SEE STANDARD Cr-CONSTRUCTION ROAD STABILIZATION.

**Du DUST CONTROL ON DISTURBED AREAS**

PRIMARY PERMITTEE  
 OWNER:  
 CITY OF TUCKER  
 PAKS AND RECREATION DEPT.  
 4898 LAVISTA RD, TUCKER, GA 30084  
 CONTACT: RIP ROBERTSON, DIRECTOR  
 PHONE: (470) 481-0205  
 EMAIL: rrobertson@tuckerga.gov

**811**  
 Know what's below.  
 Call before you dig.

24 HR. CONTACT PERSON  
 RIP ROBERTSON  
 (470) 481-0205

NO.	REVISIONS	MADE	OKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS		SCALE: AS SHOWN	DATE	<b>JOHNS HOMESTEAD PARK</b> <b>EROSION CONTROL</b> <b>GRASSING DETAIL</b>	<b>WALDEN, ASHWORTH &amp; ASSOCIATES, INC.</b> CONSULTING ENGINEERS P.O. BOX 6462 MARIETTA, GEORGIA 30065 (770) 956 - 7879	4201600	<b>42016-C-205</b>	<b>0</b>
								DESIGN BY: JR/JH	01/23					
								CITY OF TUCKER AND DEKALB COUNTY						



# TYPICAL INSTALLATION GUIDELINES FOR ROLLED EROSION CONTROL PRODUCTS (RECP)

## BLANKET AND MATTING CROSS-SECTIONS

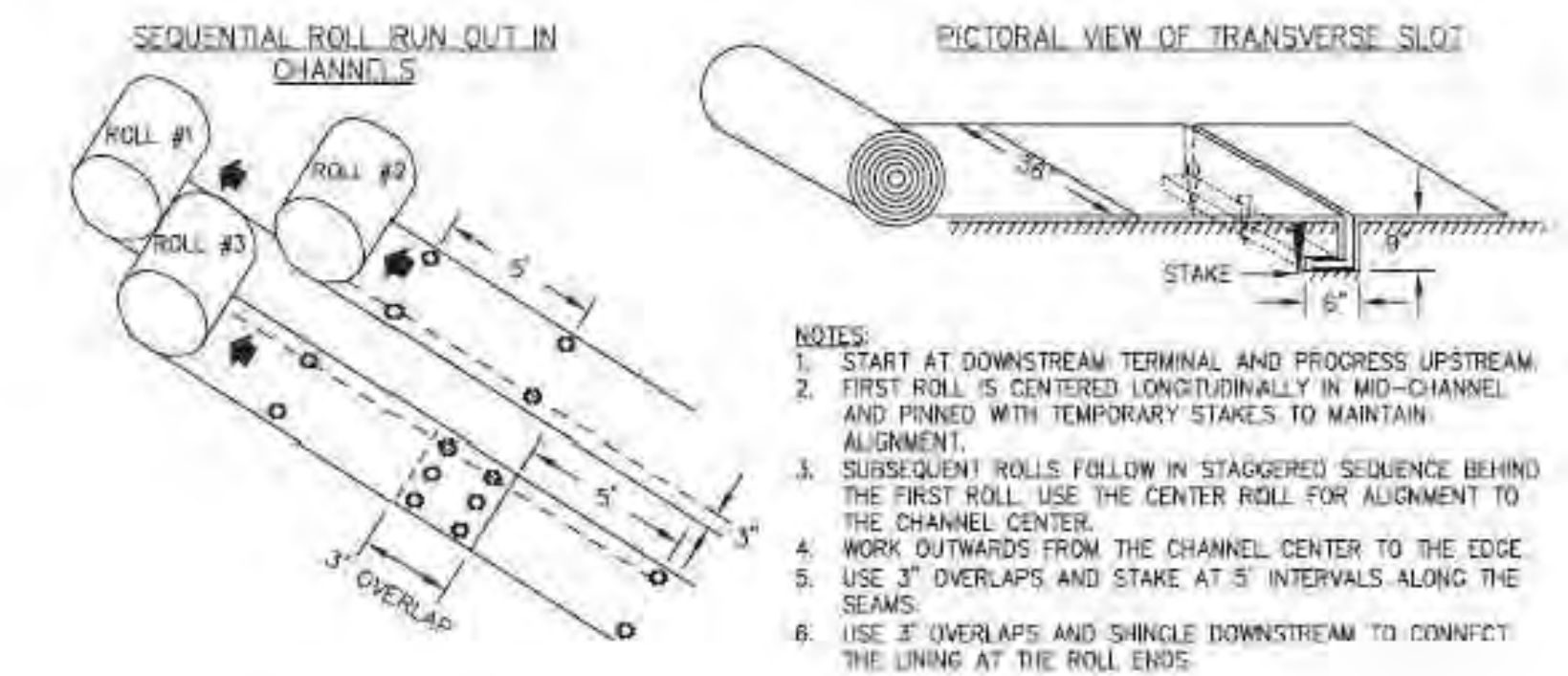
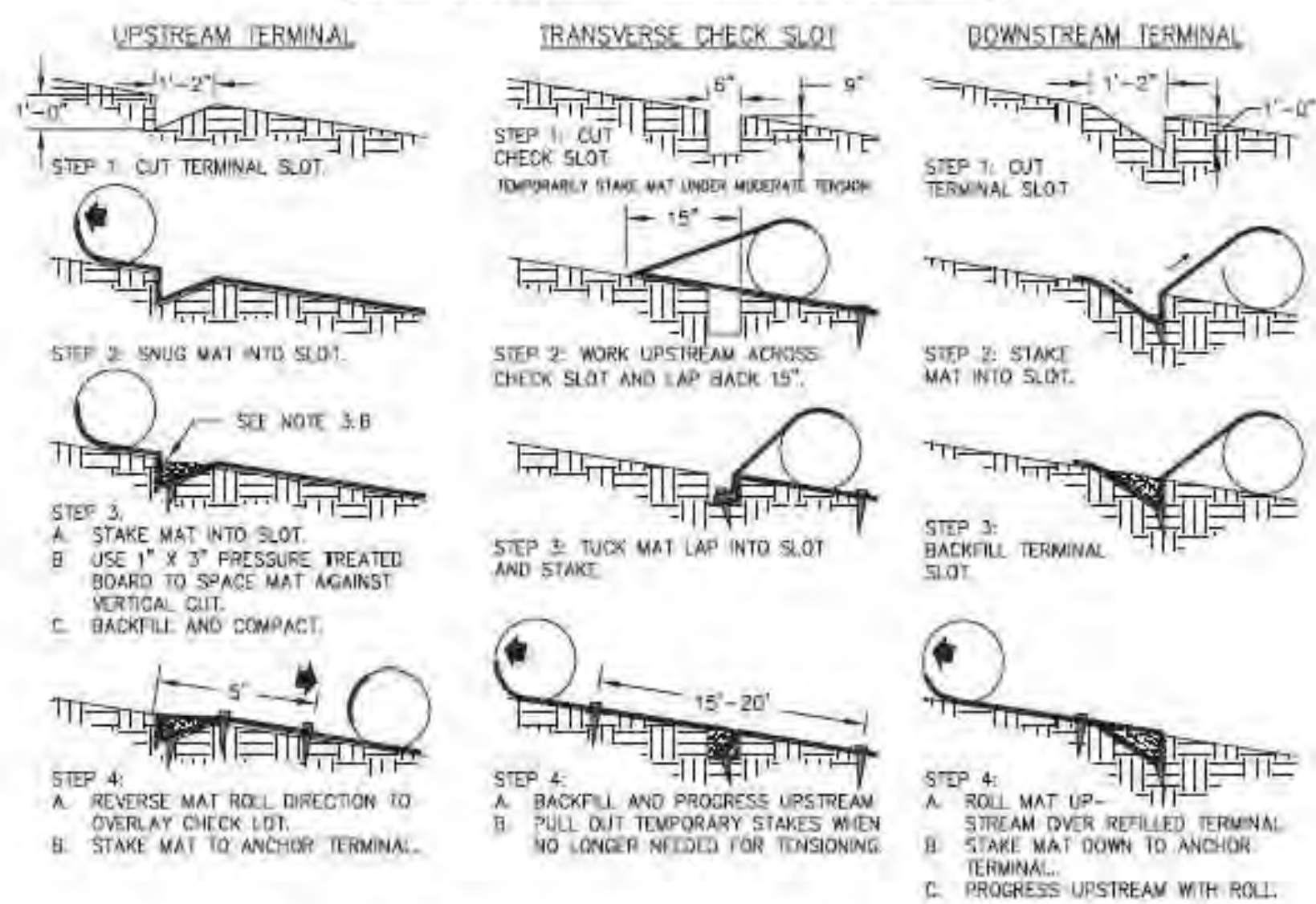
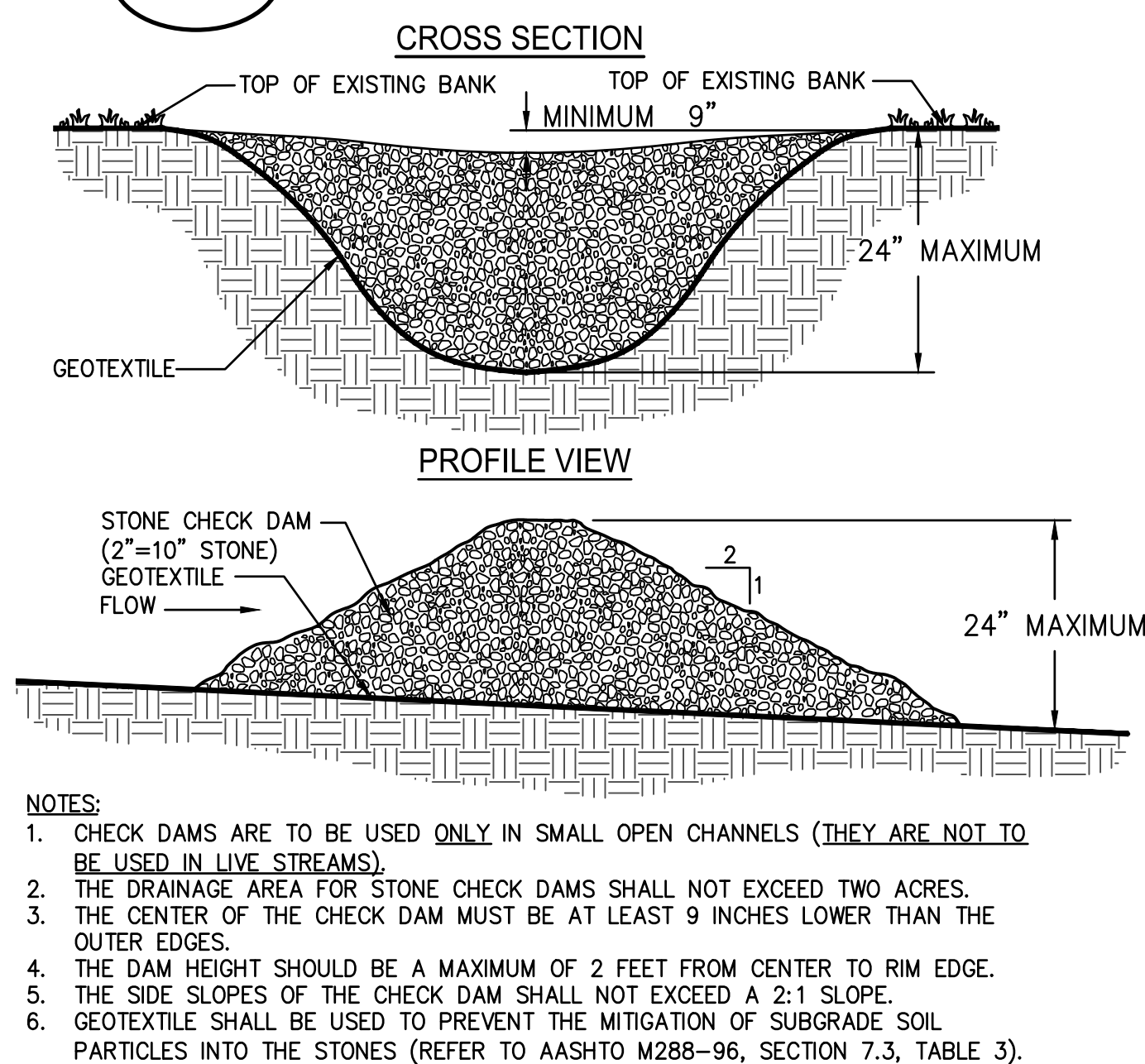


Figure 6-10.1 - Typical Installation Guidelines for Matting and Blankets

## Ss SLOPE STABILIZATION

### Cd STONE CHECK DAM



**SITE PREPARATION:** After the site has been shaped and graded to the approved design, prepare a friable seeded relatively free from clods and rocks more than one inch in diameter, and any foreign material that will prevent contact of the soil stabilization mat with the soil surface. Surface must be smooth to ensure proper contact of blankets or matting to the soil surface. If necessary, redirect any runoff from the ditch or slope during installation.

**MAINTENANCE:** All erosion control blankets and matting should be inspected periodically following installation, particularly after rainstorms to check for erosion and undermining. Any dislocation or failure should be repaired immediately. If washouts or breakage occurs, reinstall the material after repairing damage to the slope or ditch. Continue to monitor these areas until they become permanently stabilized.

**FOR THIS PROJECT:** This project shall utilize blankets for all slopes exceeding slopes of 3:1. For slopes along the roadway and behind the townhome units that exceed ten (10') vertical feet in height, extended term blankets shall be used to allow for vegetation to establish on the slopes and fully stabilize them.

**Short-Term** (functional longevity 12 mo.)  
i. Photodegradable  
Straw blankets with a top and bottom side photo degradable net. The maximum size of the mesh should be openings of 1/2" X 1/2". The blanket should be sewn together on 1.5' centers with degradable thread. Minimum thickness should be 0.35" and minimum density should be 0.5 lbs per square yard.  
ii. Biodegradable  
Straw blanket with a top and bottom side biodegradable jute net. The top side net should consist of machine direction strands that are 5/70 twisted together and then interwoven with cross direction strands (leno weave). The bottom net may be leno weave or otherwise to meet requirements. The approximate size of the mesh should be openings of 0.5" X 1.0". The blanket should be sewn together on 1.5' centers with degradable thread. Minimum thickness should be 0.25" and minimum density should be 0.5 lbs per square yard.

**Extended-Term** (functional longevity 24 mo.)  
i. Photodegradable  
Blankets that consist of 70% straw and 30% coconut with a top and bottom side photodegradable net. The top net should have ultraviolet additives to delay breakdown. The maximum size of the mesh should be openings of 0.65" X 0.65". The blanket should be sewn together on 1.5' centers with degradable thread. Minimum thickness should be 0.35" and minimum density should be 0.6 lbs per square yard.  
ii. Biodegradable  
Blankets that consist of 70% straw and 30% coconut with a top and bottom side biodegradable jute net. The top side net should consist of machine direction strands that are twisted together and then interwoven with cross direction strands (leno weave). The bottom net may be leno weave or otherwise to meet requirements. The approximate size of the mesh should be openings of 0.5" X 1.0". The blanket should be sewn together on 1.5' centers with degradable thread. Minimum thickness should be 0.25" and minimum density should be 0.6 lbs per square yard.

**Long-Term** (functional longevity 36 mo.)  
i. Photodegradable  
Blankets that consist of 100% coconut with a top and bottom side photodegradable net. Each net should have ultraviolet additives to delay breakdown. The maximum size of the mesh should be openings of 0.85" X 0.85". The blanket should be sewn together on 1.5' centers with degradable thread. Minimum thickness should be 0.3" and minimum density should be 0.5 lbs per square yard.  
ii. Biodegradable  
Blankets that consist of 100% coconut with a top and bottom side biodegradable jute net. The top side net should consist of machine direction strands that are twisted together and then interwoven with cross direction strands (leno weave). The bottom net may be leno weave or otherwise to meet requirements. The approximate size of the mesh should be openings of 0.5" X 1.0". The blanket should be sewn together on 1.5' centers with degradable thread. Minimum thickness should be 0.25" and minimum density should be 0.5 lbs per square yard.

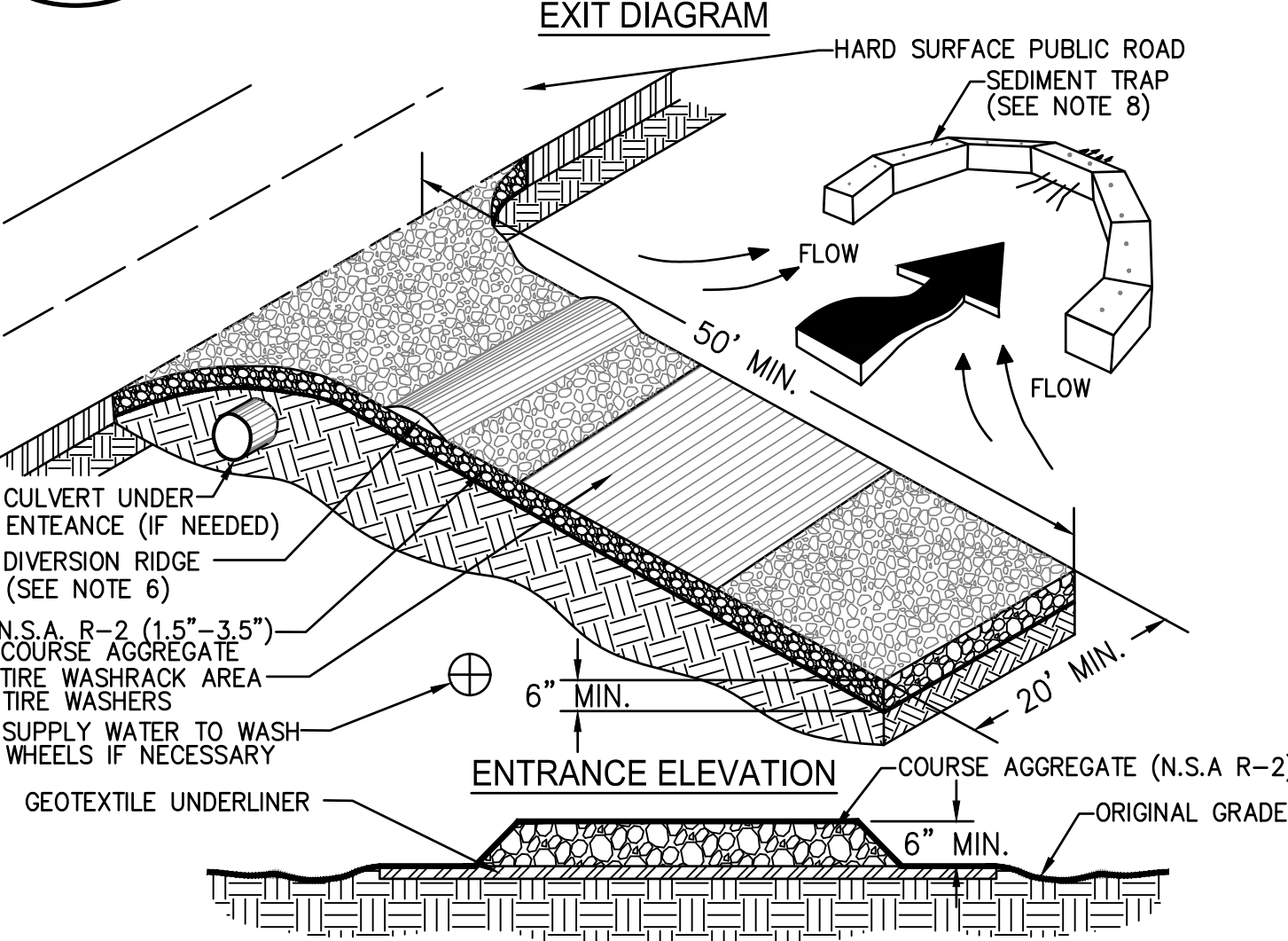
**NOTES:**  
It is the intention of this section to allow interchangeable use of RECPs and HECPs for erosion protection on slopes. The project engineer should select the type of erosion control product that best fits the need of the particular site.  
**Site Preparation:** After the site has been shaped and graded to the approved design, prepare a friable seeded relatively free from clods and rocks more than one inch in diameter, and any foreign material that will prevent contact of the soil stabilization mat with the soil surface. Surface must be smooth to ensure proper contact of blankets or matting to the soil surface. If necessary, redirect any runoff from the ditch or slope during installation.  
**MAINTENANCE:** All erosion control blankets and matting should be inspected periodically following installation, particularly after rainstorms to check for erosion and undermining. Any dislocation or failure should be repaired immediately. If washouts or breakage occurs, reinstall the material after repairing damage to the slope or ditch. Continue to monitor these areas until they become permanently stabilized.

**CRUSHED STONE CONSTRUCTION EXIT**

**EXIT DIAGRAM**  
HARD SURFACE PUBLIC ROAD  
SEDIMENT TRAP (SEE NOTE 8)  
FLOW  
50' MIN.  
20' MIN.  
6" MIN.  
ENTRANCE ELEVATION  
COURSE AGGREGATE (N.S.A R-2)  
ORIGINAL GRADE  
6" MIN.  
CULVERT UNDER-ENTRANCE (IF NEEDED)  
DIVERSION RIDGE (SEE NOTE 6)  
N.S.A R-2 (1.5"-3.5")  
COURSE AGGREGATE  
TIRE WASHRACK AREA  
TIRE WASHERS  
SUPPLY WATER TO WASH WHEELS IF NECESSARY  
GEOTEXTILE UNDERLINER

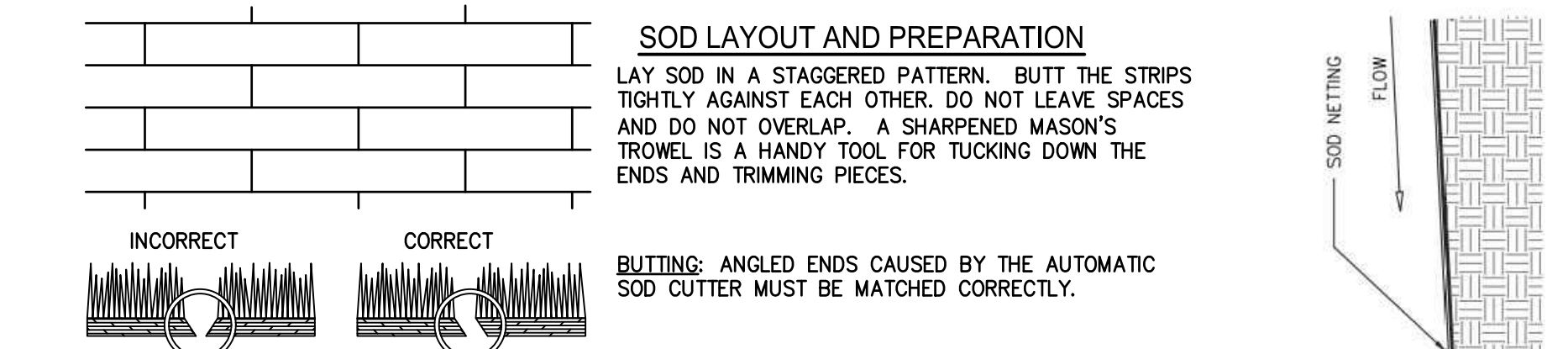
**NOTES:**  
1. AVOID LOCATING ON STEEP SLOPES OR AT CURVES ON PUBLIC ROADS.  
2. REMOVE ALL VEGETATION AND OTHER UNSUITABLE MATERIAL FROM THE FOUNDATION AREA, GRADE, AND CROWN FOR POSITIVE DRAINAGE.  
3. AGGREGATE SIZE SHALL BE IN ACCORDANCE WITH NATIONAL STONE ASSOCIATION R-2 (1.5"-3.5" STONE).  
4. GRAVEL PAD SHALL HAVE A MINIMUM THICKNESS OF 6".  
5. PAD WIDTH SHALL BE EQUAL FULL WIDTH AT ALL POINTS OF VEHICULAR EGRESS, BUT NO LESS THAN 20'.  
6. A DIVERSION RIDGE SHOULD BE CONSTRUCTED WHEN GRADE TOWARD PAVED AREA IS GREATER THAN 2%.  
7. INSTALL PIPE UNDER THE ENTRANCE IF NEEDED TO MAINTAIN DRAINAGE DITCHES.  
8. WHEN WASHING IS REQUIRED, IT SHOULD BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN (DIVERT ALL SURFACE RUNOFF AND DRAINAGE FROM THE ENTRANCE TO A SEDIMENT CONTROL DEVICE).  
9. WASHRACKS AND/OR TIRE WASHERS MAY BE REQUIRED DEPENDING ON SCALE AND CIRCUMSTANCE. IF NECESSARY, WASHRACK DESIGN MAY CONSIST OF ANY MATERIAL SUITABLE FOR TRUCK TRAFFIC THAT REMOVE MUD AND DIRT.  
10. MAINTAIN AREA IN A WAY THAT PREVENTS TRACKING AND/OR FLOW OF MUD ONTO PUBLIC RIGHTS-OF-WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT.

## Co CRUSHED STONE CONSTRUCTION EXIT

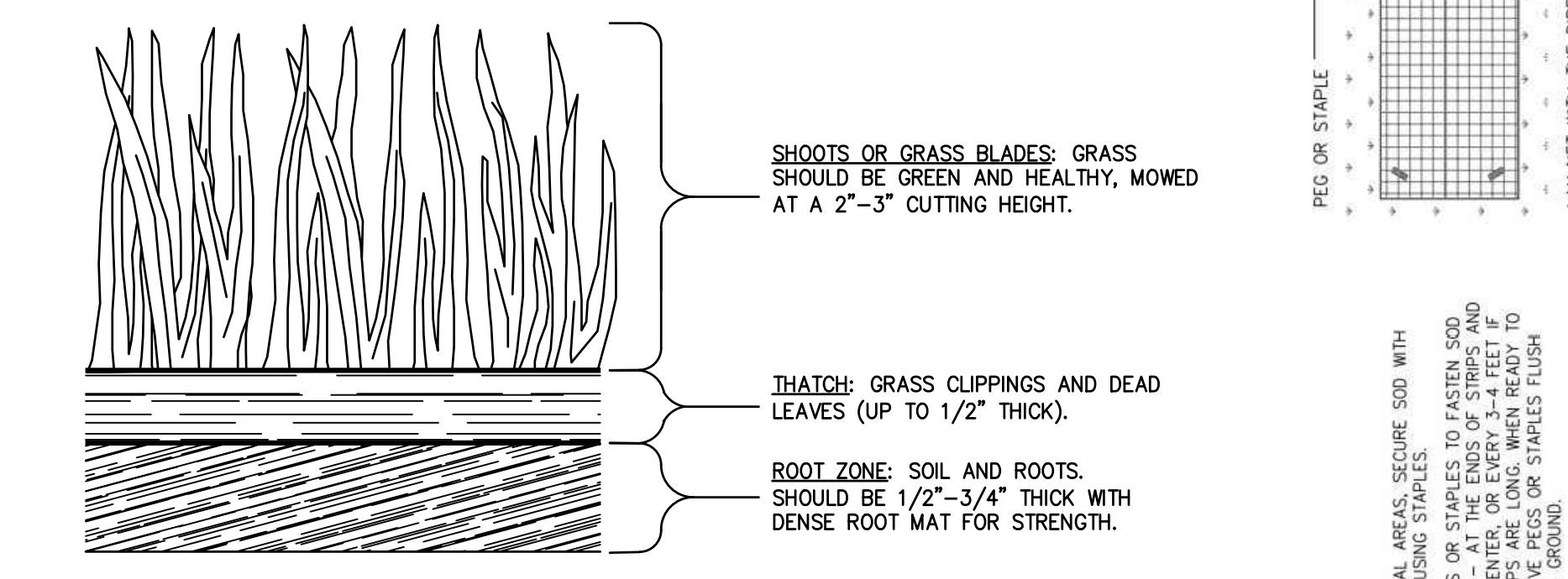


**NOTES:**  
1. AVOID LOCATING ON STEEP SLOPES OR AT CURVES ON PUBLIC ROADS.  
2. REMOVE ALL VEGETATION AND OTHER UNSUITABLE MATERIAL FROM THE FOUNDATION AREA, GRADE, AND CROWN FOR POSITIVE DRAINAGE.  
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**DIRECTIONS FOR INITIAL MAINTENANCE**  
Step 1. ROLL SOD IMMEDIATELY TO ACHIEVE FIRM CONTACT WITH THE SOIL.  
Step 2. WATER TO A DEPTH OF 4" AS NEEDED. WATER WELL AS SOON AS THE SOD IS LAID.  
Step 3. MOW WHEN THE SOD IS ESTABLISHED -- IN 2-3 WEEKS. SET THE MOWER HIGH (2"-3").



AGRICULTURAL LIME SHOULD BE APPLIED BASED ON SOIL TESTS OR AT A RATE OF 1 OR 2 TONS PER ACRE.

SOD PLANTING REQUIREMENTS			
GRASS VARIETIES	RESOURCE AREA	GROWING SEASON	Fertilizer Requirements for Soil Surface Application
BERMUDA GRASS	COMMON TIFWAY TIFGREEN TIFLAWN	M-L,P,C WARM WEATHER	Table 6-6.1
Fertilizer Type	Fertilizer Rate (lbs/acre)	Fertilizer Rate (lbs/sq ft)	Season
10-10-10	1000	.025	Fall

## Ds4 DISTURBED AREA STABILIZATION (WITH SOD)

**Ds4 INSTALLATION:** Lay sod with tight joints and in straight lines. Don't overlap joints. Stagger joints and do not stretch sod (See Figure 6-6.2). On slopes steeper than 3:1, sod should be anchored with pins or other approved methods. Installed sod should be rolled or tamped to provide good contact between sod and soil. Irrigate sod and soil to a depth of 4" immediately after installation. Sod should not be cut or spread in extremely wet or dry weather. Irrigation should be used to supplement rainfall for a minimum of 2-3 weeks.  
**MATERIALS:** Sod selected should be certified. Sod grown in the general area of the project is desirable.  
1. Sod should be machine cut and contain 3/4" (+ or -1/4") of soil, not including shoots or thatch.  
2. Sod should be cut to the desired size within + or -5%. Torn or uneven pads should be rejected.  
3. Sod should be cut and installed within 36 hours of digging.  
4. Avoid planting when subject to frost heave or hot weather, if irrigation is not available.  
5. The sod type should be shown on the plans or installed according to Table 6-6.2. See Figure 6-4.1 for your Resource Area  
**MAINTENANCE:** Re-sod areas where an adequate stand of sod is not obtained. New sod should be mowed sparingly. Grass height should not be cut less than 2"-3" or as specified (See Figure 6-6.2). Apply one ton of agricultural lime as indicated by soil test or every 4-6 years. Fertilize grasses in accordance with soil tests or Table 6-6.3.

## CONSTRUCTION SPECIFICATIONS

THE MANUFACTURER SHALL HAVE EITHER AN APPROVED COLOR MARK YARN IN THE FABRIC OR LABEL THE FABRICATED SILT FENCE WITH BOTH THE MANUFACTURER AND FABRIC NAME EVERY 100'.  
THE TEMPORARY SILT FENCE SHALL BE INSTALLED ACCORDING TO THIS SPECIFICATION, AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER. FOR INSTALLATION OF THE FABRIC, SEE FIGURES 6-20.4, 6-20.5, AND 6-20.6 RESPECTIVELY.  
POST INSTALLATION SHALL START AT THE CENTER OF THE LOW POINT (IF APPLICABLE) WITH REMAINING POSTS SPACED 6 FEET APART FOR TYPE "A" AND "B" SILT FENCES AND 4 FEET APART FOR TYPE "C" SILT FENCE. WHILE TYPE "A" AND "B" SILT FENCES CAN BE USED WITH BOTH WOOD AND STEEL POSTS, ONLY STEEL POSTS SHALL BE USED WITH TYPE "C" SILT FENCE. FOR POST SIZE REQUIREMENTS, SEE TABLE 6-20.3. FASTENERS FOR WOOD POSTS ARE LISTED IN TABLE 6-20.4.  
ALONG STREAM BUFFERS AND OTHER SENSITIVE AREAS, TWO ROWS OF TYPE "C" SILT FENCE OR ONE ROW OF TYPE "C" SILT FENCE BACKED BY HAYBALES SHALL BE USED.

## MAINTENANCE

SEDIMENT SHALL BE REMOVED ONCE IT HAS ACCUMULATED TO ONE-HALF THE ORIGINAL HEIGHT OF THE BARRIER. FILTER FABRIC SHALL BE REPLACED WHENEVER IT HAS DETERIORATED TO SUCH AN EXTENT THAT THE EFFECTIVENESS OF THE FABRIC IS REDUCED (APPROXIMATELY SIX MONTHS). TEMPORARY SEDIMENT BARRIERS SHALL REMAIN IN PLACE UNTIL DISTURBED AREAS HAVE BEEN PERMANENTLY STABILIZED. ALL SEDIMENT ACCUMULATED AT THE BARRIER SHALL BE REMOVED AND PROPERLY DISPOSED OF BEFORE THE BARRIER IS REMOVED.

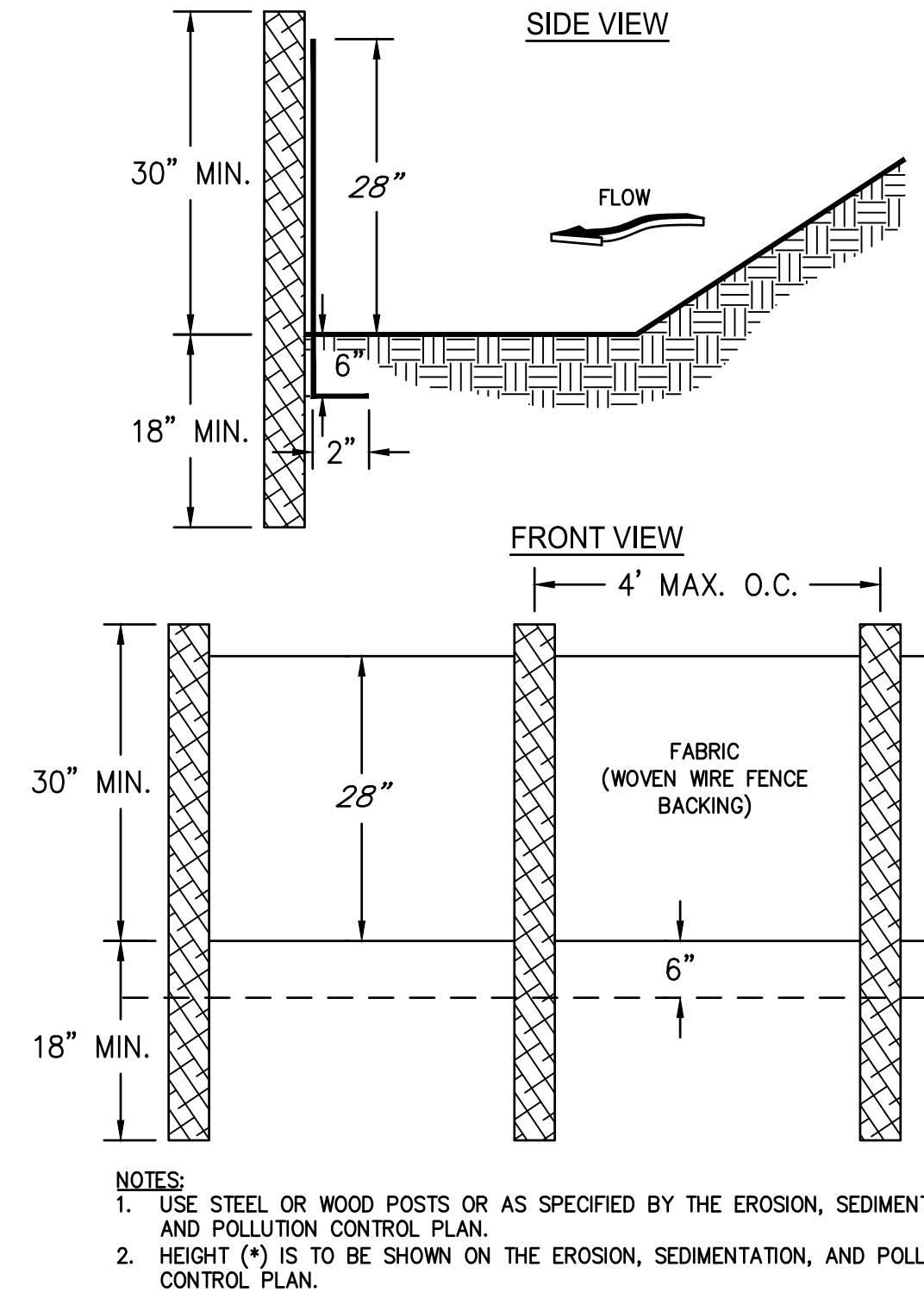
**POST SIZE**  
TABLE 6-13.3

TYPE	MIN. LENGTH	TYPE OF POST	SIZE OF POST
TYPE NS	4'	SOFT WOOD OAK STEEL	3" DIA. OR 2x4 1.5"x1.5" 1.3 LB/FT MIN.
TYPE S	4'	STEEL	1.3LB/FT MIN.

**PRIMARY PERMITTEE**  
OWNER:  
CITY OF TUCKER  
PAKS AND RECREATION DEPT.  
4898 LAVISTA RD, TUCKER, GA 30084  
CONTACT: RIP ROBERTSON, DIRECTOR  
PHONE: (470) 481-0205  
EMAIL: rrobertson@tuckerga.gov

**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205

## SILT FENCE - Sd1-S (TYPE C)



## Sd1-C SILT FENCE - Sd1-S (TYPE C)

SCALE: N.T.S.

**JOHNS HOMESTEAD PARK**  
EROSION CONTROL  
EROSION CONTROL DETAILS 1

SCALE: AS SHOWN  
DESIGN BY: JR/JH 01/23  
DRAWN BY: JH/EC 01/23  
CHECKED BY: JR 01/23  
CD# PEF 000707 EXP. 06/30/2024

JASON RAPPLEAN, PE, EDR  
LEVEL 2 CERTIFICATION # 3891  
EXPIRATION DATE: 12/07/26

06/19/2024

**CITY OF TUCKER AND DEKALB COUNTY**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
CONSULTING ENGINEERS  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
(770) 956-7879

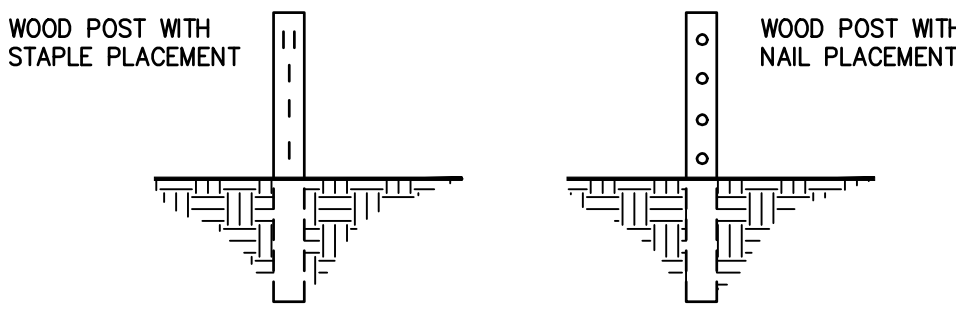
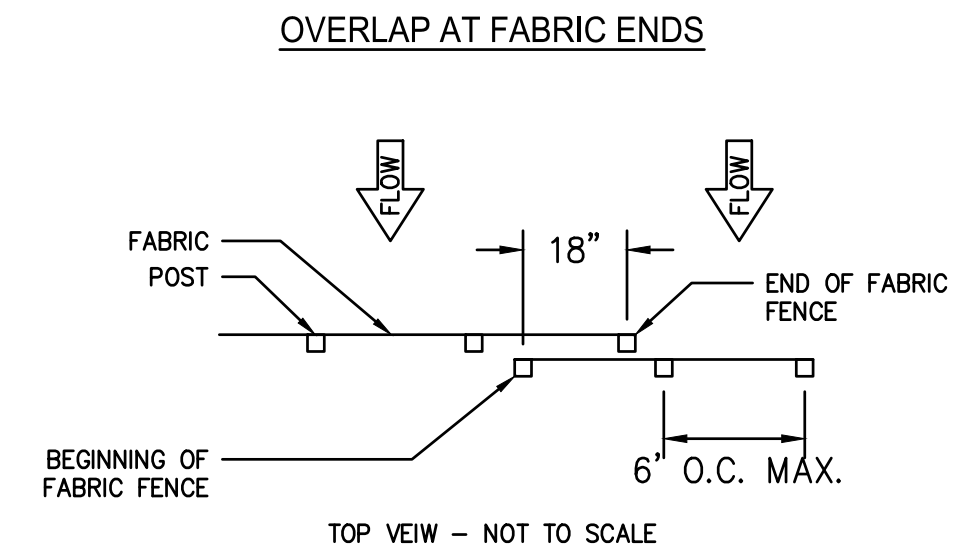
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NO.	REVISIONS	MADE	OK'D.	DATE	DRAWING NO.	REFERENCE DRAWINGS





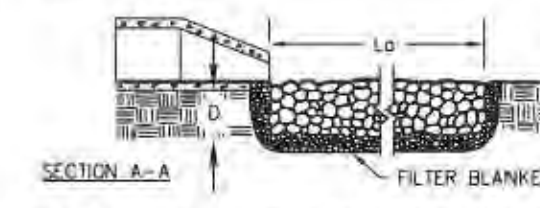
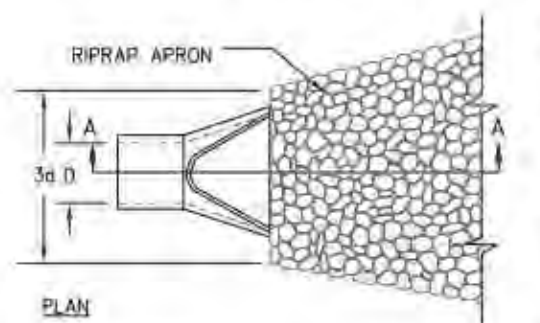
### FASTENERS FOR SILT FENCES



**NOTES:**  
1. THE FABRIC AND WIRE SHOULD BE SECURELY FASTENED TO POSTS AND FABRIC ENDS MUST BE OVERLAPPED A MINIMUM OF 18" OR WRAPPED TOGETHER AROUND A POST TO PROVIDE A CONTINUOUS FABRIC BARRIER AROUND THE INLET.

### St RIPRAP OUTLET PROTECTION

PIPE OUTLET TO FLAT AREA - NO WELL DEFINED CHANNEL



PIPE OUTLET TO WELL DEFINED CHANNEL

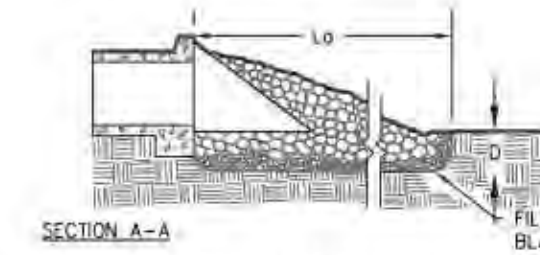
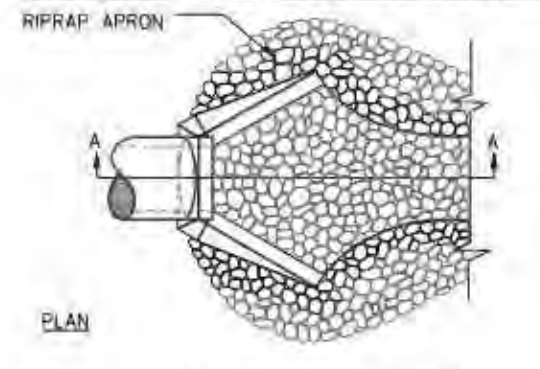


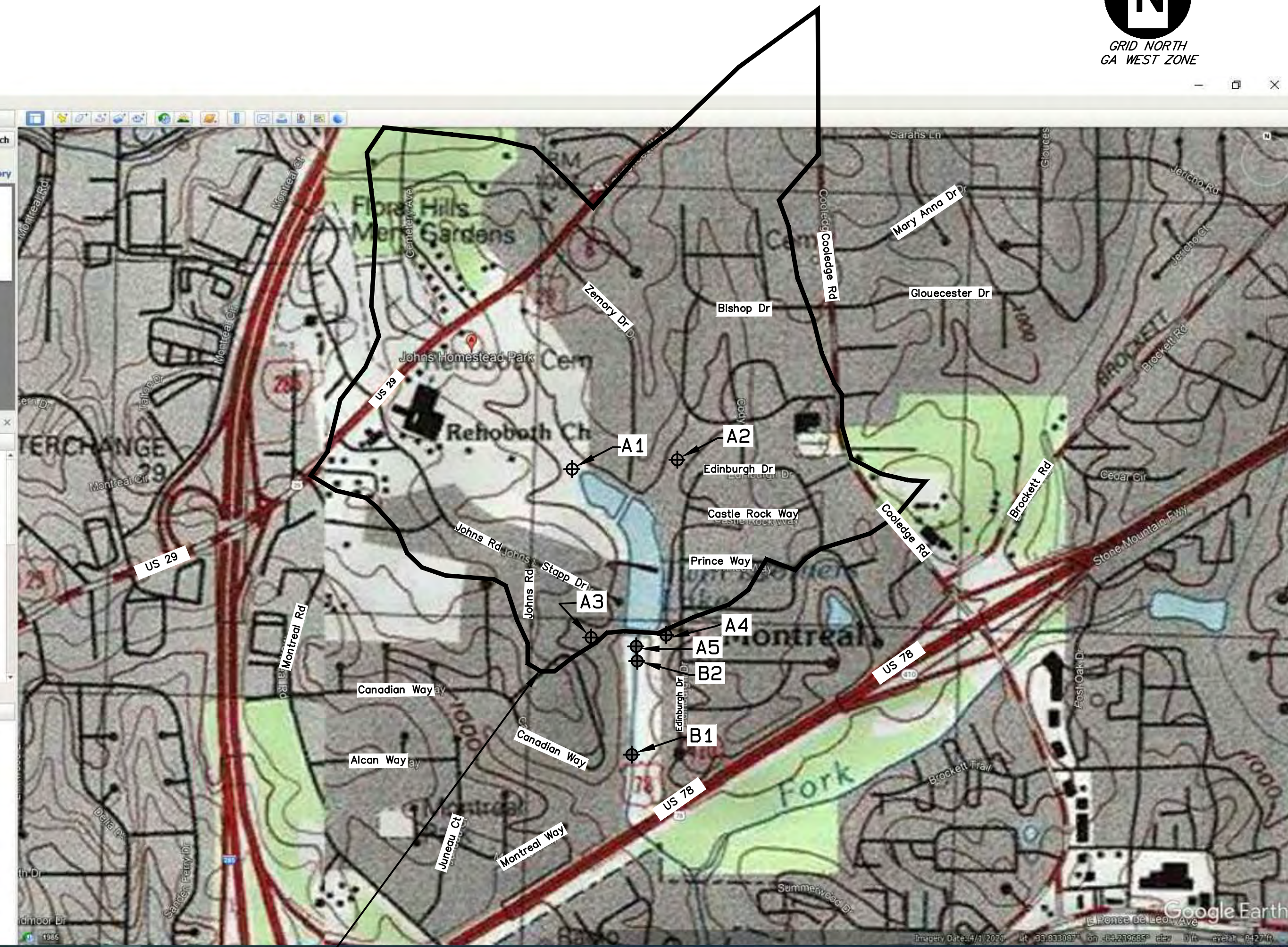
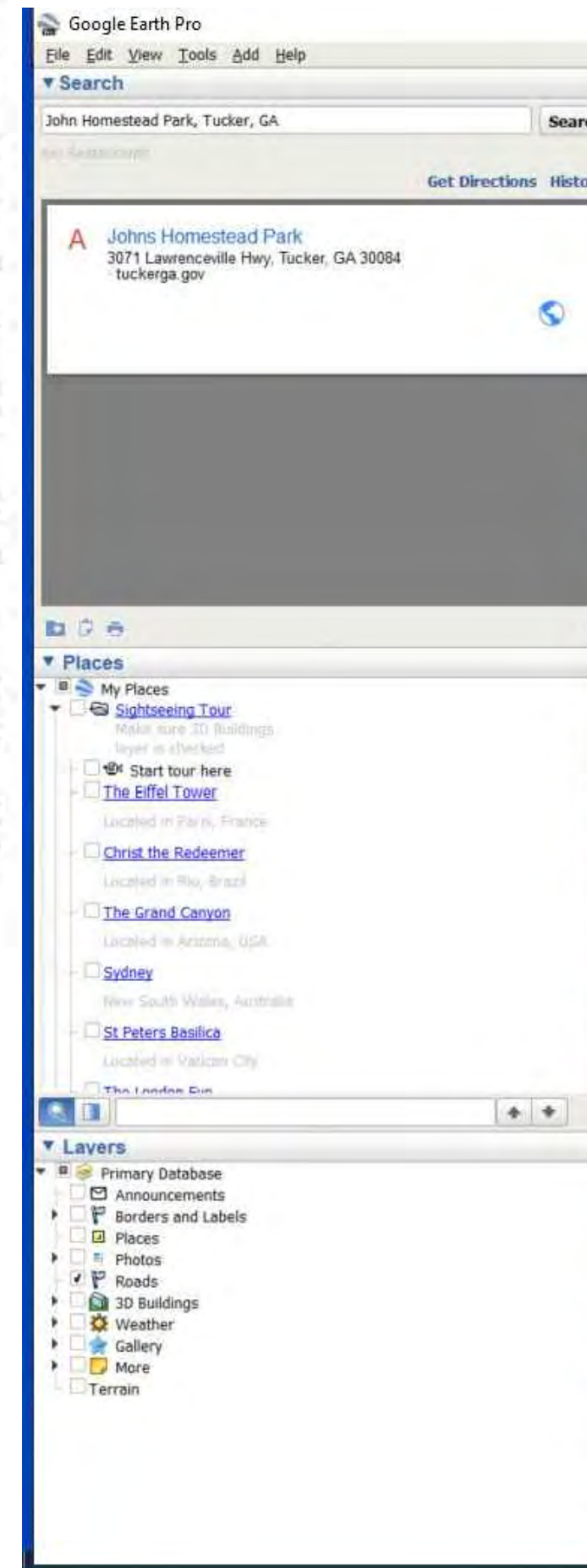
Figure 6-34.3 - Riprap Outlet Protection (Modified From VA SWCC)

#### St RIPRAP OUTLET CALCULATIONS CHART

PIPE NO.	W (FT)	L <sub>a</sub> (FT)	3D <sub>o</sub> (FT)	PIPE DIA (FT)	OUTLET VEL (FPS)	TW<0.5D <sub>o</sub> (YES or NO)	PIPE INV (FT)	d50 (FT)	D (FT)
1B	4.1	5.2	6	2.0	6.4	YES	984.0	0.75	2.0

#### CONSTRUCTION SPECIFICATIONS

- Ensure that the subgrade for the filter and riprap follows the required lines and grades shown in the plan. Compact any fill required in the subgrade to the density of the surrounding undisturbed material. Low areas in the subgrade on undisturbed soil may also be filled by increasing the riprap thickness.
  - The riprap and gravel filter must conform to the specified grading limits shown on the plans.
  - Geotextile must meet design requirements and be properly protected from puncturing or tearing during installation. Repair any damage by removing the riprap and placing another piece of filter fabric over the damaged area. All connecting joints should overlap a minimum of 1 ft. If the damage is extensive, replace the entire filter fabric.
  - Riprap may be placed by equipment, but take care to avoid damaging the filter.
  - The minimum thickness of the riprap should be 1.5 times the maximum stone diameter.
  - Construct the apron on zero grade with no overfall at the end. Make the top of the riprap at the downstream end level with the receiving area or slightly below it.
  - Ensure that the apron is properly aligned with the receiving stream and preferably straight throughout its length. If a curve is needed for site conditions, place it in the upper section of the apron.
  - Immediately after construction, stabilize all disturbed areas with vegetation.
  - Stone quality - Select stone for riprap from field stone or quarry stone. The stone should be hard, angular, and highly weather-resistant. The specific gravity of the individual stones should be at least 2.5.
  - Filter - Install a filter to prevent soil movement through the openings in the riprap. The filter should consist of a graded gravel layer or a synthetic filter cloth. See Appendix C, p. C-1.
- MAINTENANCE**  
Inspect riprap outlet structures after heavy rains to see if any erosion around or below the riprap has taken place or if stones have been dislodged. Immediately make all needed repairs to prevent further damage.



SAMPLING LOCATION (USGS) MAP  
SCALE: 1"=1,000'

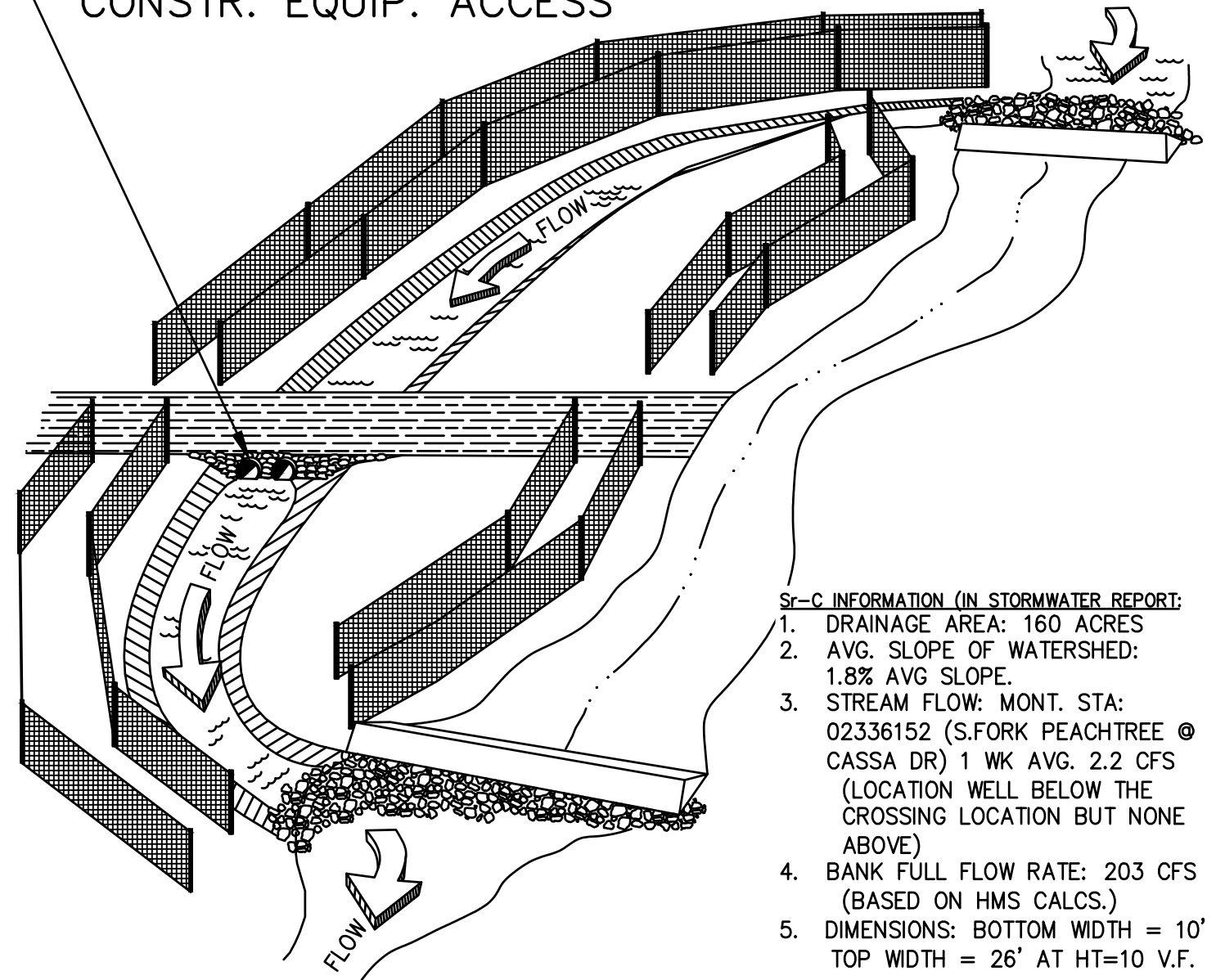
DRAINAGE BASIN: 0.9700 SQ. MI.  
(DISTURBING 3.5 AC.)

**EXISTING:**  
CATCH SAMPLE AND ADD A1 THRU A5 TOGETHER TO GET A COLLECTIVE FLOWING INTO SOUTH FORK PEACHTREE CREEK AND THEN CATCH A SAMPLE IN THE CREEK B1, WHICH WILL BE DOWNSTREAM WERE ALL THE WORK WILL TIE INTO THE STREAM SEGMENT (SEE C-200 NOTES PLAN) FOR SAMPLING LOCATION POINT).

**PROPOSED:**  
CATCH SAMPLE POINTS WILL BE THE DISCHARGE FROM THE LOWER DAM'S LABYRINTH SPILLWAY'S RIPRAP PAD (POINT B2) AND THE SAME POINT B1 WHERE THE WORK CONNECTS TO THE STREAM SEGMENT SO THAT THE EXISTING CONDITIONS CAN BE COMPARED TO THE PROPOSED CONDITIONS.

### Sr-C STREAM DIVERSION CHANNEL TEMPORARY CULVERT CROSSING

FOUR (4) HDPE (24" SOLID WALL PIPES) TO BE PLACED IN EXIST. SIDE CHANNEL FOR TEMP. CONSTR. EQUIP. ACCESS



- Sr-C INFORMATION (IN STORMWATER REPORT):**
- DRAINAGE AREA: 160 ACRES
  - AVG. SLOPE OF WATERSHED: 1.8% AVG SLOPE.
  - STREAM FLOW: MONT. STA: 02336152 (S.FORK PEACHTREE @ CASSA DR) 1 WK AVG. 2.2 CFS (LOCATION WELL BELOW THE CROSSING LOCATION BUT NONE ABOVE)
  - BANK FULL FLOW RATE: 203 CFS (BASED ON HMS CALCS.)
  - DIMENSIONS: BOTTOM WIDTH = 10' TOP WIDTH = 26' AT HT=10' V.F.

- NOTES:**
- THE BOTTOM WIDTH OF THE STREAM DIVERSION SHALL BE A MINIMUM OF SIX FEET OR EQUAL TO THE BOTTOM WIDTH OF THE EXISTING STREAMBED (WHICHEVER IS GREATER).
  - SIDE SLOPES OF THE STREAM DIVERSION CHANNEL SHALL BE NO STEEPER THAN 2:1.
  - THE CHANNEL SHALL BE EXCAVATED, CONSTRUCTING PLUGS AT BOTH ENDS.
  - TWO ROWS OF TYPE S SEDIMENT BARRIERS SHALL BE PLACED ALONG THE SIDES OF THE CHANNEL TO PREVENT UNFILTERED RUNOFF FROM ENTERING THE STREAM.
  - THE CHANNEL SURFACE SHALL BE SMOOTH (TO PREVENT TEARING OF THE LINER) AND LINED WITH THE MATERIAL SPECIFIED IN THE PLANS.
  - THE PLUGS ARE REMOVED WHEN THE LINER INSTALLATION IS COMPLETE (REMOVING THE DOWNSTREAM PLUG FIRST).
  - CMP PIPES ARE NOT ALLOWED BY THE GA SAFE DAMS PROGRAM FOR TEMPORARY OR PERMANENT USE AS A DRAINAGE PIPE OR STRUCTURE ON A DAM.

### Cr TEMP. CONSTRUCTION ROAD STABILIZATION

**Grade and Alignment**  
The gradient and vertical and horizontal alignment shall be adapted to the intensity of use, mode of travel, and level of development. Grades for temporary roads should not exceed 10 percent except for very short lengths (200 feet or less), but maximum grades of 20 percent or more may be used if necessary for special uses. Frequent grade changes generally cause fewer erosion problems than long continuous gradients. Curves and switchbacks must be of sufficient radius for trucks and other large vehicles to negotiate easily. On temporary roads, the radius should be no less than 35 feet for standard vehicles and 50 feet for tractor-trailers. Grades for temporary parking areas should be sufficient to provide drainage but should not exceed 4 percent.

**Width**  
Temporary roadbeds shall be at least 14 feet wide for one-way traffic and 20 feet wide for two way traffic. The width for two-way traffic shall be increased approximately 4 feet for trailer traffic. A minimum shoulder width shall be 2 feet on each side. Where turnouts are used, road width shall be increased to a minimum of 20 feet for a distance of 30 feet.

**Side Slopes**  
All cuts and fills shall have side slopes designed to be stable for the particular site conditions and soil materials involved. All cut and fills shall be 2:1 or flatter to the extent possible. When maintenance by machine mowing is planned, side slopes shall be no steeper than 3:1.

**Drainage Structures**  
See plan for temporary drainage structures. A design will be denoted on the plan or in the plan notes. Drainage Structures will be designed for the 25-yr 24-hr storm event or greater. Water breaks or bars may be added to control surface runoff.

**Stabilization**  
Geotextile should be applied to the roadbed for additional stability. Geotextile selection shall be based on AASHTO M288-06 specification:  
1. For subgrades with a CBR greater than or equal to 3 or shear strength greater than 90 kPa, geotextile must meet requirements of section AASHTO M288-06 Section 7.3. Separation Requirements.  
2. For subgrades with a CBR between 1 and 3 or shear strength between 30 and 90 kPa, geotextile must meet requirements of section AASHTO M288-06 Section 8. Geotextile Property Requirements.

A 6-inch course of coarse aggregate shall be applied immediately after grading or the completion of utility installation within the right-of-way. In areas experiencing heavy duty traffic situations, stone should be placed at an 8 to 10 inch depth to avoid excessive dissipation or maintenance needs.  
All roadside ditches, cuts, fills and disturbed areas adjacent to parking areas and/or roads shall be stabilized with appropriate temporary and/or permanent vegetation according to D-2 specifications. Permanent Parking Areas shall be installed in accordance with the local authority's standards or GDOT's if the local authority does not have a standard for temporary parking.

NO.	REVISIONS	MADE	OK'D.	DATE	DRAWING NO.	REFERENCE DRAWINGS



SCALE: AS SHOWN	DATE
DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3091
EXPIRATION DATE:	12/07/26

**JOHNS HOMESTEAD PARK**  
**EROSION CONTROL**  
**EROSION CONTROL DETAILS 2**

**CITY OF TUCKER AND DEKALB COUNTY**

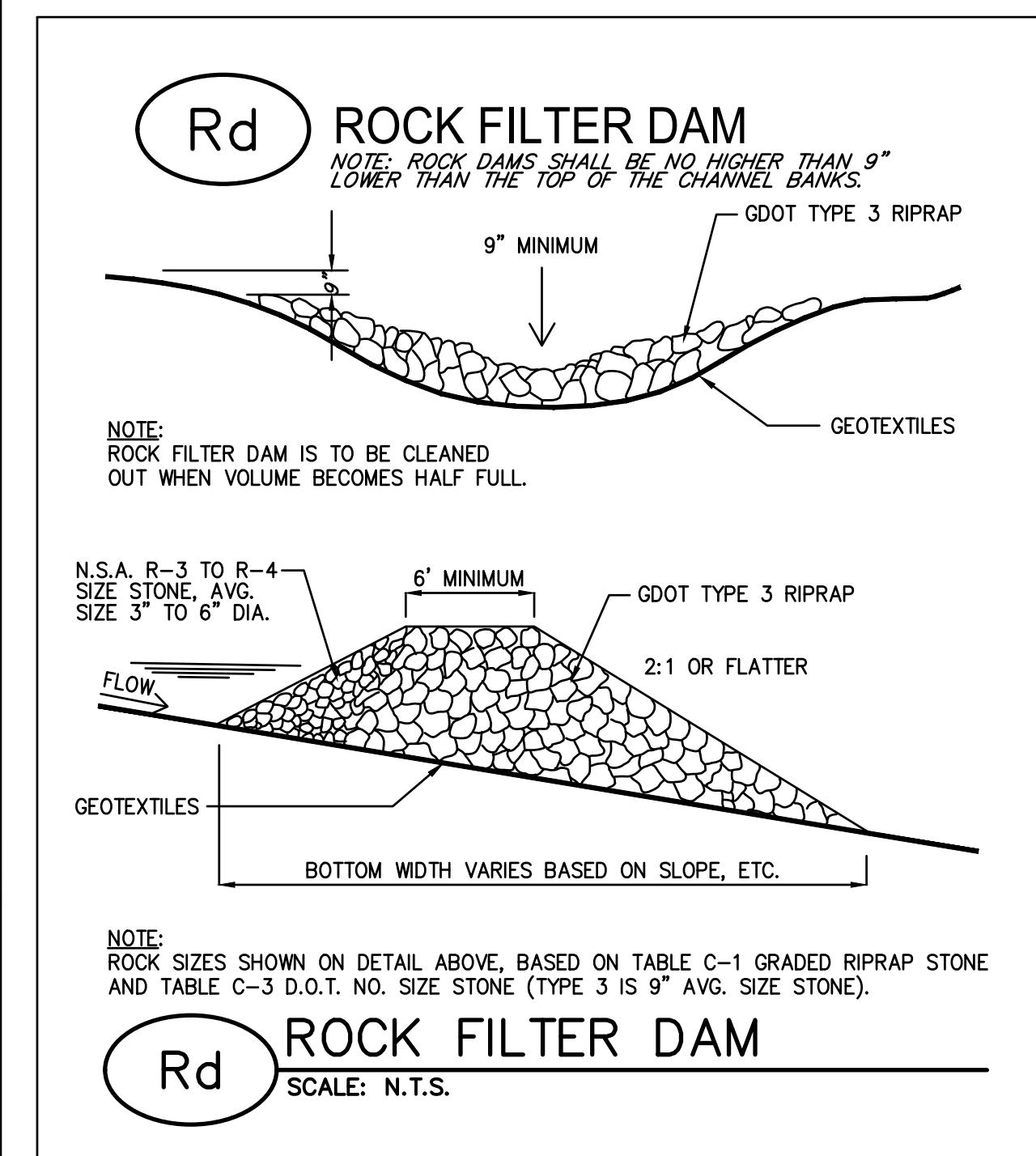
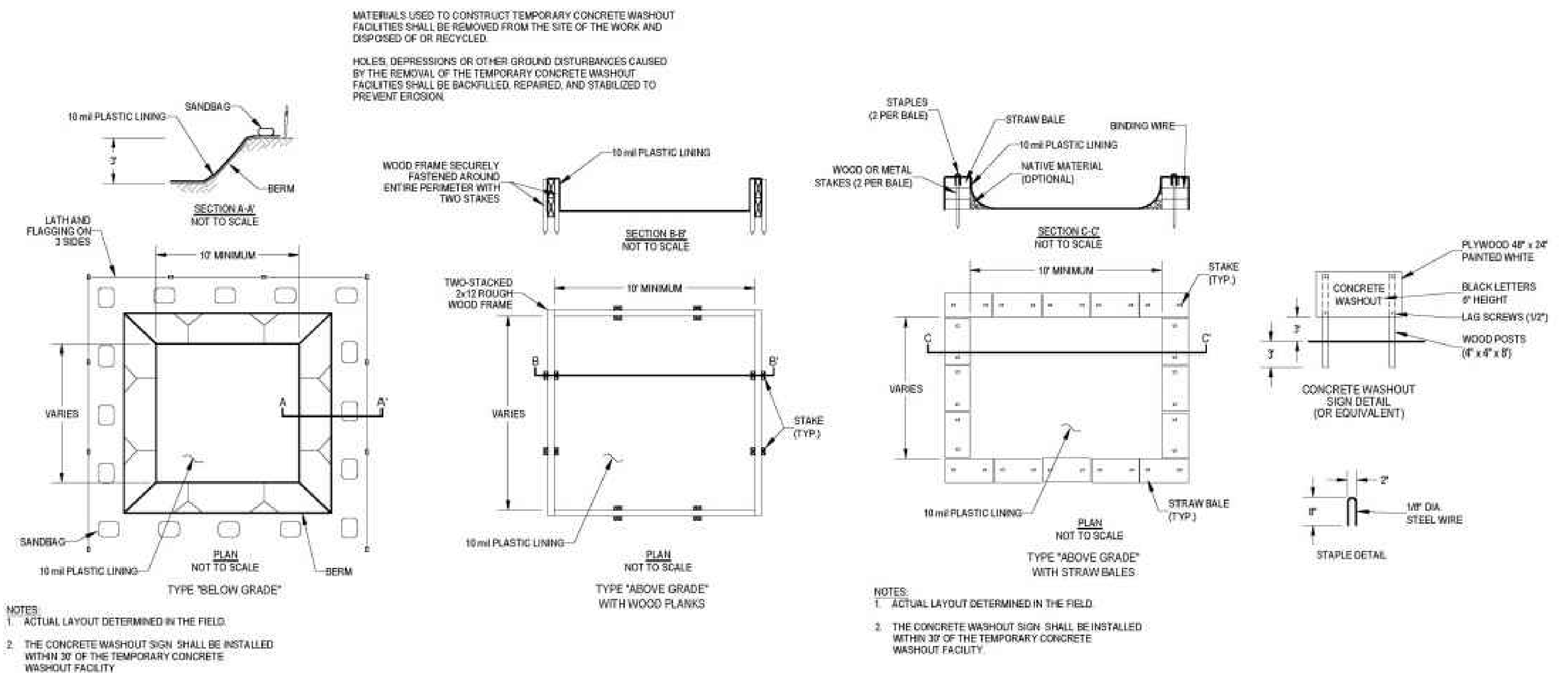
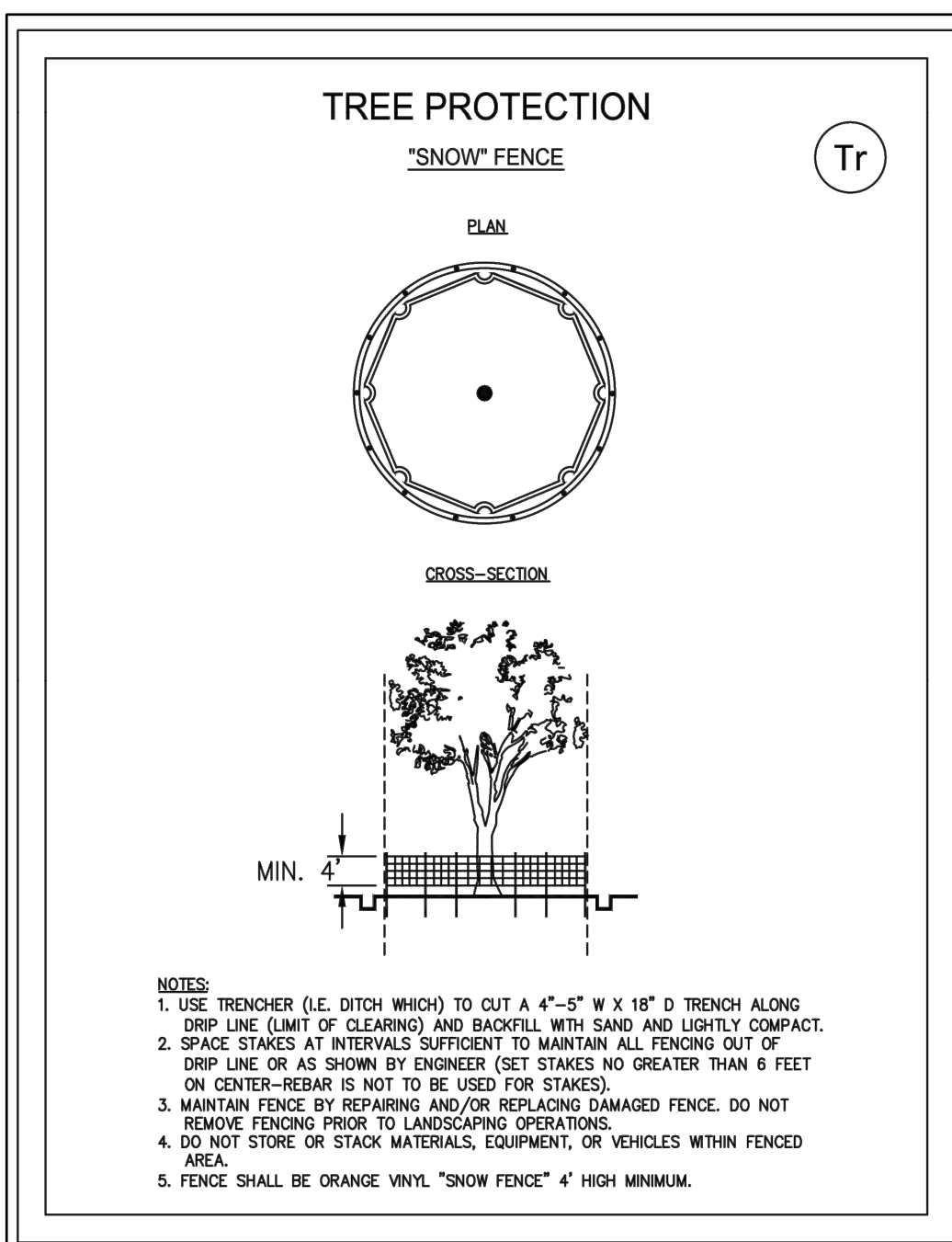
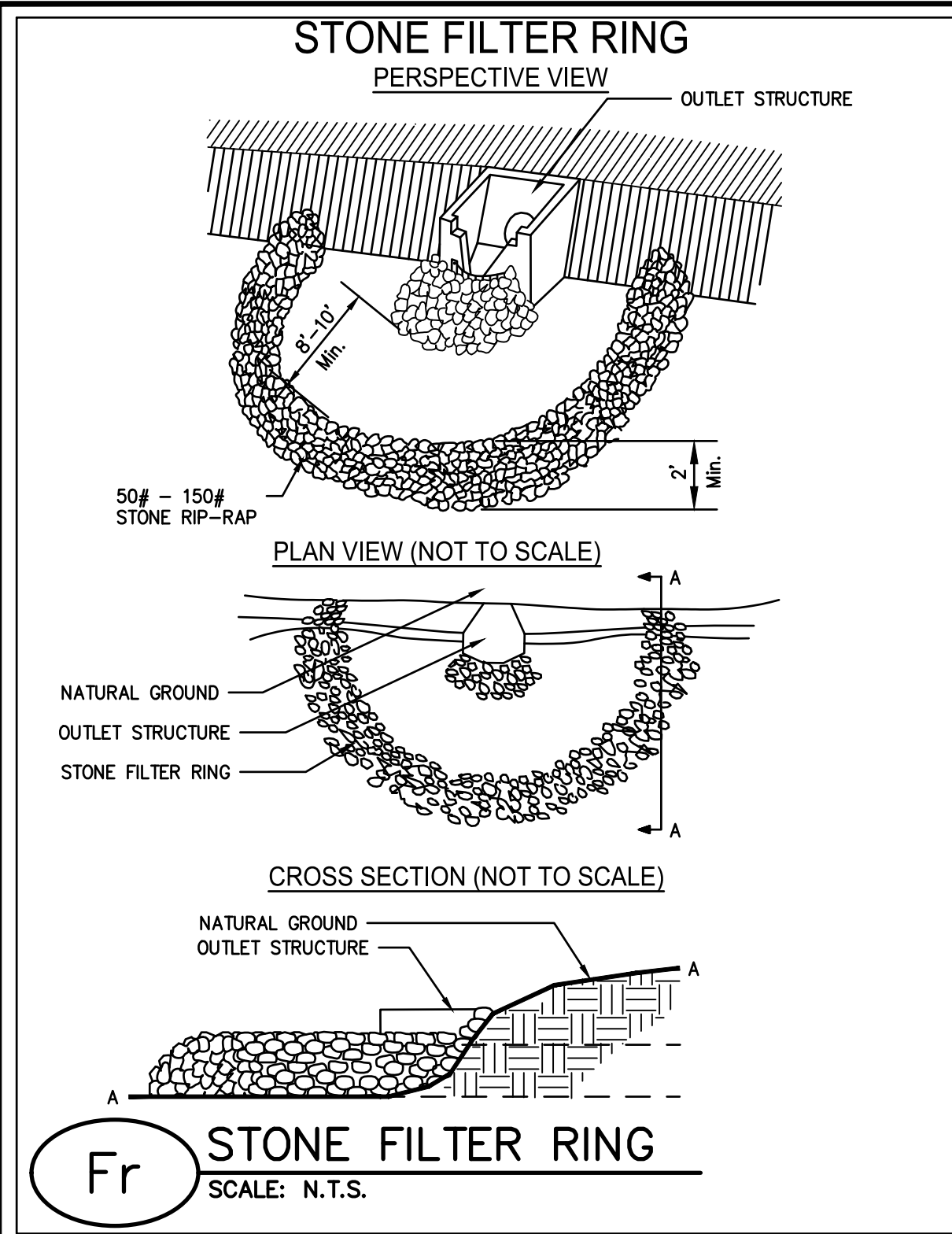
**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
CONSULTING ENGINEERS  
P.O. BOX 6462  
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(770) 956 - 7879

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RIP ROBERTSON  
(470) 481-0205

4201600 **42016-C-207** **0**





**Cw CONCRETE WASHOUT DETAIL**  
SCALE: N.T.S.

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OWNER:  
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4898 LAVISTA RD, TUCKER, GA 30084  
CONTACT: RIP ROBERTSON, DIRECTOR  
PHONE: (470) 481-0205  
EMAIL: rrobertson@tuckerga.gov

**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205

NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

**GEORGIA REGISTERED PROFESSIONAL ENGINEER**  
JASON F. RAPPLEAN  
No. 30441  
EXPIRES 12/31/26

SCALE: AS SHOWN DATE: 01/23  
DESIGN BY: JR/JH 01/23  
DRAWN BY: JH/EC 01/23  
CHECKED BY: JR 01/23  
CD# PEF 000707 EXP. 06/30/2024

JASON RAPPLEAN, PE, EDR  
LEVEL 2 CERTIFICATION # 3091  
EXPIRATION DATE: 12/07/26

**JOHNS HOMESTEAD PARK**  
**EROSION CONTROL**  
**EROSION CONTROL DETAILS 3**

**CITY OF TUCKER AND DEKALB COUNTY**

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(770) 956 - 7879

4201600 **42016-C-208** **0**







**PEDESTRIAN BRIDGE AND FENCING NOTES**

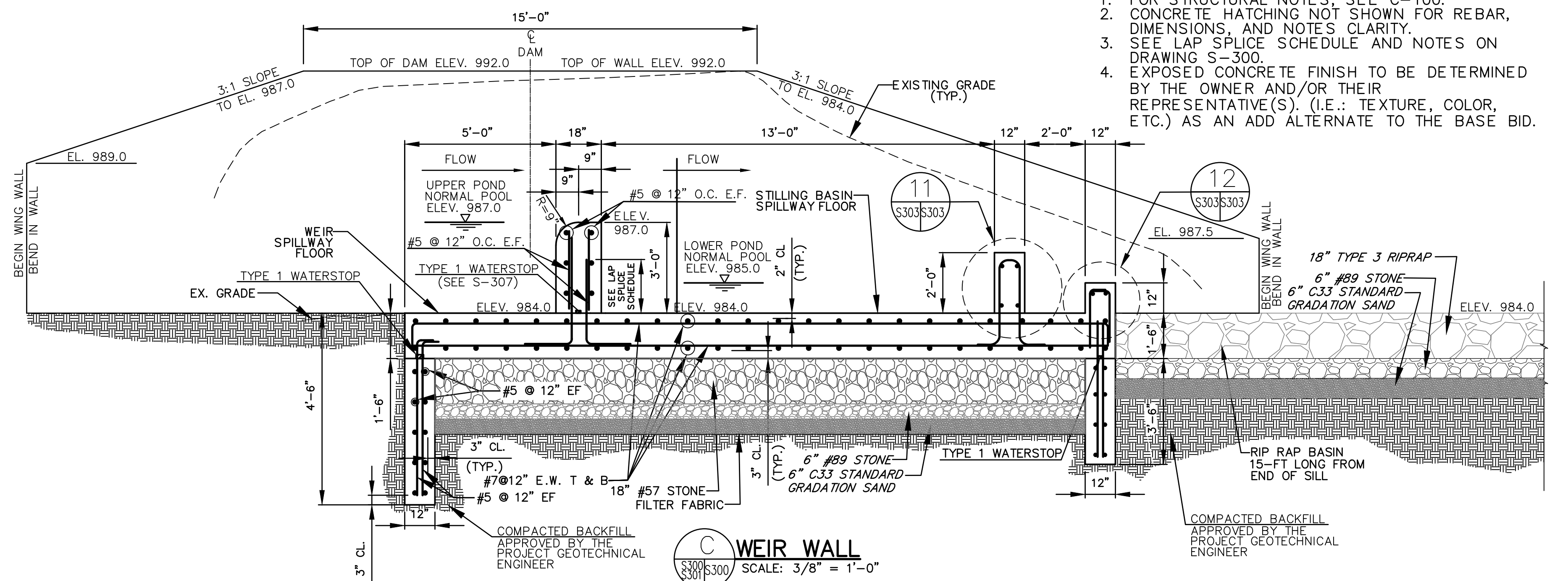
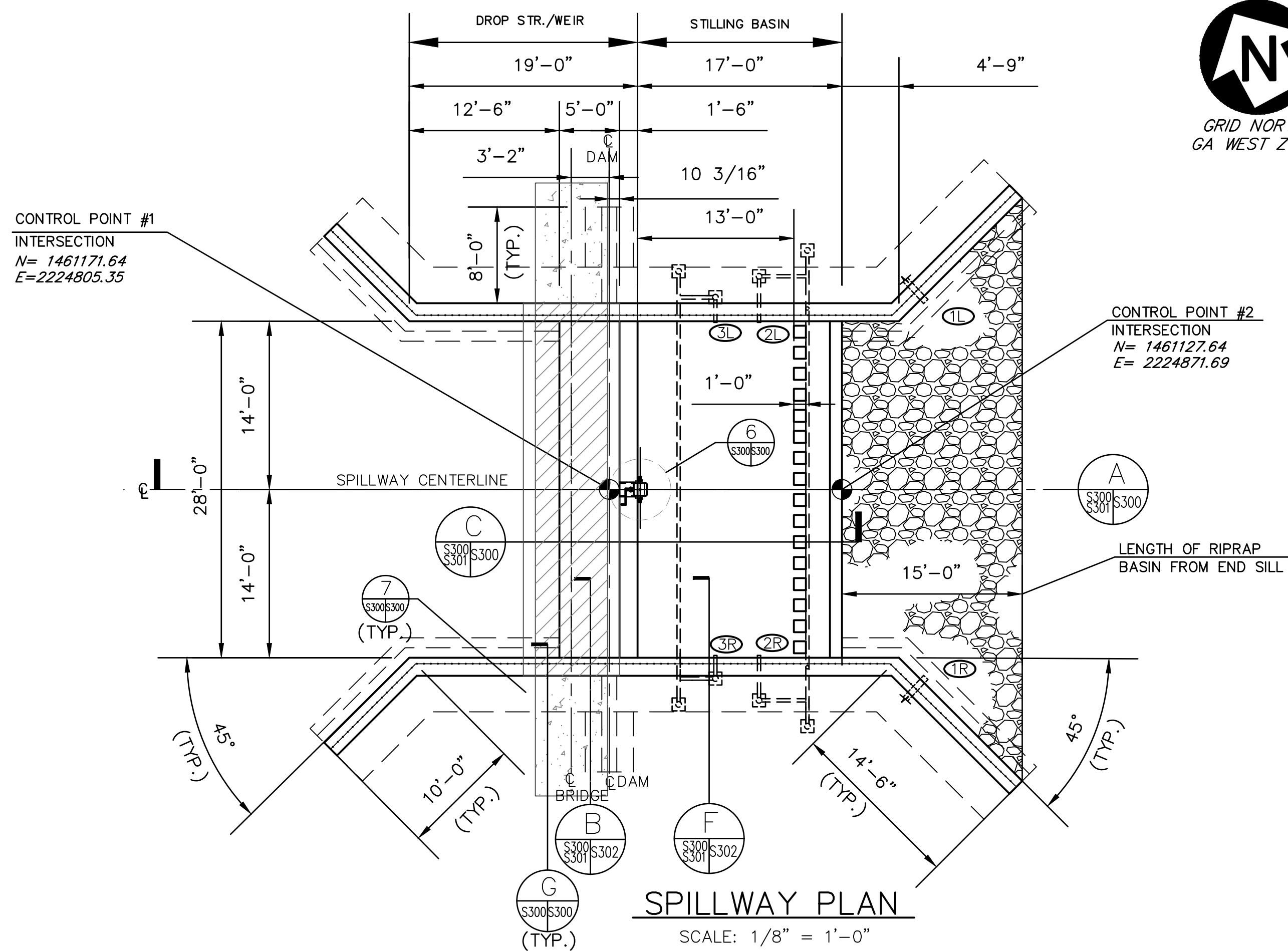
- FOR PEDESTRIAN BRIDGE(S) DETAILS, SEE PLANS BY ROOT DESIGN GROUP.
- FOR FENCING ALONG SIDEWALLS AND ALONG THE WING WALLS OF SPILLWAYS, SEE PLANS BY ROOT DESIGN GROUP.

**NOTES:**

- FOR GENERAL NOTES AND LEGEND, SEE C-100.
- FOR GEOTECHNICAL NOTES, SEE DRAWINGS: C-101A, C-101B, AND C-101C.
- TOPOGRAPHIC SURVEY BY: GASKINS-LECRAW, INC. 1266 POWDER SPRINGS RD., MARIETTA, GA, 30064 PH: 770-424-7168, DATED: OCT. 02, 2023

**STRUCTURAL NOTES**

- FOR STRUCTURAL NOTES, SEE C-100.
- CONCRETE HATCHING NOT SHOWN FOR REBAR, DIMENSIONS, AND NOTES CLARITY.
- SEE LAP SPLICE SCHEDULE AND NOTES ON DRAWING S-300.
- EXPOSED CONCRETE FINISH TO BE DETERMINED BY THE OWNER AND/OR THEIR REPRESENTATIVE(S). (I.E.: TEXTURE, COLOR, ETC.) AS AN ADD ALTERNATE TO THE BASE BID.



**LAP SPLICE SCHEDULE**

BAR SIZE	TENSION BARS (INCHES)				COMPRESSION BARS (INCHES)
	A	B	A	B	ALL BARS
#3	18	23	14	18	12
#4	24	31	18	24	15
#5	30	38	23	30	19
#6	35	46	27	35	23
#7	51	67	40	51	27
#8	59	76	45	59	30
#9	66	86	51	66	34
#10	73	95	56	73	38
#11	80	104	62	80	42

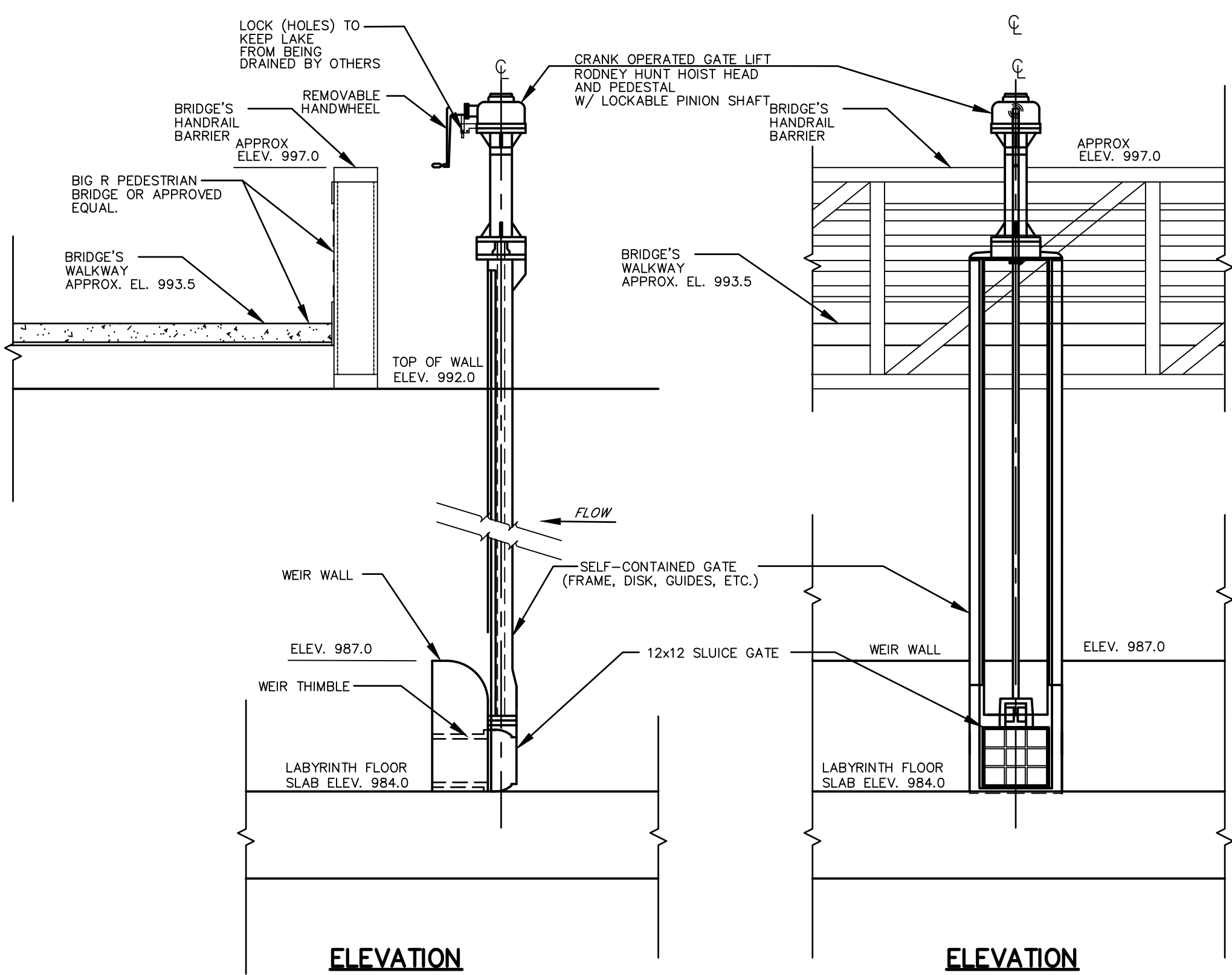
- LAP SPLICES SHALL BE IN ACCORDANCE WITH THE TABLE ABOVE, UNLESS OTHERWISE NOTED. WHERE CLASSES ARE NOT CALLED OUT ON DRAWINGS, USE CLASS "B" SPLICES.
- COMPRESSION DOWEL EMBEDMENT = 22 BAR DIAMETERS.
- LAP WELDED WIRE FABRIC ONE SLIPE OF CROSS WIRES PLUS 2 INCHES.
- WHERE THE CLEAR SPACING BETWEEN BARS BEING SPLICED IS LESS THAN TWO BAR DIAMETERS, INCREASE THE LAP LENGTH BY 50%.
- WHERE THE BAR COVER IS LESS THAN OR EQUAL TO THE BAR DIAMETER, INCREASE THE LAP LENGTH BY 50%.
- TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
- LAP SPLICE LENGTHS ARE PROVIDED FOR NORMAL WEIGHT CONCRETE. WHERE LIGHTWEIGHT CONCRETE IS USED, INCREASE LAP SPLICE LENGTHS BY 30%.
- SPLICES OF HORIZONTAL REINFORCEMENT IN WALLS SHALL BE STAGGERED.
- SPLICES OF HORIZONTAL REINFORCEMENT IN WALLS CONTAINING TWO MATS OF REINFORCEMENT SHALL NOT OCCUR IN THE SAME LOCATIONS.

**ADD ALTERNATE FOR UTILIZING ELASTOMERIC FORMLINER(S) FOR TEXTURING CONCRETE TO PROVIDE STANDARD PATTERNS:**

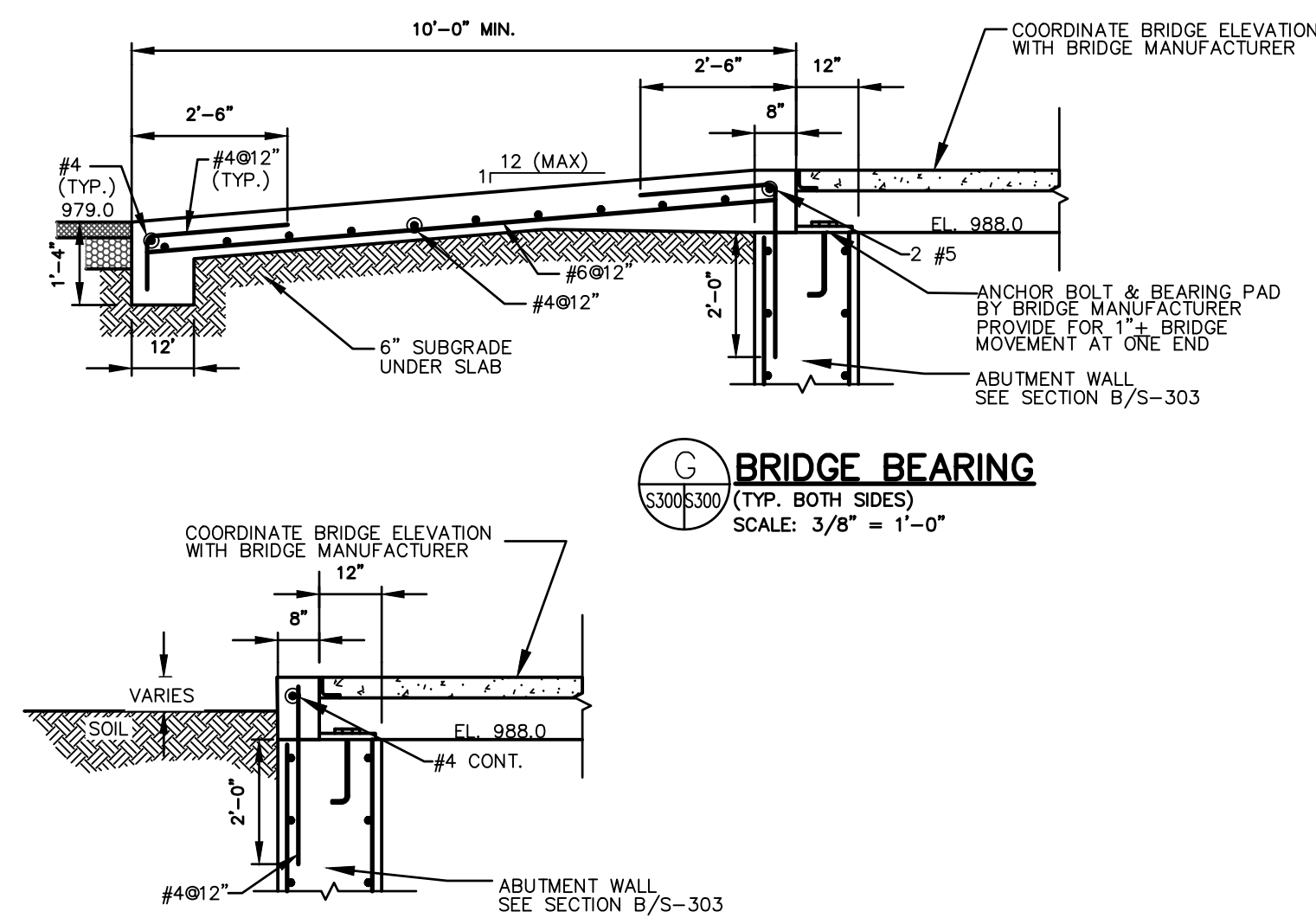
- THIS ADD ALTERNATE WILL BE IN THE BID DOCUMENTS AND IS TO BE USED TO DETERMINE THE COSTS ASSOCIATED WITH PROVIDING AN ARCHITECTURAL TEXTURING PATTERN IN THE EXPOSED SPILLWAY'S CONCRETE WALLS.
- REFER TO SPECIFICATION SECTION 03-11-16 FOR ADDITIONAL INFORMATION.
- IF THERE IS SUFFICIENT BUDGET, THE PATTERNS WILL BE ADDED TO THE SPILLWAY'S CONSTRUCTION.

**SPILLWAY BRIDGE NOTES:**

- FOR PEDESTRIAN BRIDGE(S) DETAILS, SEE PLANS BY OTHERS.
- REFER TO SPILLWAY BRIDGE NOTES LOCATED ON DWG. C-100 FOR ADDITIONAL INFORMATION.



**6 DRAWDOWN GATE ACCESS DETAIL**  
SCALE: 3/8" = 1'-0"



**7 SECTION AT CURB**  
SCALE: 3/8" = 1'-0"

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CONTACT: RIP ROBERTSON, DIRECTOR  
PHONE: (470) 481-0205  
EMAIL: rrobertson@tuckerga.gov

**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205

NO.	REVISIONS	MADE	OKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

**GEORGIA REGISTERED PROFESSIONAL ENGINEER**  
No. 30441  
JASON F. RAPPLEAN

06/19/2024

JASON RAPPLEAN, PE, EDR  
LEVEL 2 CERTIFICATION # 3091  
EXPIRATION DATE: 12/07/26

SCALE: AS SHOWN DATE: 01/23

DESIGN BY: JR/JH 01/23

DRAWN BY: JH/EC 01/23

CHECKED BY: JR 01/23

CD# PEF 000707 EXP. 06/30/2024

**JOHNS HOMESTEAD PARK**  
**STRUCTURAL**  
**UPPER SPILLWAY**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
CONSULTING ENGINEERS  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
(770) 956-7879

4201600 **42016-S-300** 0

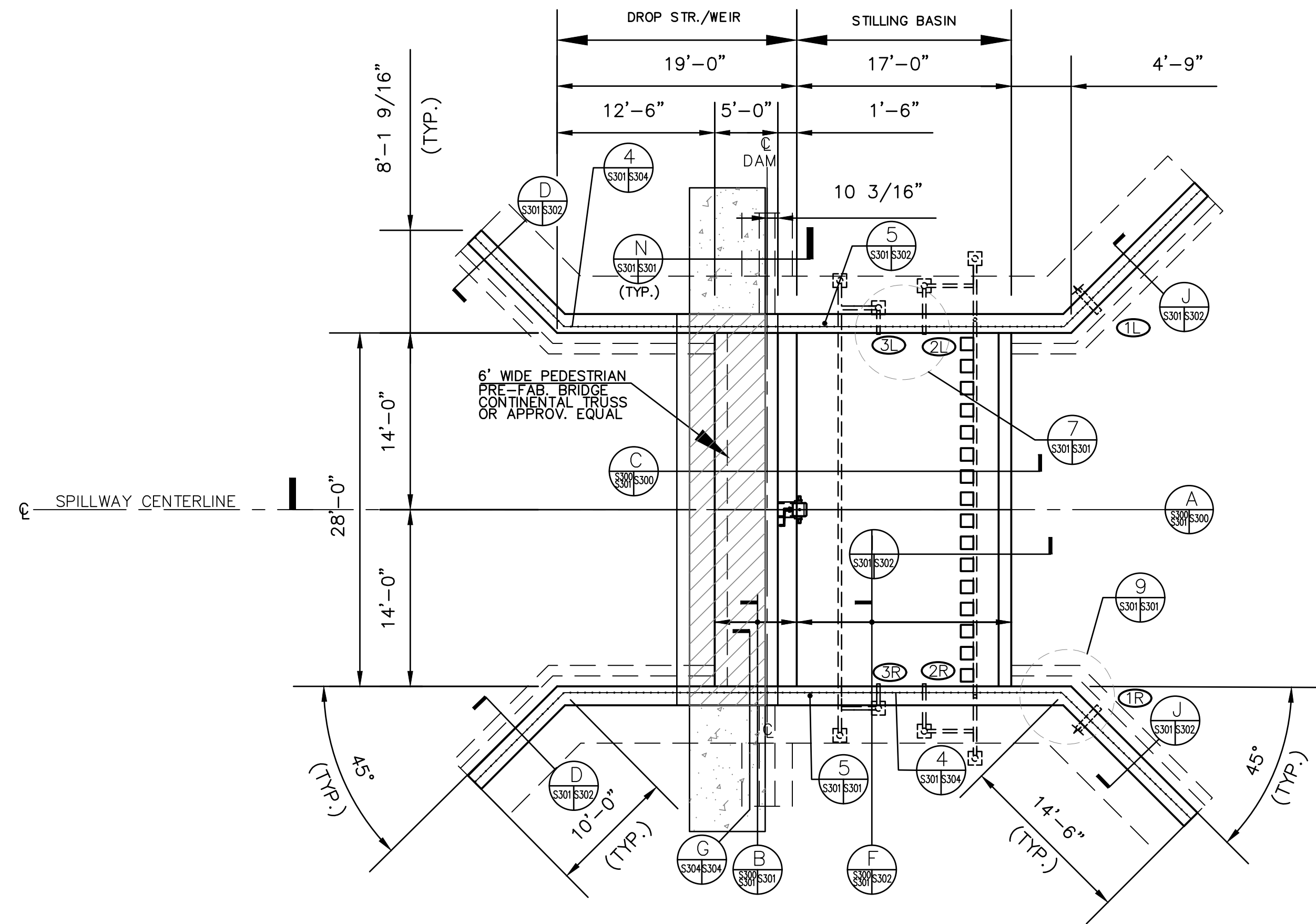


**NOTES:**

1. FOR GENERAL NOTES AND LEGEND, SEE C-100.
2. FOR GEOTECHNICAL NOTES, SEE DRAWINGS: C-101A, C-101B, AND C-101C.
3. TOPOGRAPHIC SURVEY BY: GASKINS-LECRAW, INC. 1266 POWDER SPRINGS RD., MARIETTA, GA 30064 PH: 770-424-7168, DATED: OCT. 02, 2023

**STRUCTURAL NOTES**

1. FOR STRUCTURAL NOTES, SEE C-100.
2. CONCRETE HATCHING NOT SHOWN FOR REBAR, DIMENSIONS, AND NOTES CLARITY.
3. SEE LAP SPLICE SCHEDULE AND NOTES ON DRAWING S-300.



**SPILLWAY WEIR WALL PLAN**

SCALE: 1/8" = 1'-0"

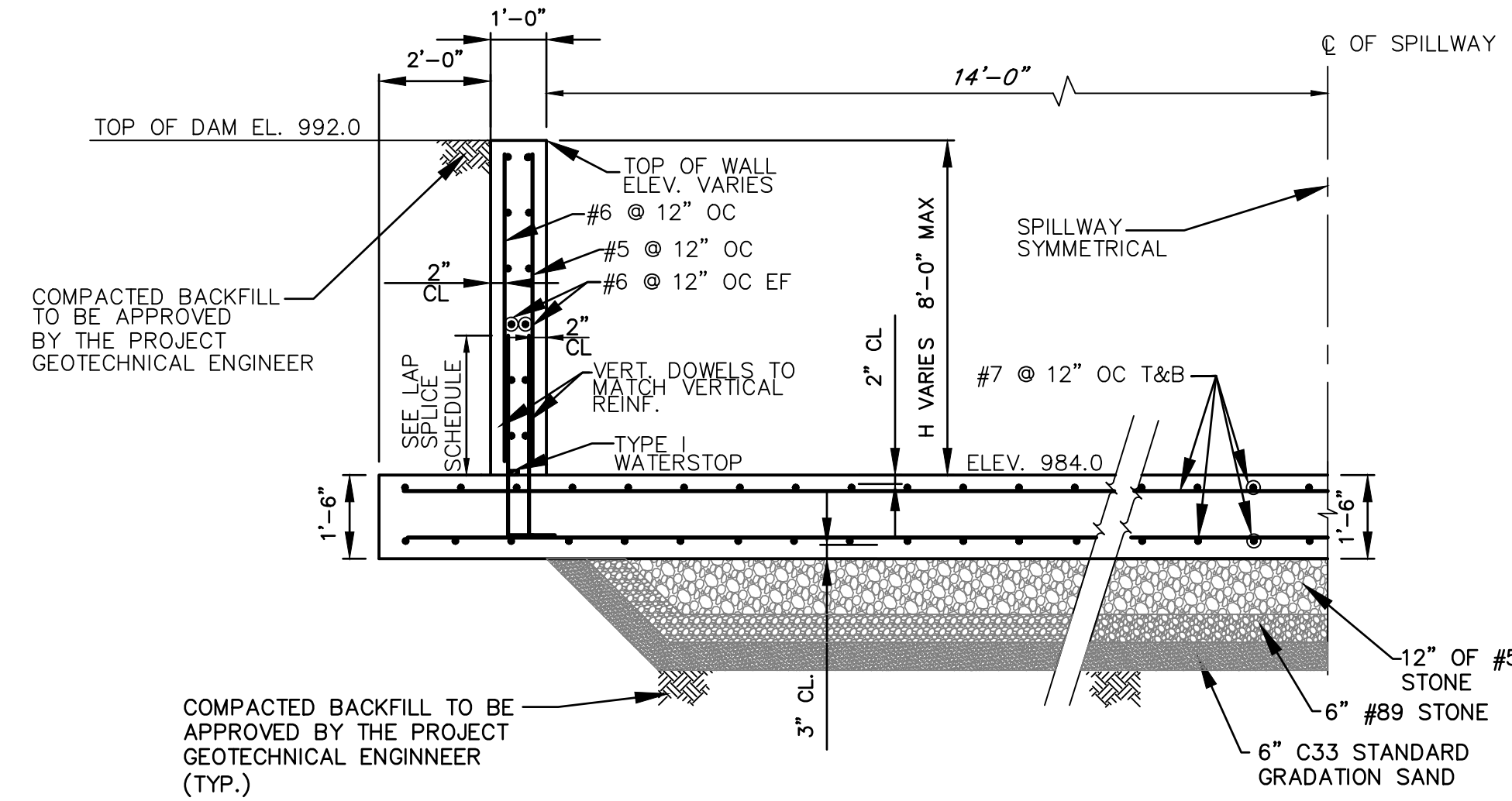
NOTE - DOWEL HORIZ. BARS INTO ABUTMENT WALL WITH 2'-0" LAP. INSTALL TYPE 1 WATER STOP AT CENTER OF WALL ALONG ENTIRE JOINT WITH ABUTMENT WALL.

TO BE APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER

NOTE: BACKFILL SHALL BE PLACED ON BOTH SIDES OF WALL EQUALLY SO THAT WALL WILL NOT BE LOADED ON ONE SIDE GREATER THAN THE OTHER SIDE.

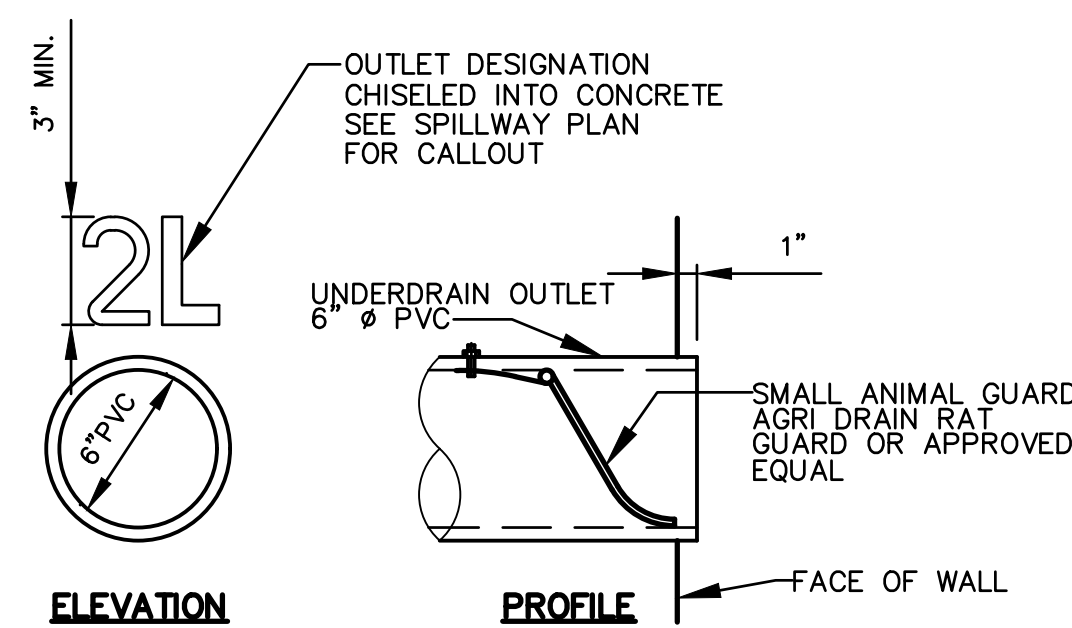
**SECTION THRU ANTI-SEEP WALL**

SCALE: 3/8" = 1'-0"



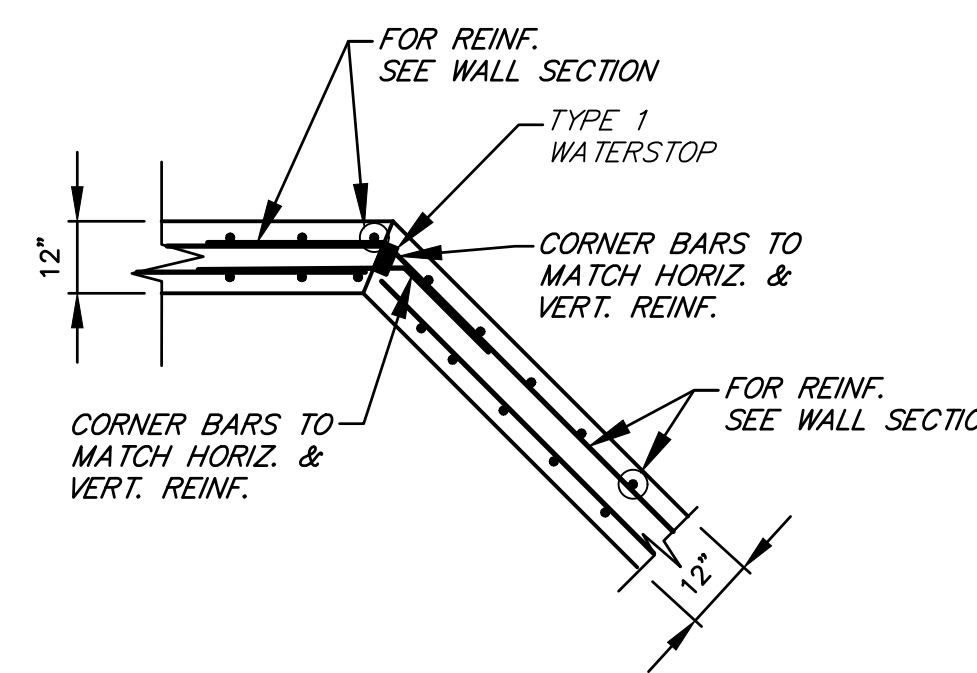
**SECTION THROUGH ABUTMENT WALL & SLAB**

SCALE: 3/8" = 1'-0"



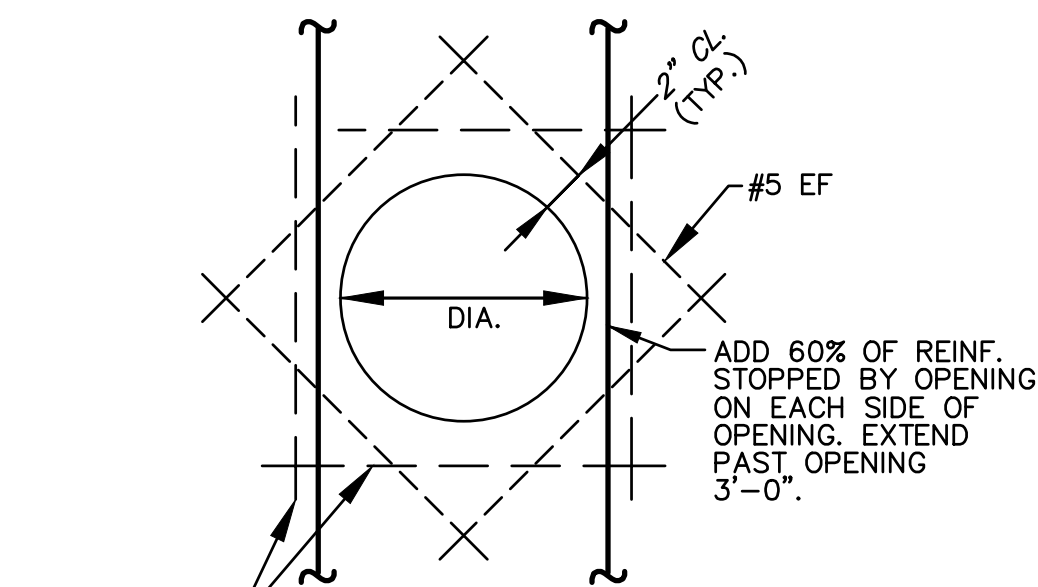
**DETAIL AT UNDERDRAIN OUTLET**

N.T.S. (TYPICAL 8 PLACES)



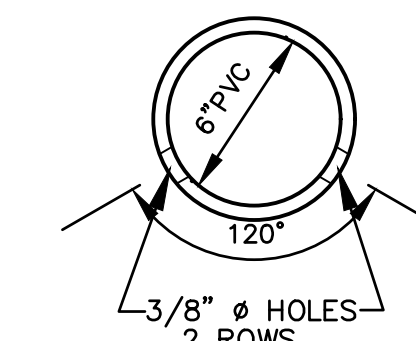
**DETAIL AT WALL JOINT**

SCALE: 3/8" = 1'-0"



**REINFORCEMENT AROUND ORIFICE OR PIPE OPENING**

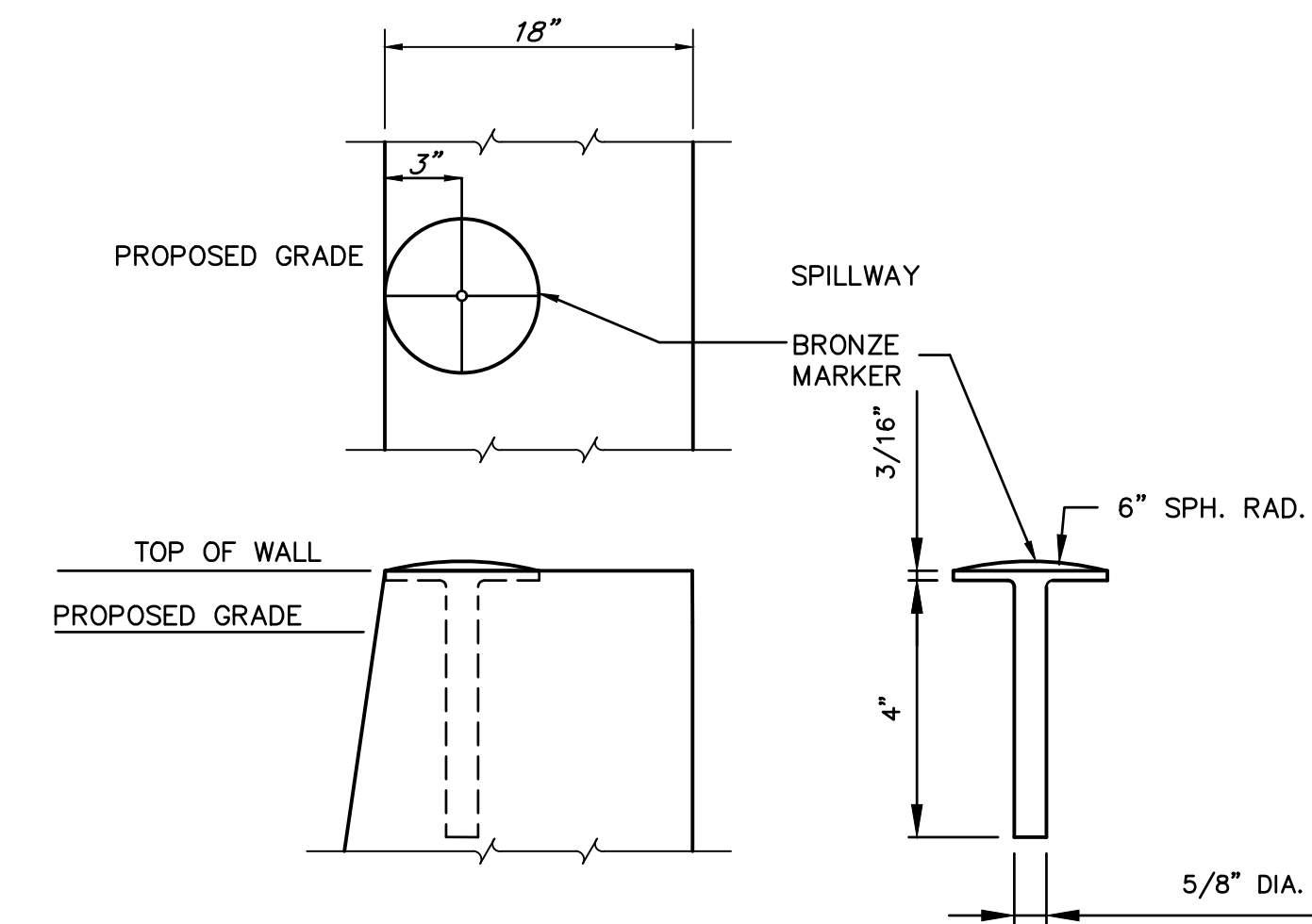
N.T.S. - TYPICAL UNLESS SHOWN OTHERWISE



NOTE: LAY PIPE WITH PERFORATIONS TURNED DOWN.

**DETAIL AT UNDERDRAIN PIPE**

N.T.S.



**SETTLEMENT PLATE DETAIL**

SCALE: N.T.S.

**PRIMARY PERMITTEE**  
**OWNER:**  
 CITY OF TUCKER  
 PAKS AND RECREATION DEPT.  
 4898 LAVISTA RD, TUCKER, GA 30084  
 CONTACT: RIP ROBERTSON, DIRECTOR  
 PHONE: (470) 481-0205  
 EMAIL: rrobertson@tuckerga.gov

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 (470) 481-0205

NO.	REVISIONS	MADE	OKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

**GEORGIA REGISTERED PROFESSIONAL ENGINEER**  
 No. 20441  
 JASON F. RAPPLEAN  
 06/19/2024

SCALE: AS SHOWN	DATE
DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024

JASON RAPPLEAN, PE, EDR  
 LEVEL 2 CERTIFICATION # 3031  
 EXPIRATION DATE: 12/07/26

**JOHNS HOMESTEAD PARK STRUCTURAL UPPER SPILLWAY PLAN, SECTIONS AND DETAILS**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
 CONSULTING ENGINEERS  
 P.O. BOX 6462  
 MARIETTA, GEORGIA 30065  
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4201600 **42016- S-301** **0**

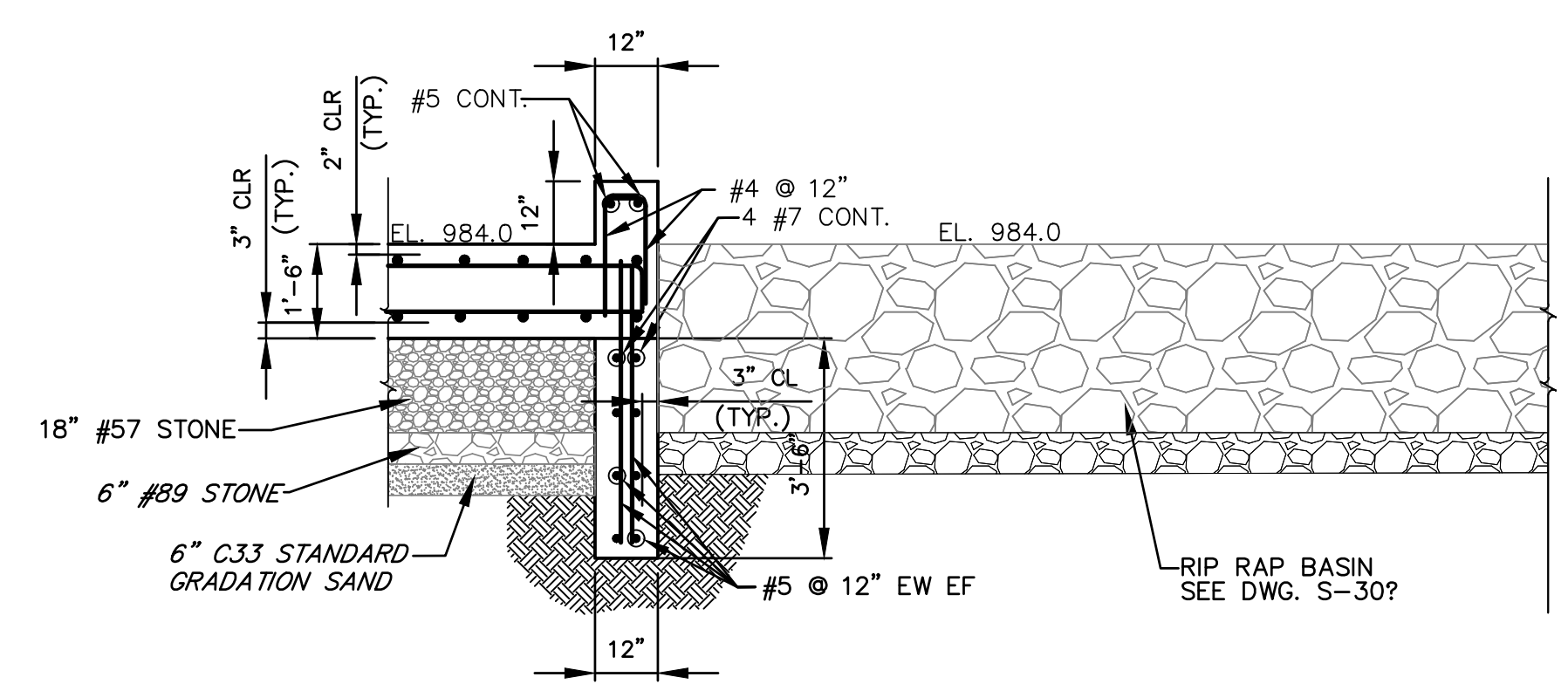
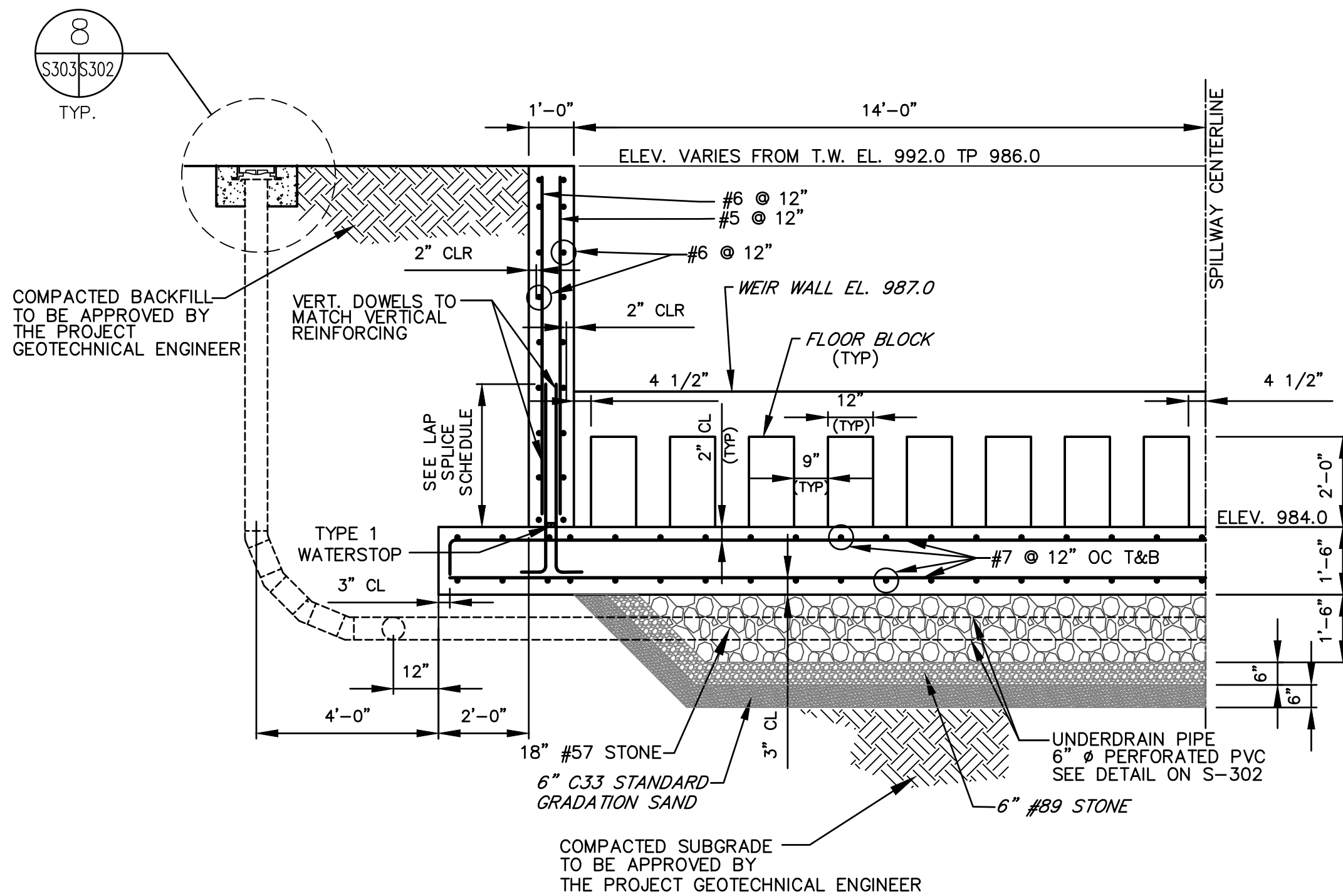


**NOTES:**

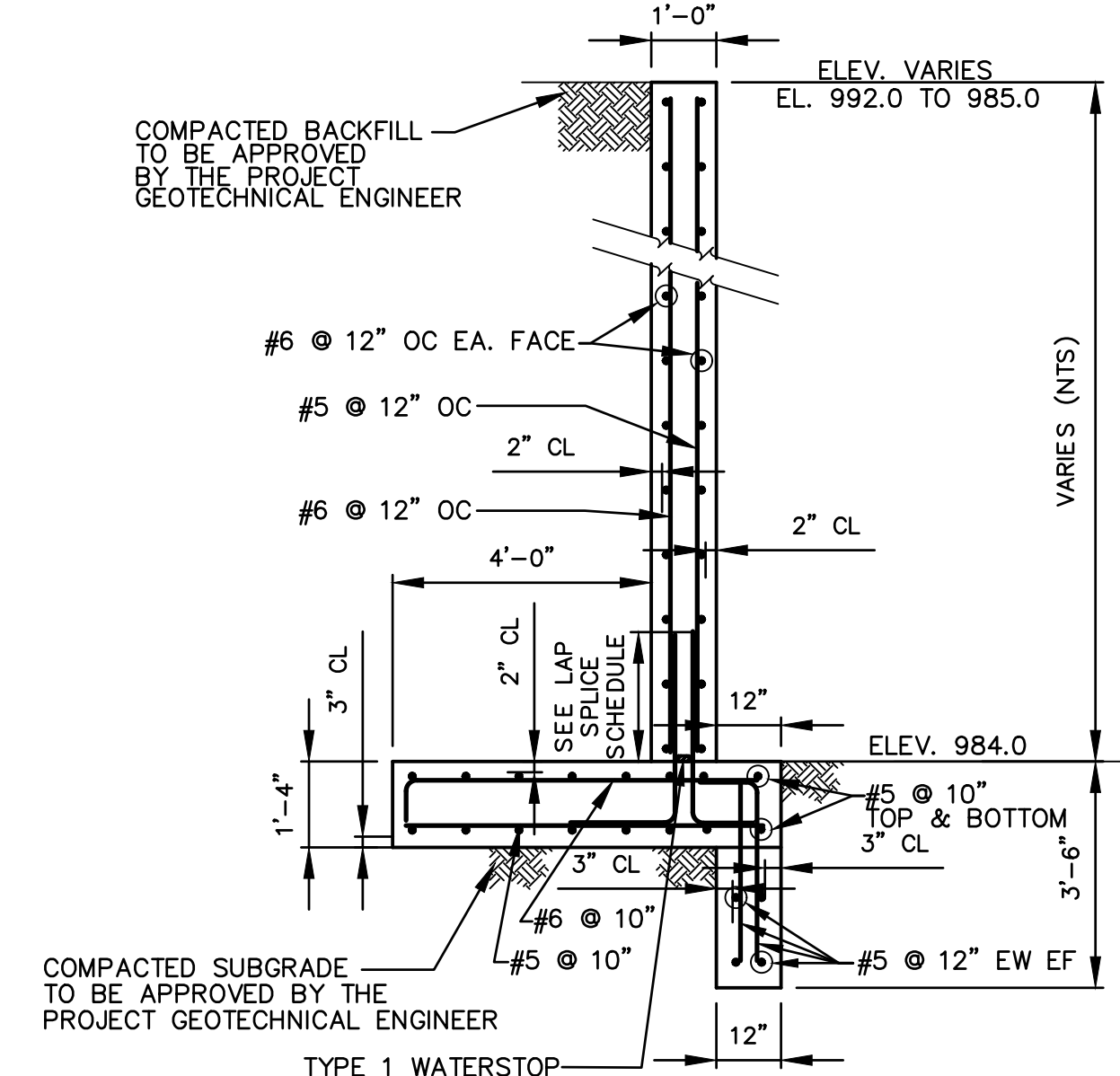
1. FOR GENERAL NOTES AND LEGEND, SEE C-100.
2. FOR GEOTECHNICAL NOTES, SEE DRAWINGS: C-101A, C-101B AND C-101C.
3. TOPOGRAPHIC SURVEY BY: GASKINS-LECRAW, INC. 1266 POWDER SPRINGS RD., MARIETTA, GA 30064 PH: 770-424-7168, DATED: OCT. 02, 2023

**STRUCTURAL NOTES**

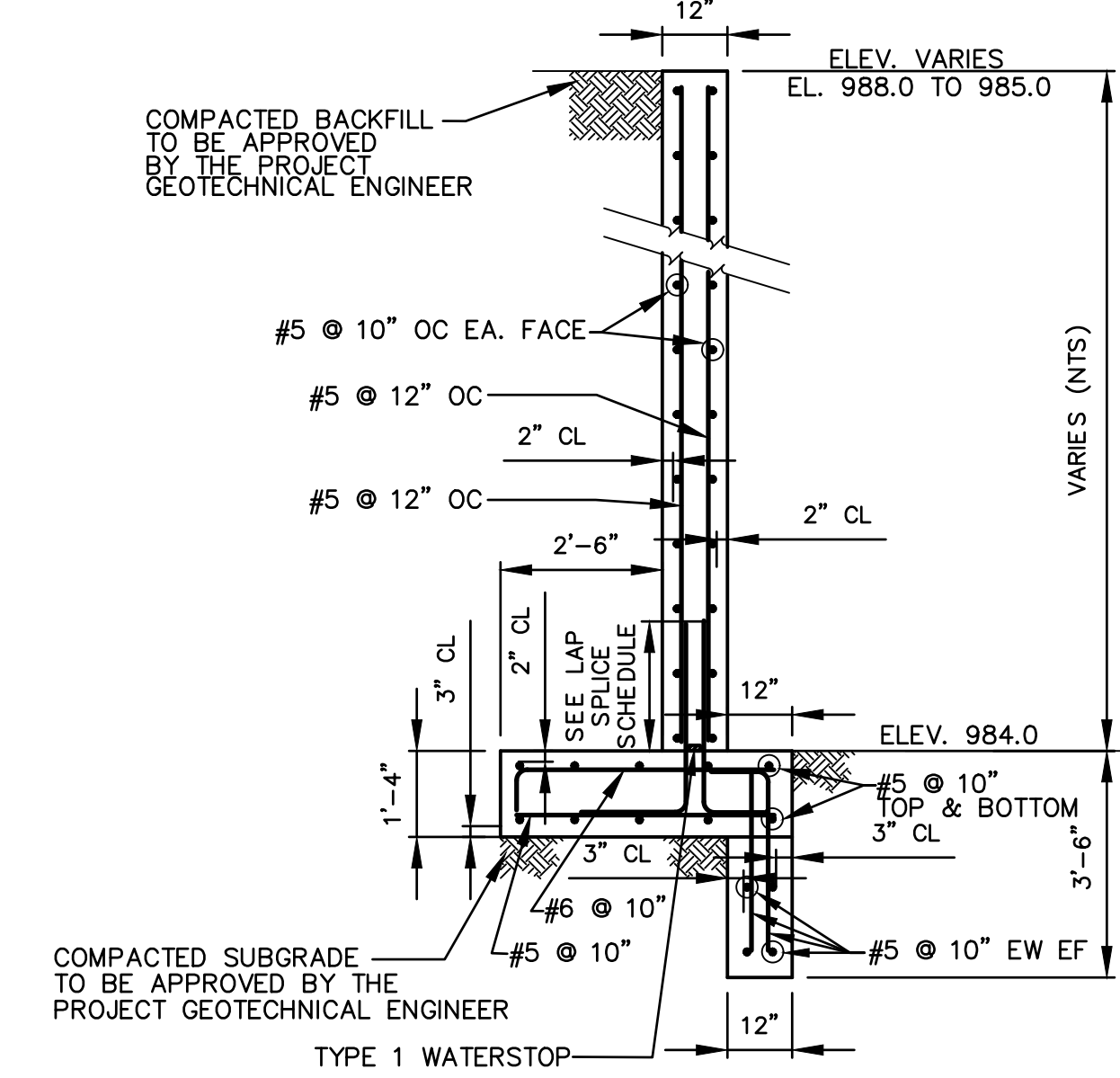
1. FOR STRUCTURAL NOTES, SEE C-100.
2. CONCRETE HATCHING NOT SHOWN FOR REBAR, DIMENSIONS, AND NOTES CLARITY.
3. SEE LAP SPLICE SCHEDULE AND NOTES ON DRAWING S-300.



**12 DETAIL AT SLAB END**  
SCALE: 3/8" = 1'-0"

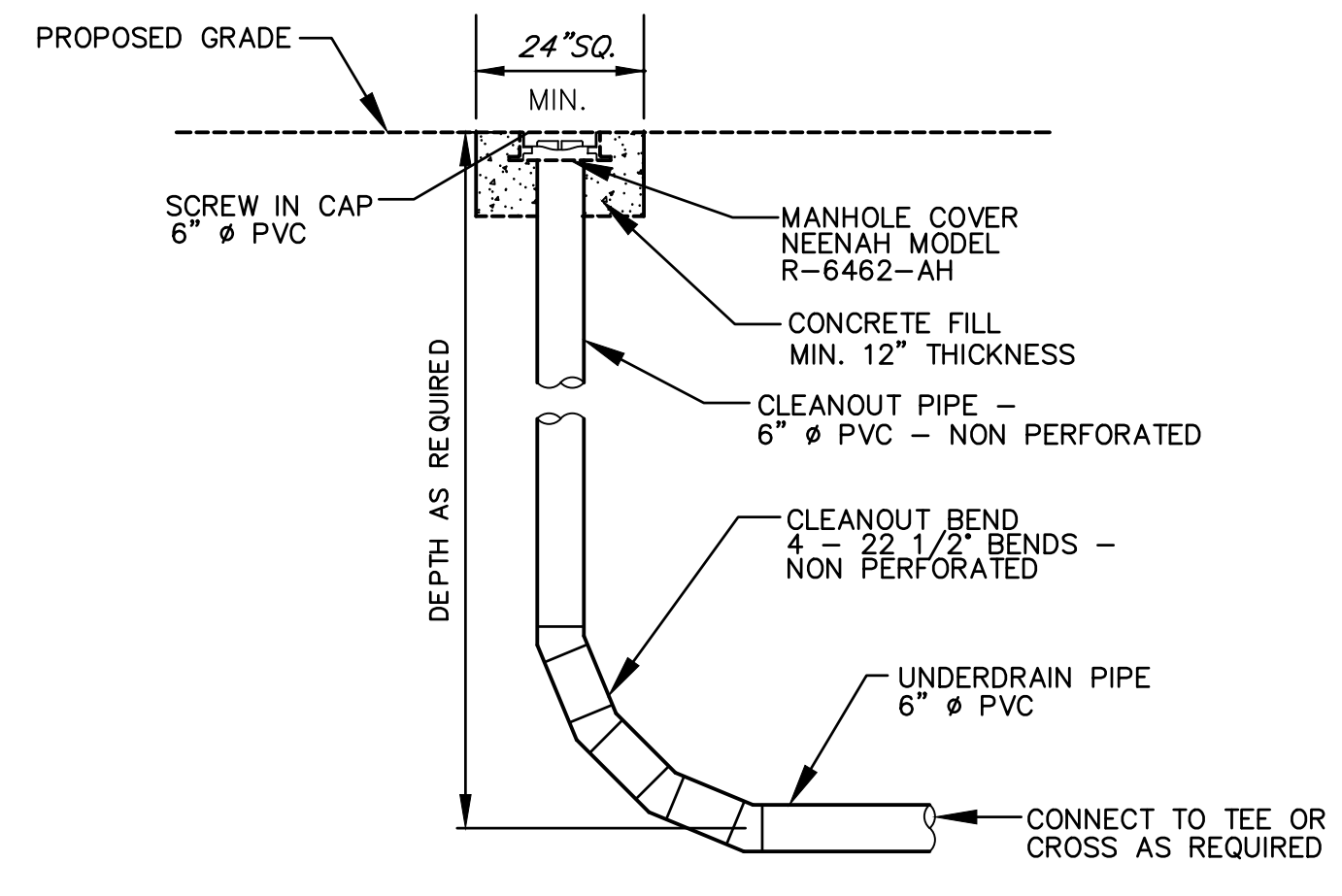


**D SECTION THRU UPPER WING WALL**  
SCALE: 3/8" = 1'-0"

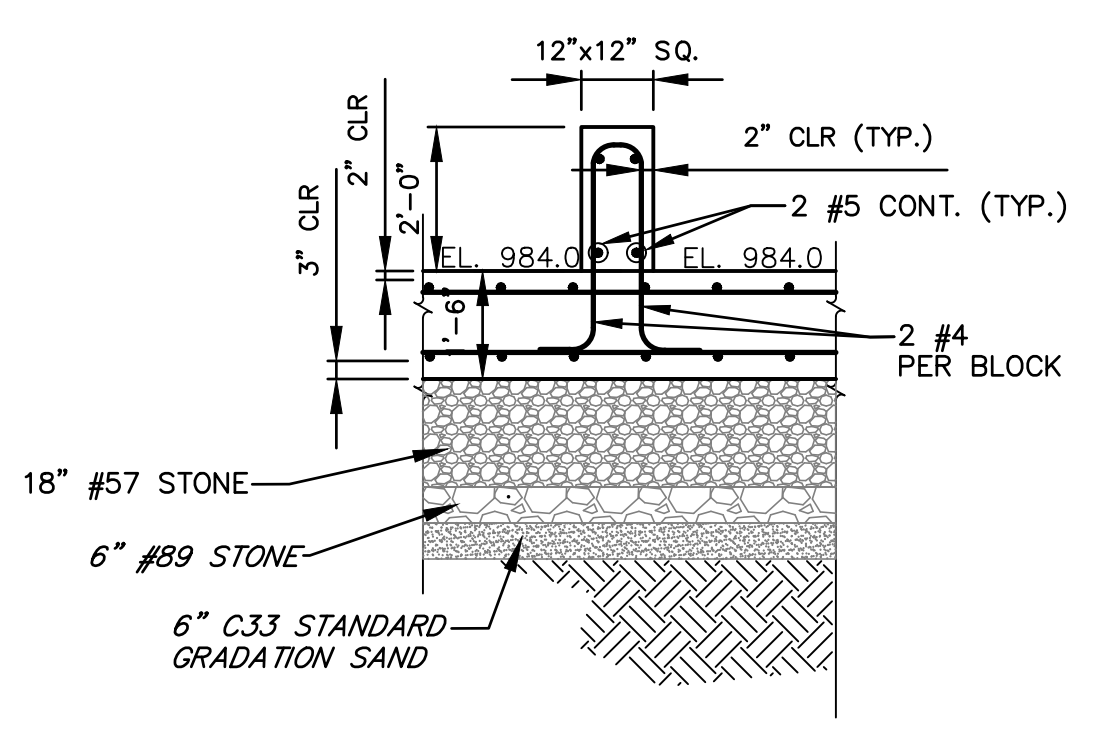


**J SECTION THRU LOWER WING WALL**  
SCALE: 3/8" = 1'-0"  
NOTE: THIS DETAIL MAY BE USED FOR ALL WALLS LESS THAN FIVE (5) FEET TALL.

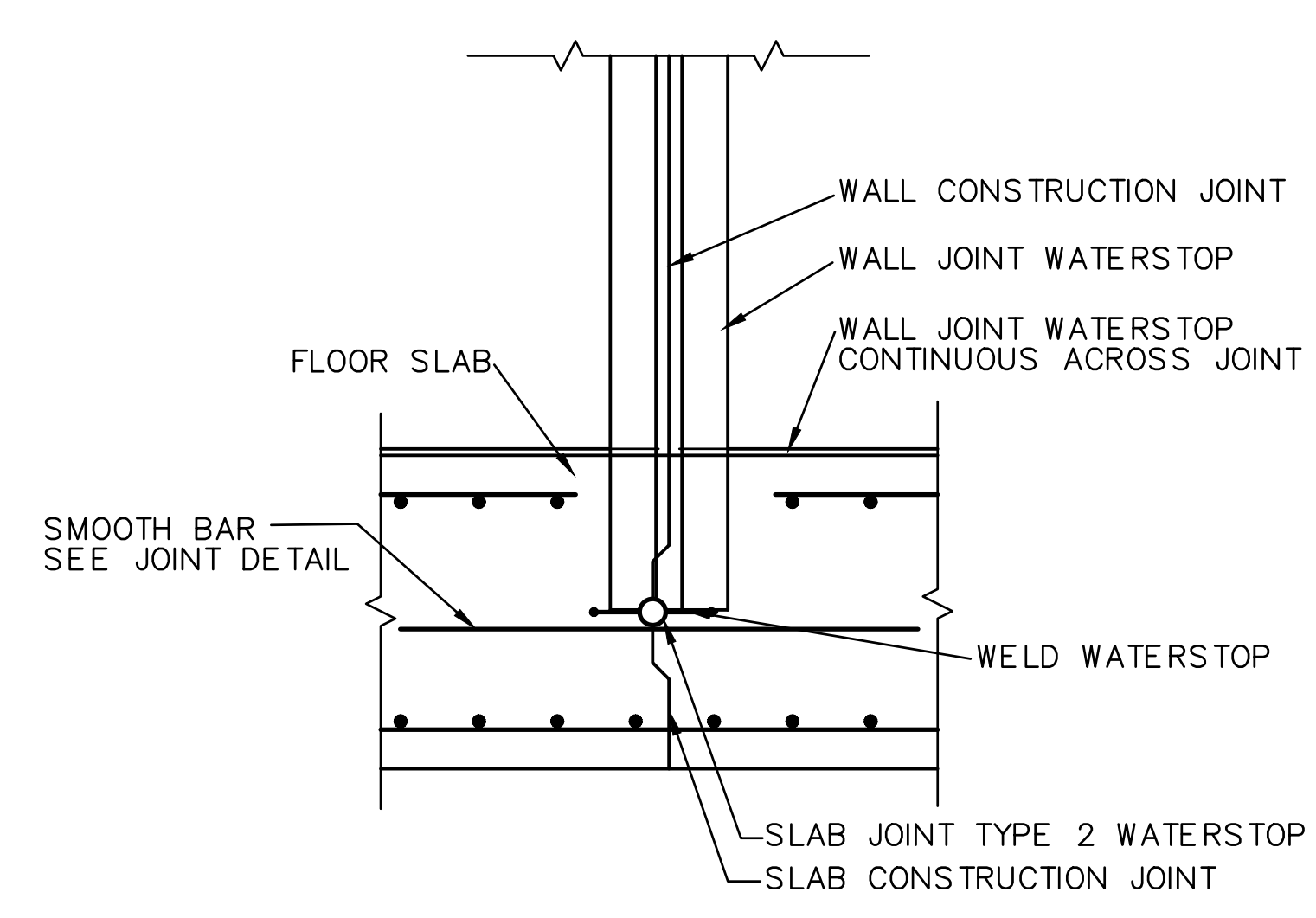
**F SECTION THRU STILLING BASIN**  
SCALE: 3/8" = 1'-0"



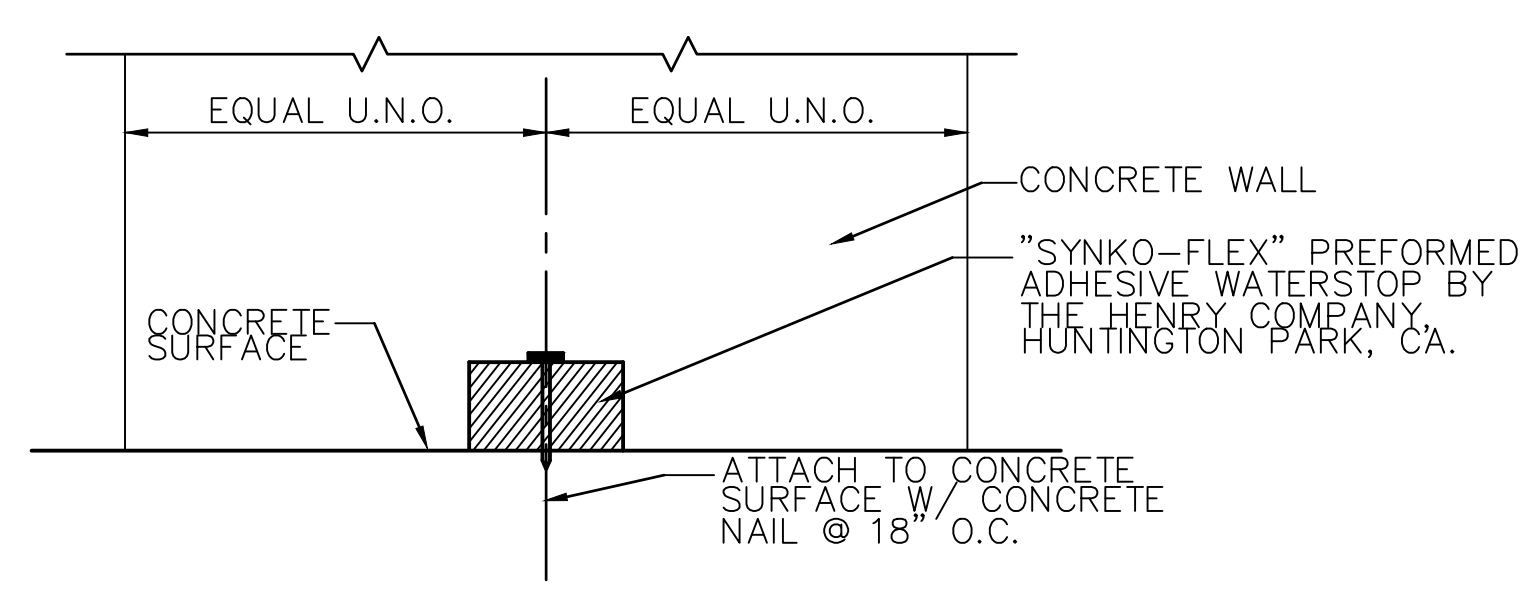
**8 DETAIL AT UNDERDRAIN CLEANOUT**  
N.T.S.



**11 DETAIL AT FLOOR BLOCK**  
SCALE: 3/8" = 1'-0"



**MOVEMENT JOINT INTERSECTION DETAIL**  
N.T.S.



**TYPE 1 WATERSTOP DETAIL**  
N.T.S. (FOR USE AT CONSTRUCTION JOINTS)

**PRIMARY PERMITTEE**  
OWNER:  
CITY OF TUCKER  
PAKS AND RECREATION DEPT.  
4898 LAVISTA RD, TUCKER, GA 30084  
CONTACT: RIP ROBERTSON, DIRECTOR  
PHONE: (470) 481-0205  
EMAIL: rrobertson@tuckerga.gov

**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205

**811**  
Know what's below.  
Call before you dig.

NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

**GEORGIA REGISTERED PROFESSIONAL ENGINEER**  
JASON F. RAPPLEAN  
No. 20441  
EXPIRES 06/30/2024

SCALE: AS SHOWN  
DESIGN BY: JR/JH 01/23  
DRAWN BY: JH/EC 01/23  
CHECKED BY: JR 01/23  
CD# PEF 000707 EXP: 06/30/2024

JASON RAPPLEAN, PE, EDR  
LEVEL 2 CERTIFICATION # 3031  
EXPIRATION DATE: 12/07/26

06/19/2024

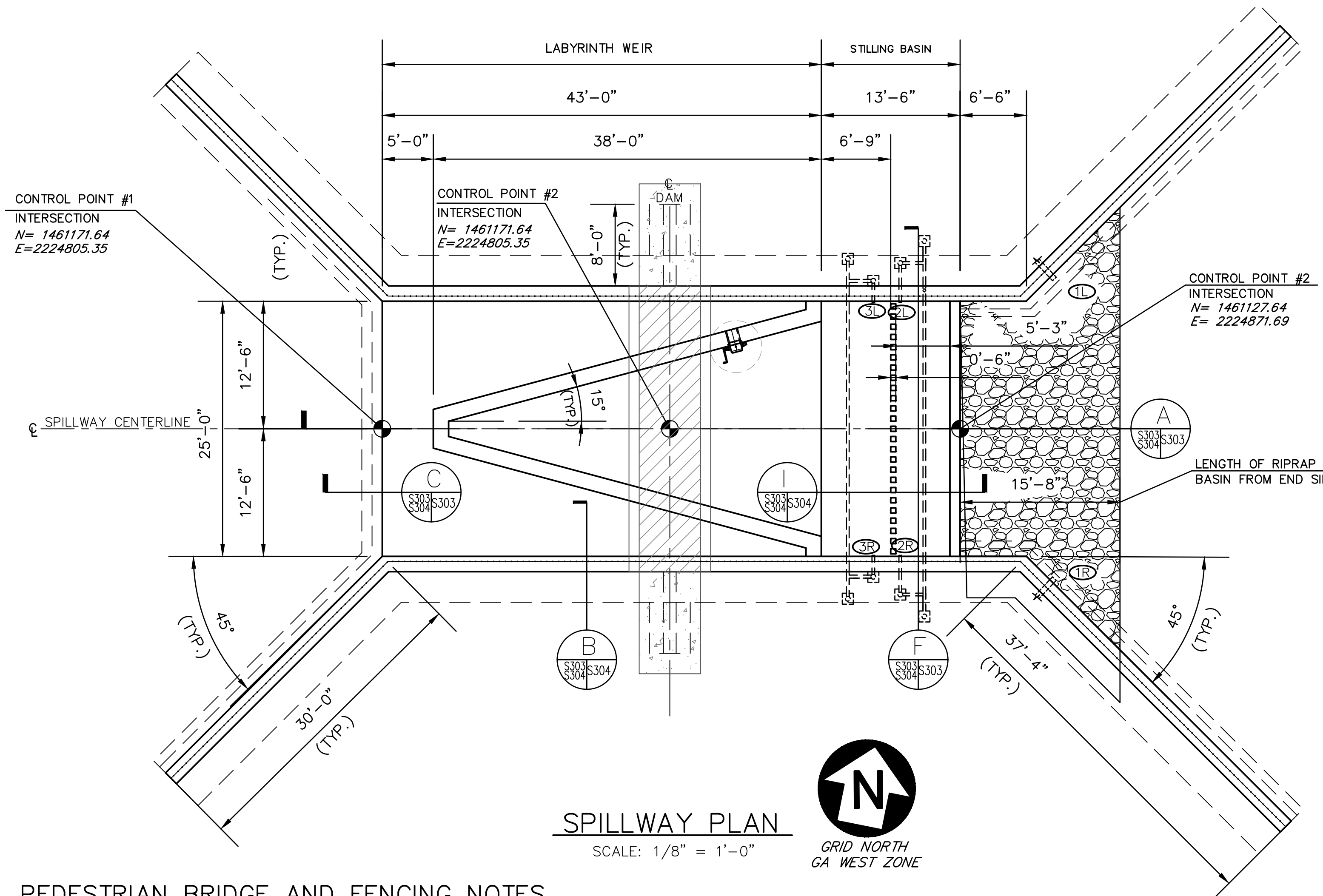
**JOHNS HOMESTEAD PARK**  
**STRUCTURAL**  
**UPPER SPILLWAY PLAN, SECTIONS AND DETAILS**

**CITY OF TUCKER**

**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
CONSULTING ENGINEERS  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
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4201600 **42016-S-302** 0



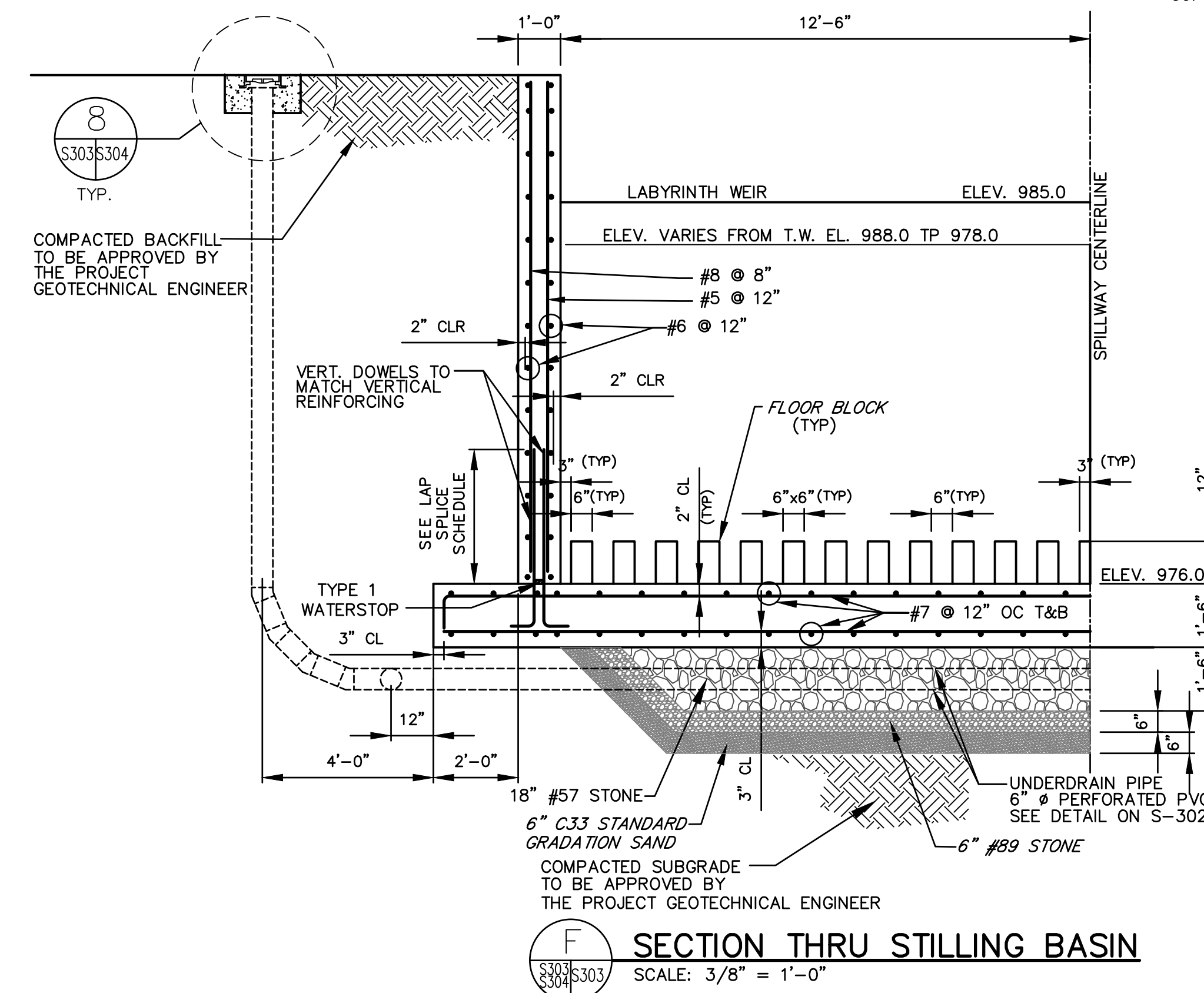


**PEDESTRIAN BRIDGE AND FENCING NOTES**

- FOR PEDESTRIAN BRIDGE(S) DETAILS, SEE PLANS BY OTHERS.
- REFER TO SPILLWAY BRIDGE NOTES LOCATED ON DWG. C-100 FOR ADDITIONAL INFORMATION.

**ADD ALTERNATE FOR UTILIZING ELASTOMERIC FORMLINER(S) FOR TEXTURING CONCRETE TO PROVIDE STANDARD PATTERNS:**

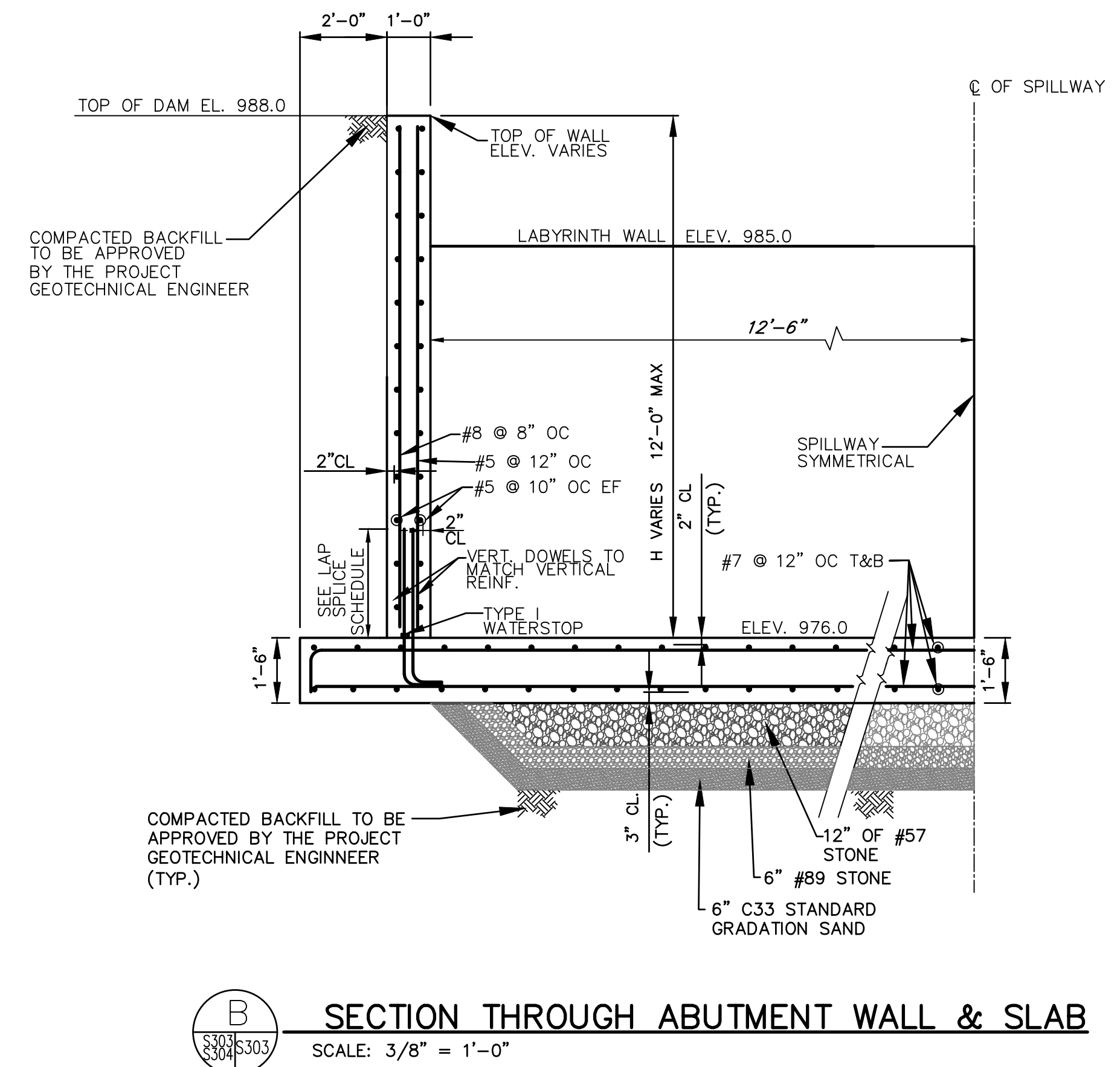
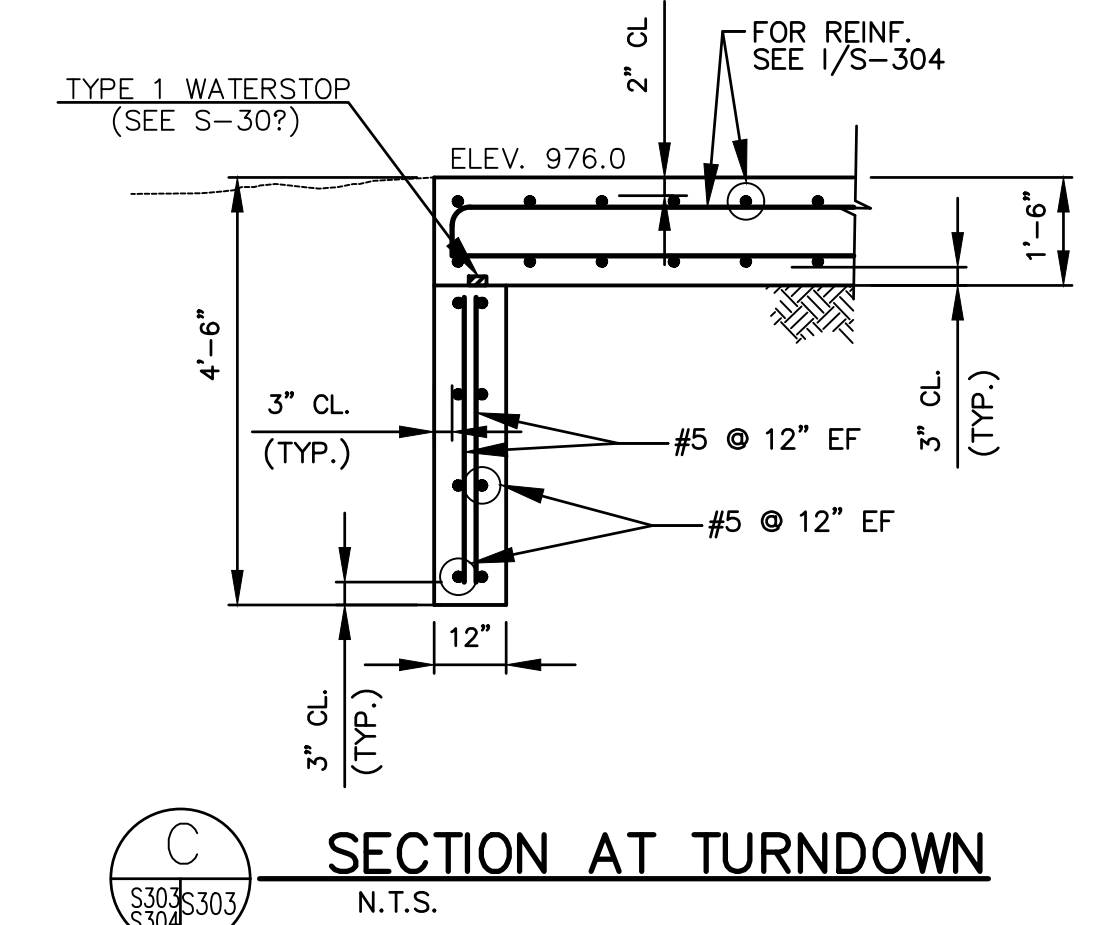
- THIS ADD ALTERNATE WILL BE IN THE BID DOCUMENTS AND IS TO BE USED TO DETERMINE THE COSTS ASSOCIATED WITH PROVIDING AN ARCHITECTURAL TEXTURING PATTERN IN THE EXPOSED SPILLWAY'S CONCRETE WALLS.
- REFER TO SPECIFICATION SECTION 03-11-16 FOR ADDITIONAL INFORMATION.
- IF THERE IS SUFFICIENT BUDGET, THE PATTERNS WILL BE ADDED TO THE SPILLWAY'S CONSTRUCTION.



- NOTES:**
- FOR GENERAL NOTES AND LEGEND, SEE C-100.
  - FOR GEOTECHNICAL NOTES, SEE DRAWINGS: C-101A, C-101B, AND C-101C.
  - TOPOGRAPHIC SURVEY BY: GASKINS-LECRAW, INC. 1266 POWDER SPRINGS RD, MARIETTA, GA 30064 PH: 770-424-7168, DATED: OCT. 02, 2023

**STRUCTURAL NOTES**

- FOR STRUCTURAL NOTES, SEE C-100.
- CONCRETE HATCHING NOT SHOWN FOR REBAR, DIMENSIONS, AND NOTES CLARITY.
- SEE LAP SPLICE SCHEDULE AND NOTES ON DRAWING S-300.
- EXPOSED CONCRETE FINISH TO BE DETERMINED BY THE OWNER AND/OR THEIR REPRESENTATIVE(S) (I.E.: TEXTURE, COLOR, ETC.).



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(470) 481-0205

NO.	REVISIONS	MADE	CKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS



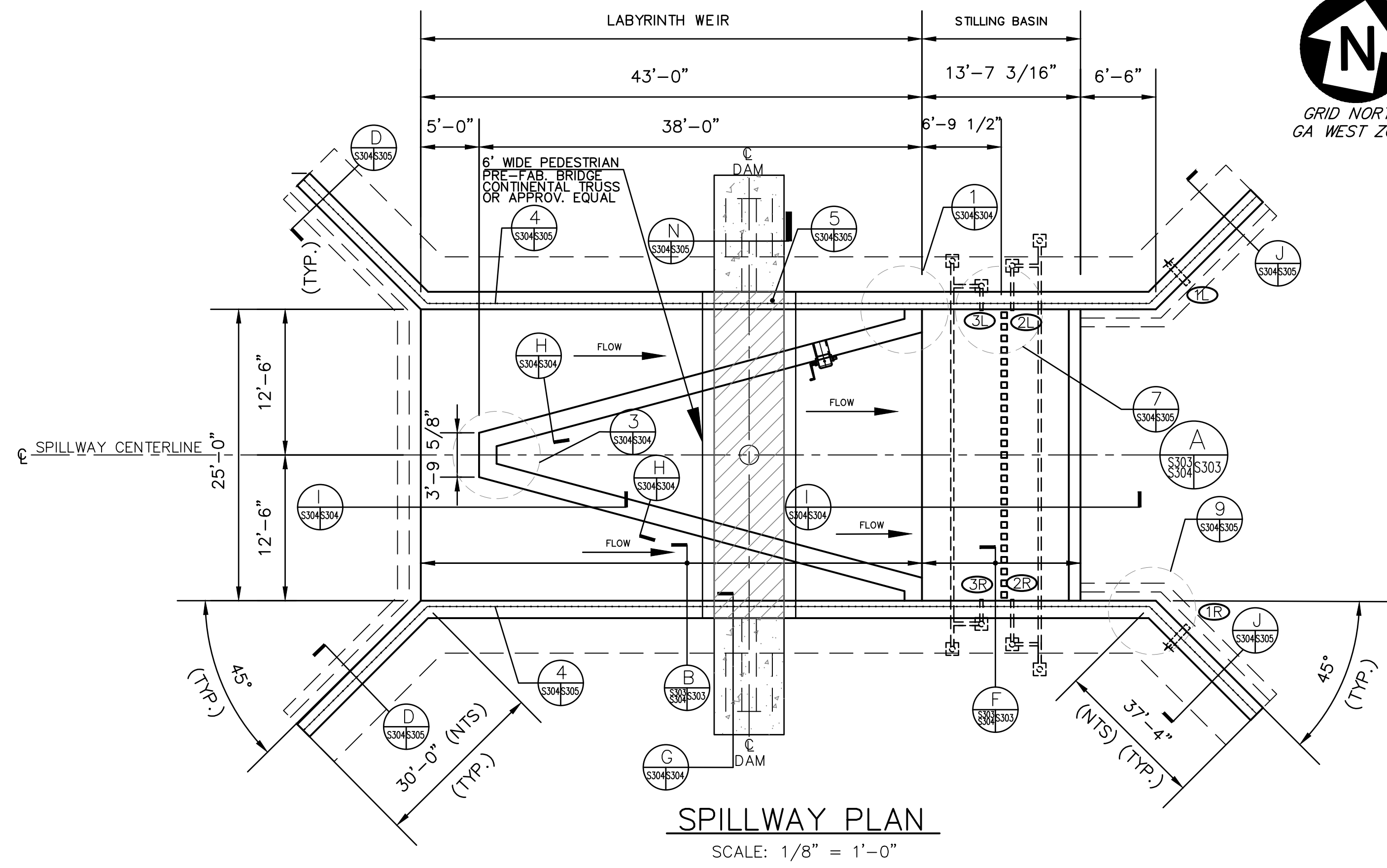
SCALE: AS SHOWN	DATE
DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE:	12/07/26

**JOHNS HOMESTEAD PARK**  
**STRUCTURAL**  
**LOWER SPILLWAY**  
**CITY OF TUCKER**

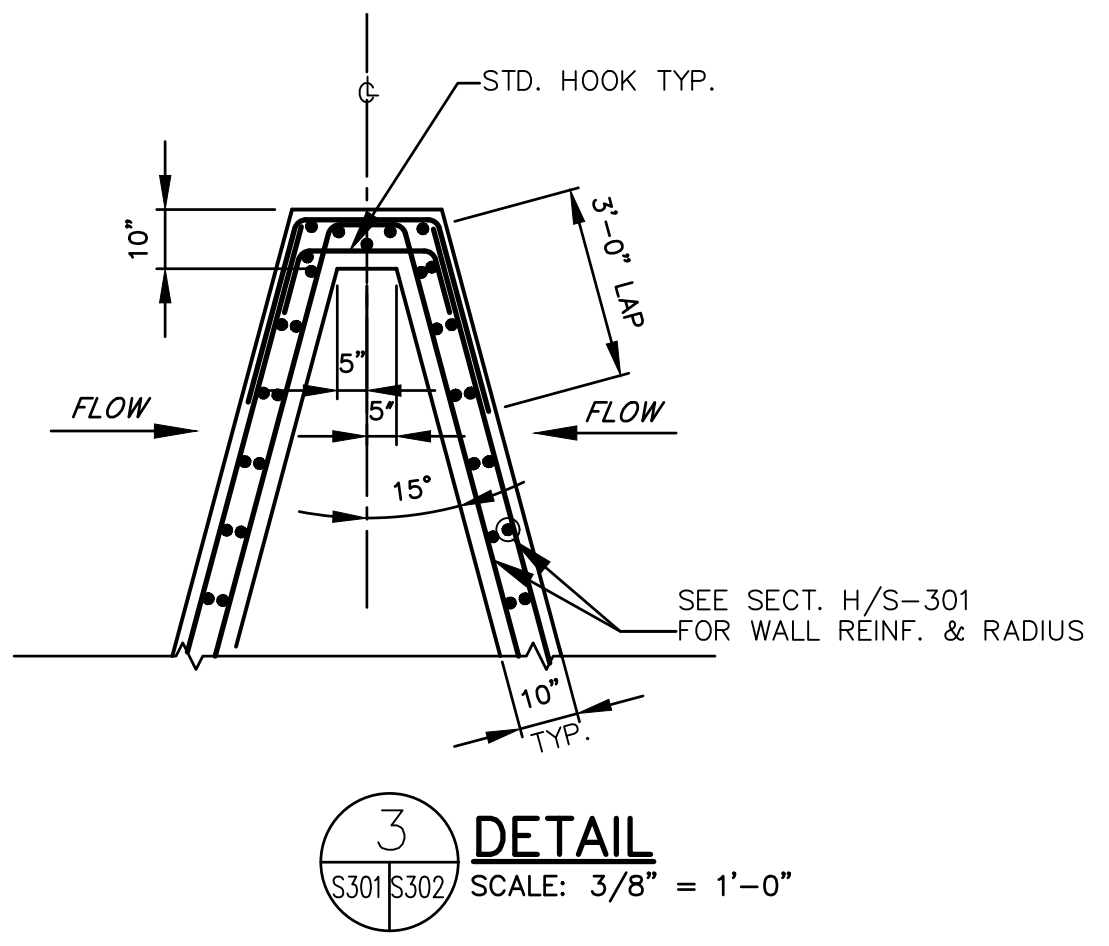
**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
CONSULTING ENGINEERS  
P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
(770) 956 - 7879

4201600 **42016- S-303** **0**

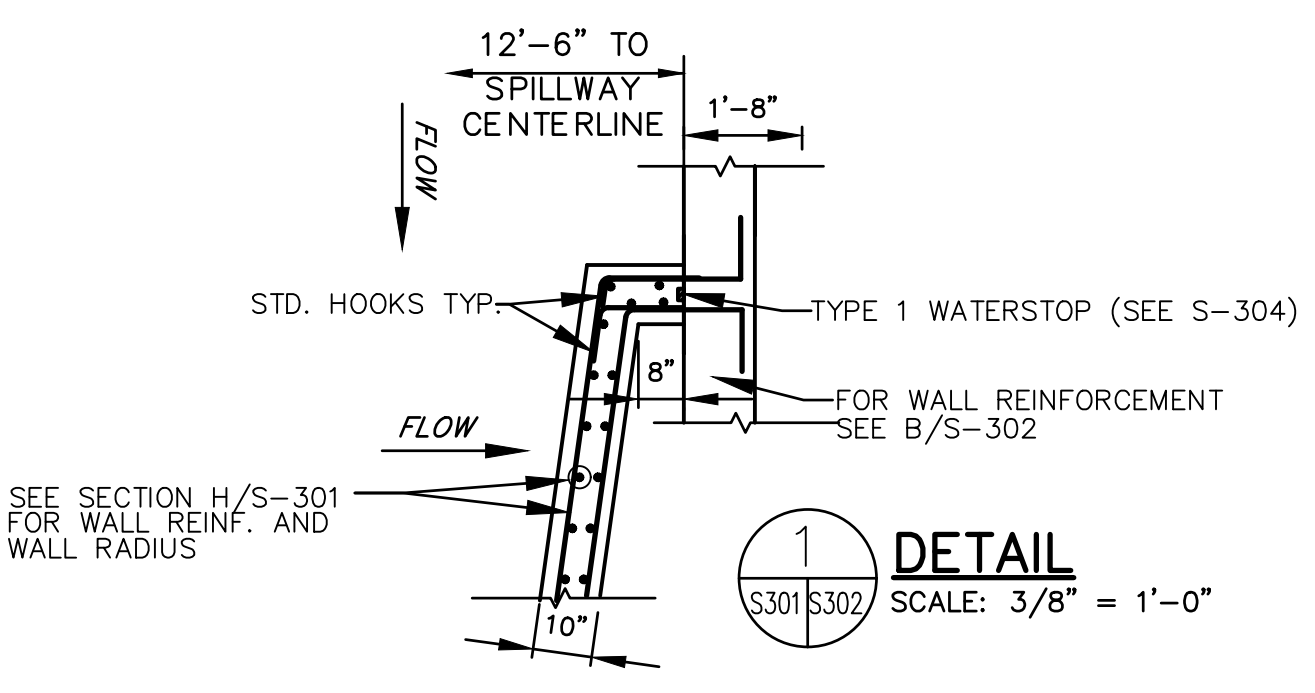




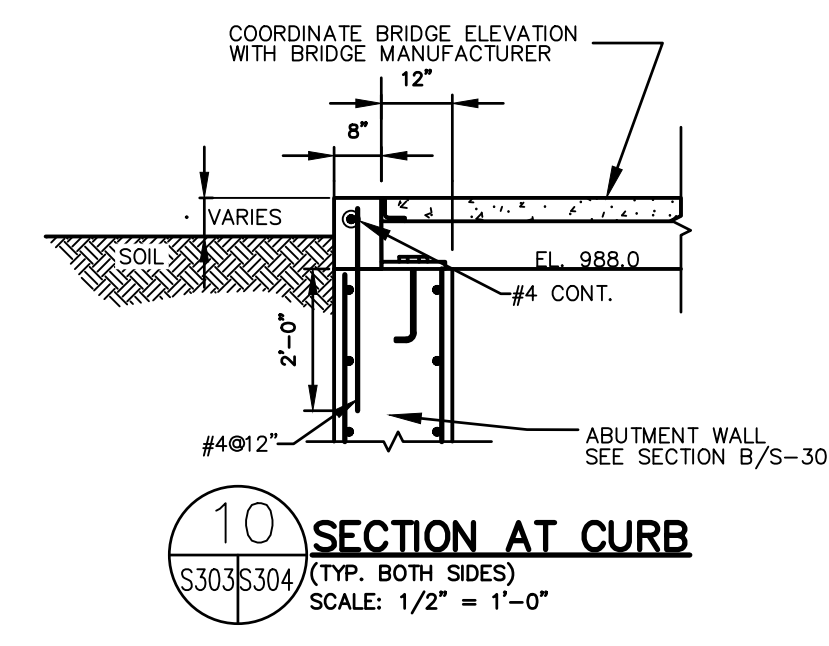
**SPILLWAY PLAN**  
SCALE: 1/8" = 1'-0"



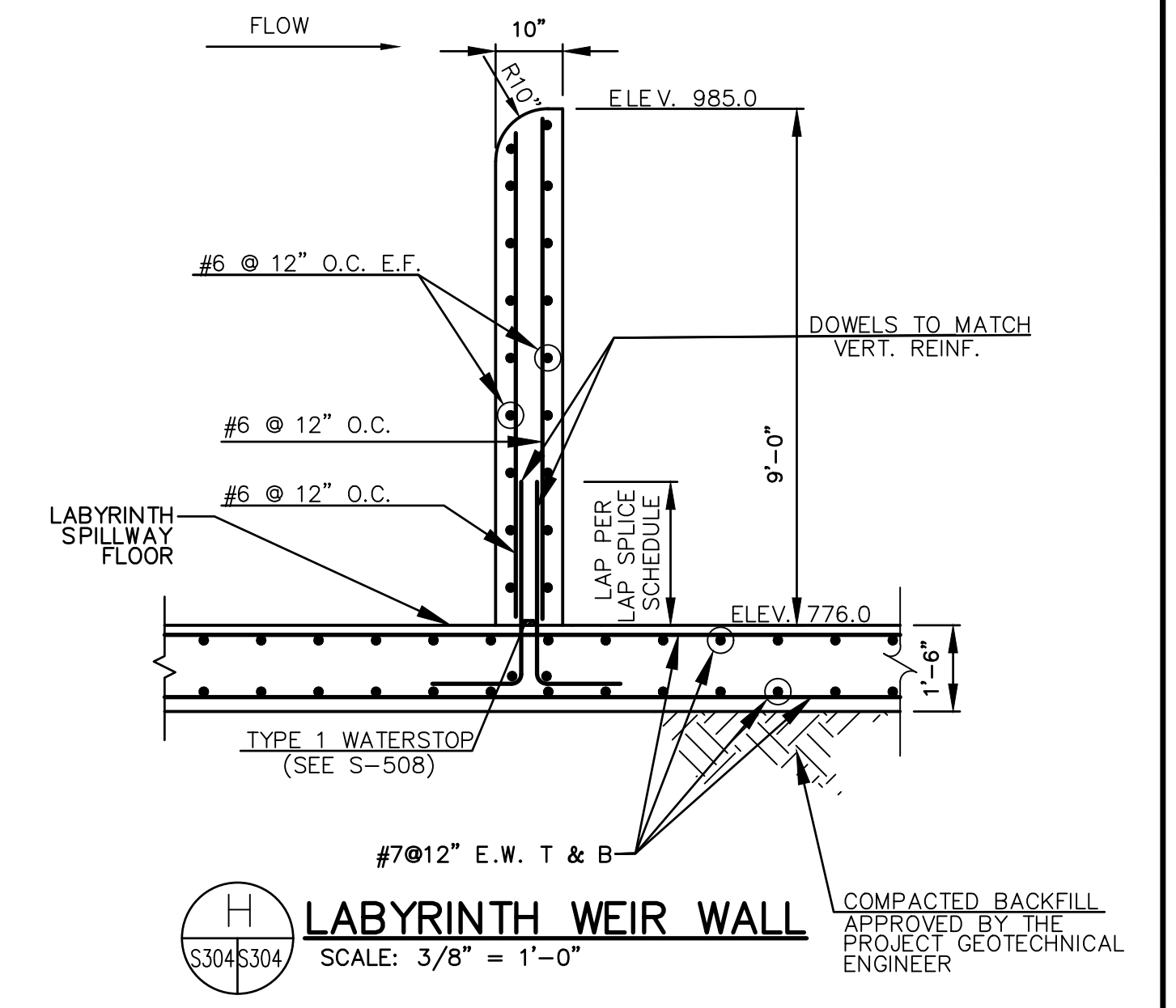
**3 DETAIL**  
SCALE: 3/8" = 1'-0"



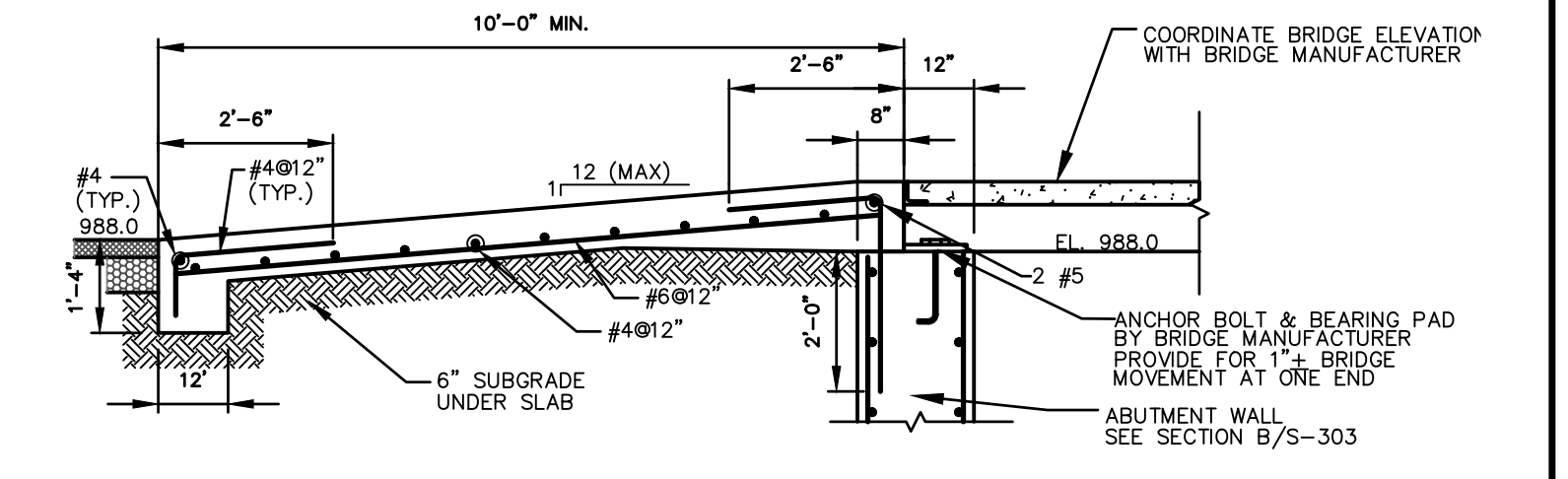
**1 DETAIL**  
SCALE: 3/8" = 1'-0"



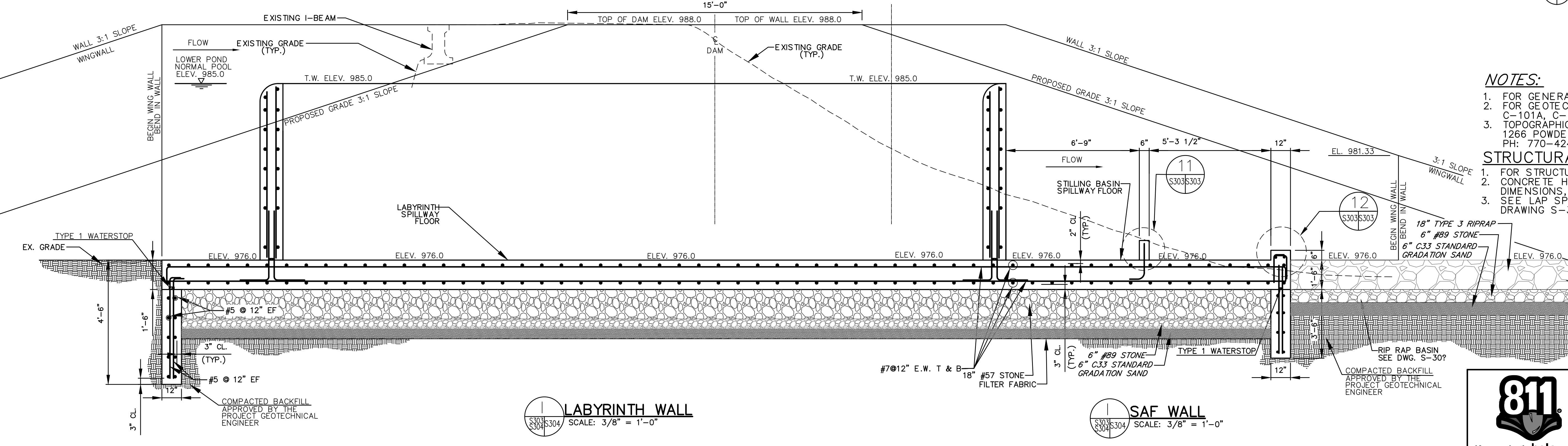
**10 SECTION AT CURB**  
SCALE: 1/2" = 1'-0"



**H LABYRINTH WEIR WALL**  
SCALE: 3/8" = 1'-0"



**G BRIDGE BEARING**  
SCALE: 1/2" = 1'-0"



**I LABYRINTH WALL**  
SCALE: 3/8" = 1'-0"

**I SAF WALL**  
SCALE: 3/8" = 1'-0"

- NOTES:**
- FOR GENERAL NOTES AND LEGEND, SEE C-100.
  - FOR GEOTECHNICAL NOTES, SEE DRAWINGS: C-101A, C-101B, AND C-101C.
  - TOPOGRAPHIC SURVEY BY: GASKINS-LE CRAW, INC. 1266 POWDER SPRINGS RD., MARIETTA, GA 30064 PH: 770-424-7168, DATED: OCT. 02. 2023
- STRUCTURAL NOTES**
- FOR STRUCTURAL NOTES, SEE C-100.
  - CONCRETE HATCHING NOT SHOWN FOR REBAR, DIMENSIONS, AND NOTES CLARITY.
  - SEE LAP SPLICE SCHEDULE AND NOTES ON DRAWING S-300.

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NO.	REVISIONS	MADE	CHKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS



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JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE:	12/07/26

**JOHNS HOMESTEAD PARK**  
**STRUCTURAL**  
**LOWER SPILLWAY**

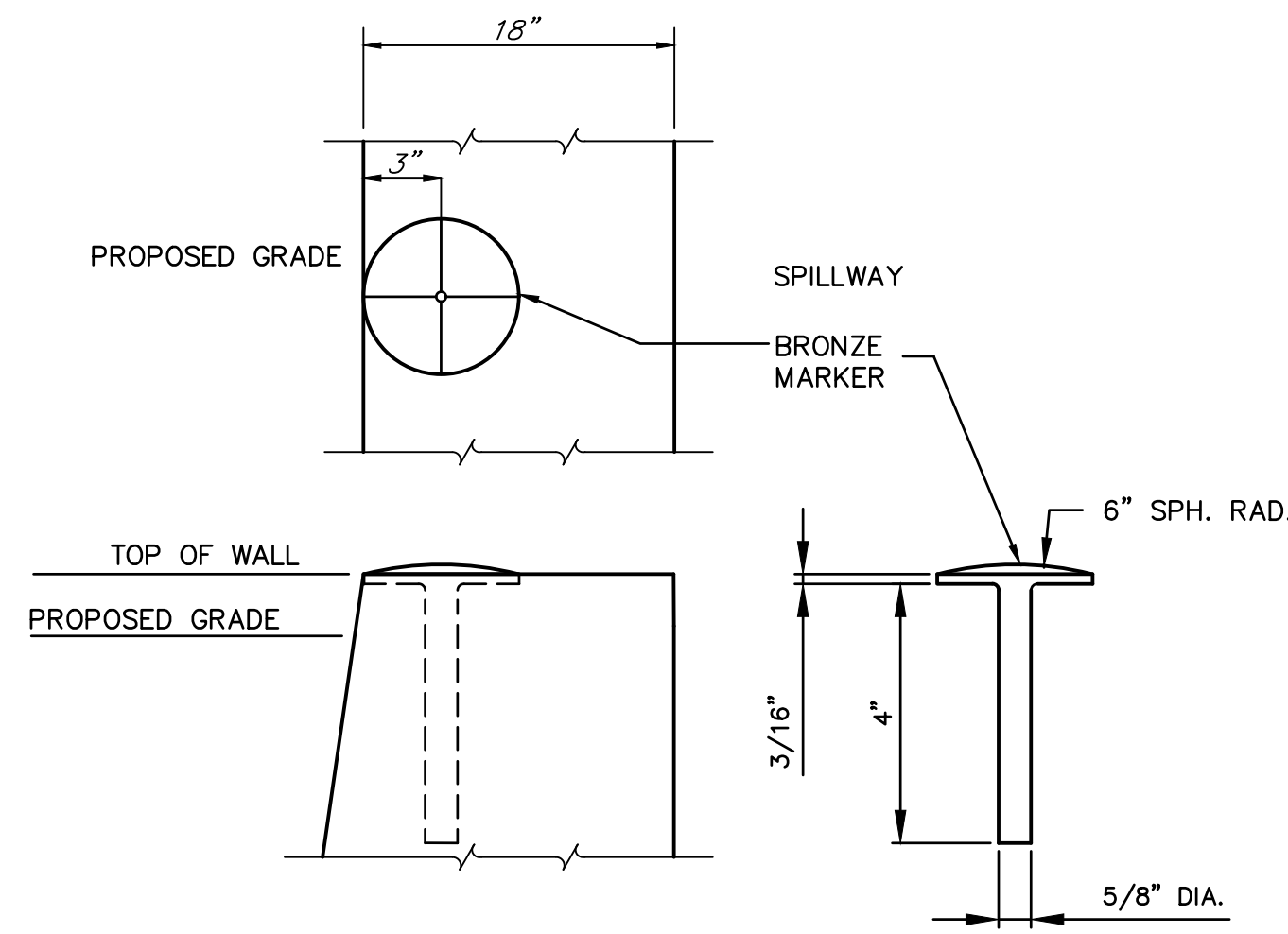
**CITY OF TUCKER**



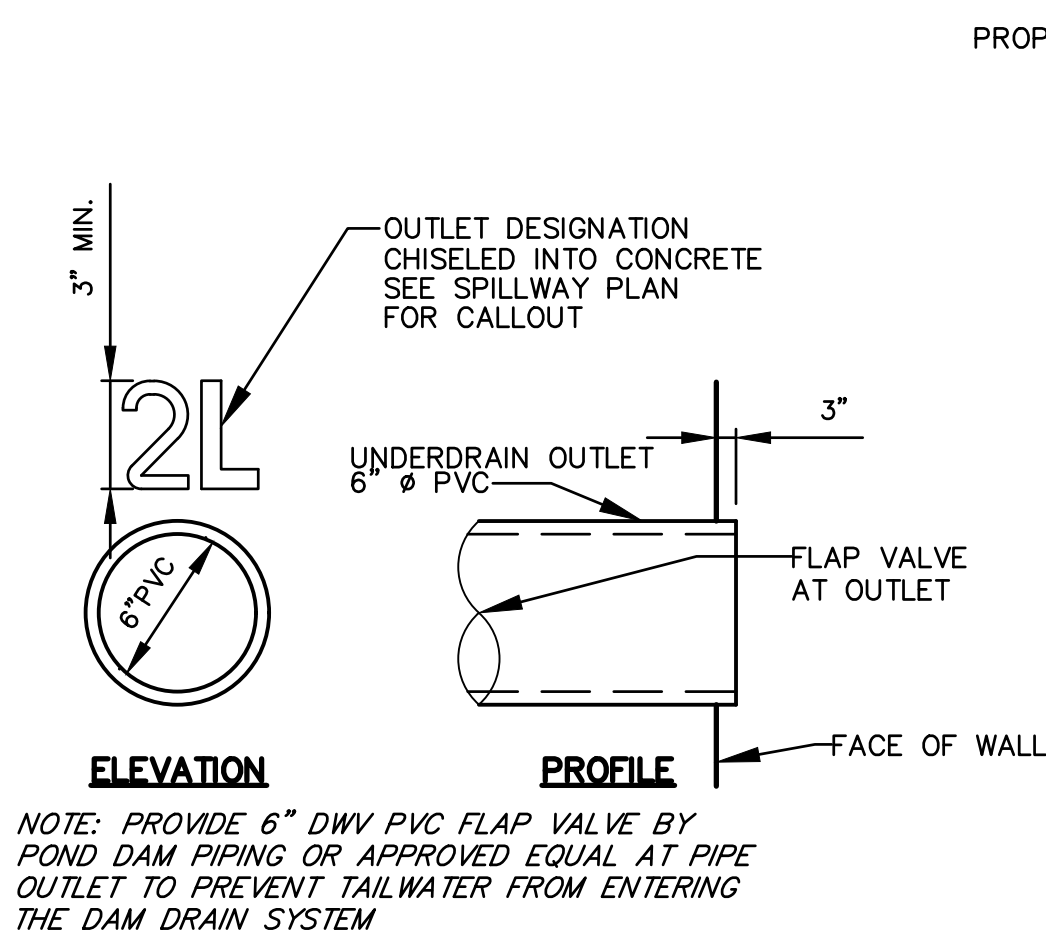
**WALDEN, ASHWORTH & ASSOCIATES, INC.**  
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P.O. BOX 6462  
MARIETTA, GEORGIA 30065  
(770) 956-7879

420600 **42016-S-304** **0**

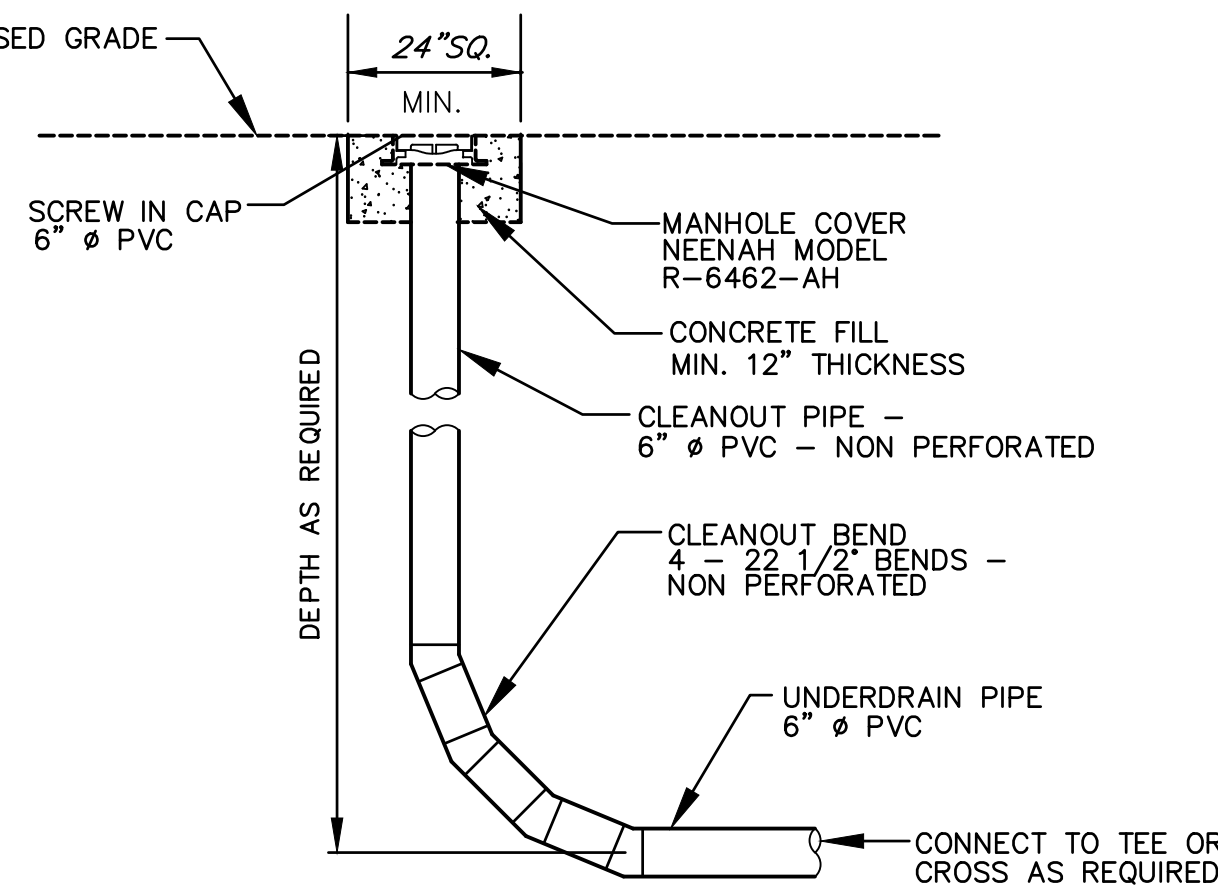




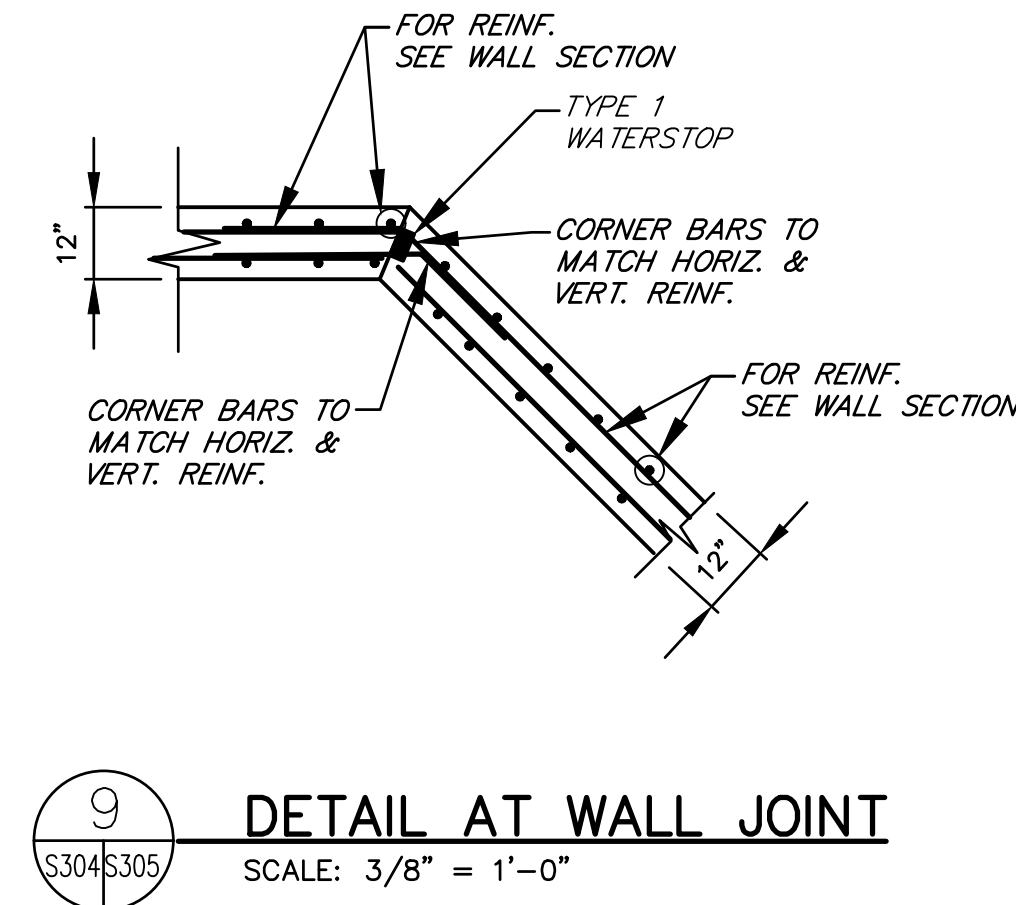
**5 SETTLEMENT PLATE DETAIL**  
SCALE: NTS



**7 DETAIL AT UNDERDRAIN OUTLET**  
N.T.S. (TYPICAL 8 PLACES)



**8 DETAIL AT UNDERDRAIN CLEANOUT**  
N.T.S.



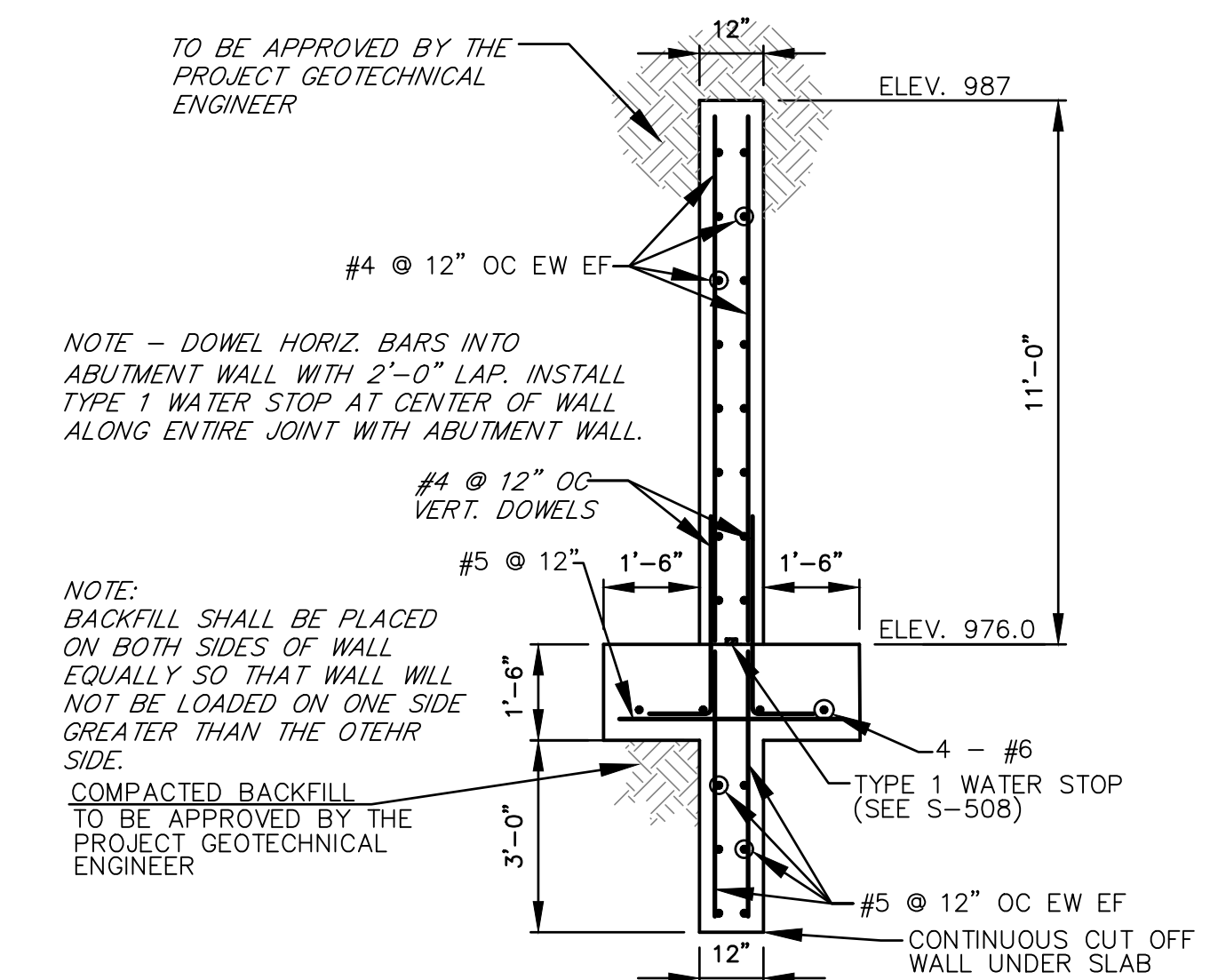
**9 DETAIL AT WALL JOINT**  
SCALE: 3/8" = 1'-0"

**NOTES:**

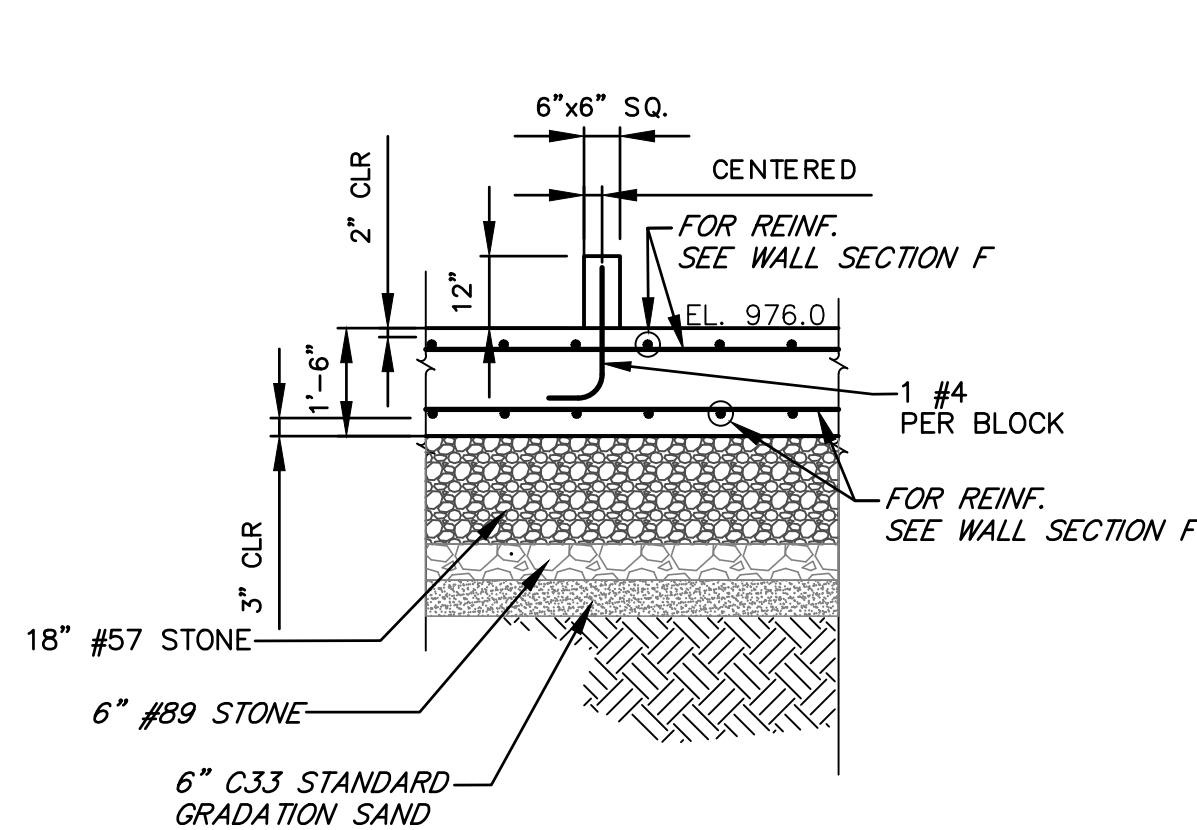
- FOR GENERAL NOTES AND LEGEND, SEE C-100.
- FOR GEOTECHNICAL NOTES, SEE DRAWINGS: C-101A, C-101B, AND C-101C.
- TOPOGRAPHIC SURVEY BY: GASKINS-LoCRAW, INC. 1266 POWDER SPRINGS RD., MARIETTA, GA 30064 PH: 770-424-7168, DATED: OCT. 02, 2023

**STRUCTURAL NOTES**

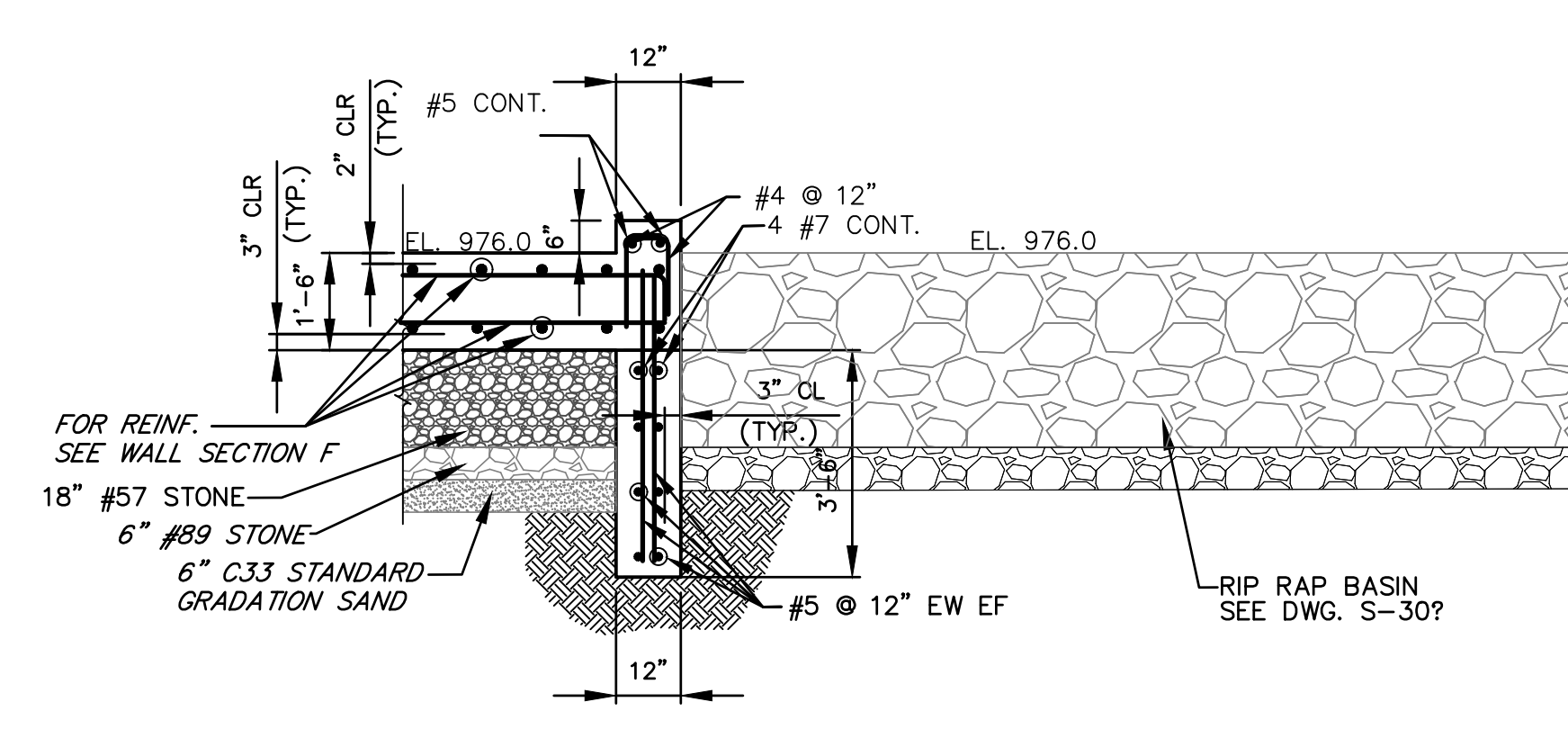
- FOR STRUCTURAL NOTES, SEE C-100.
- CONCRETE HATCHING NOT SHOWN FOR REBAR, DIMENSIONS, AND NOTES CLARITY.
- SEE LAP SPlice SCHEDULE AND NOTES ON DRAWING S-300.



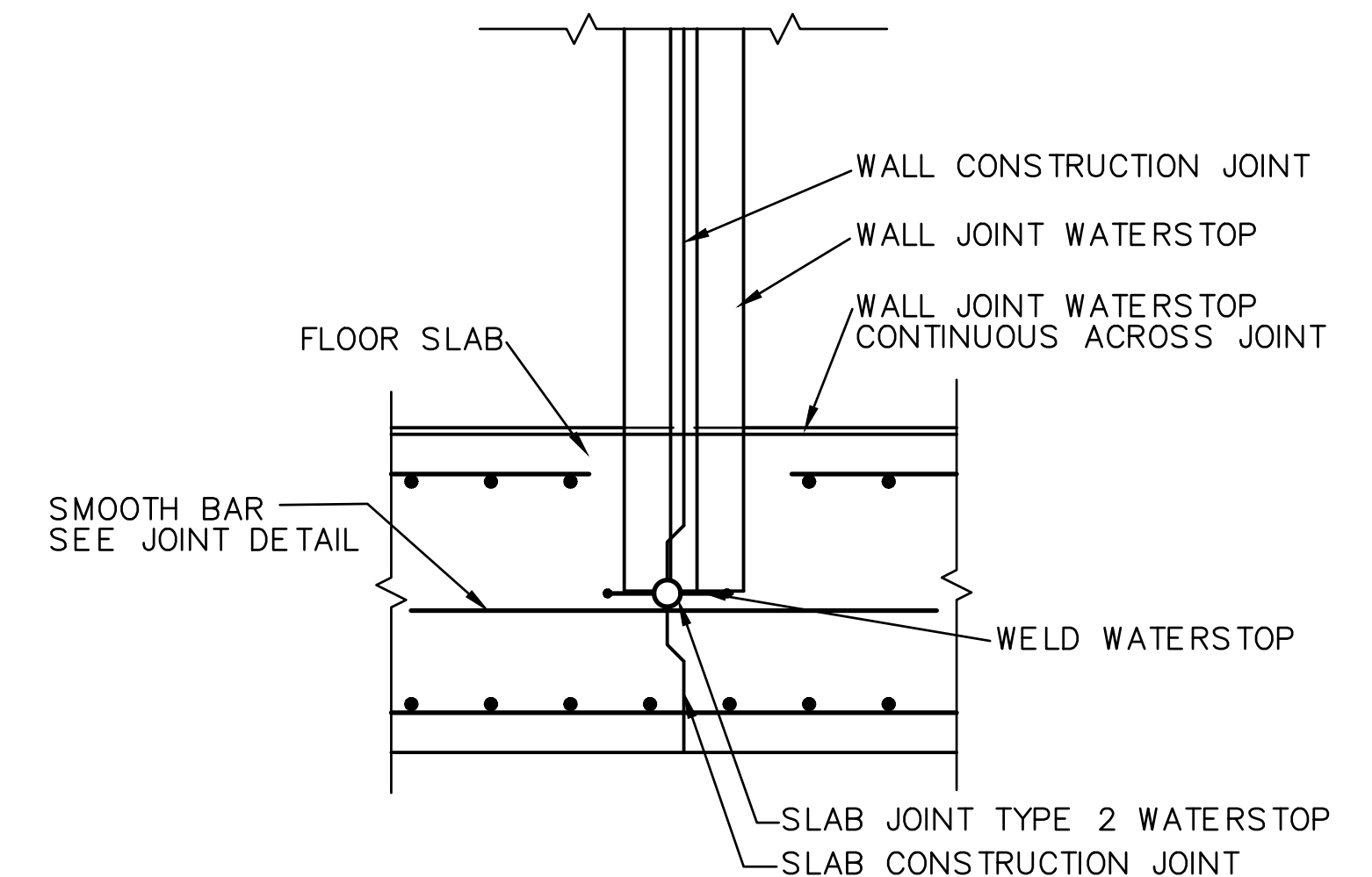
**N SECTION THRU CUT-OFF WALL (BEYOND SPILLWAY SIDE WALL)**  
SCALE: 3/8" = 1'-0"



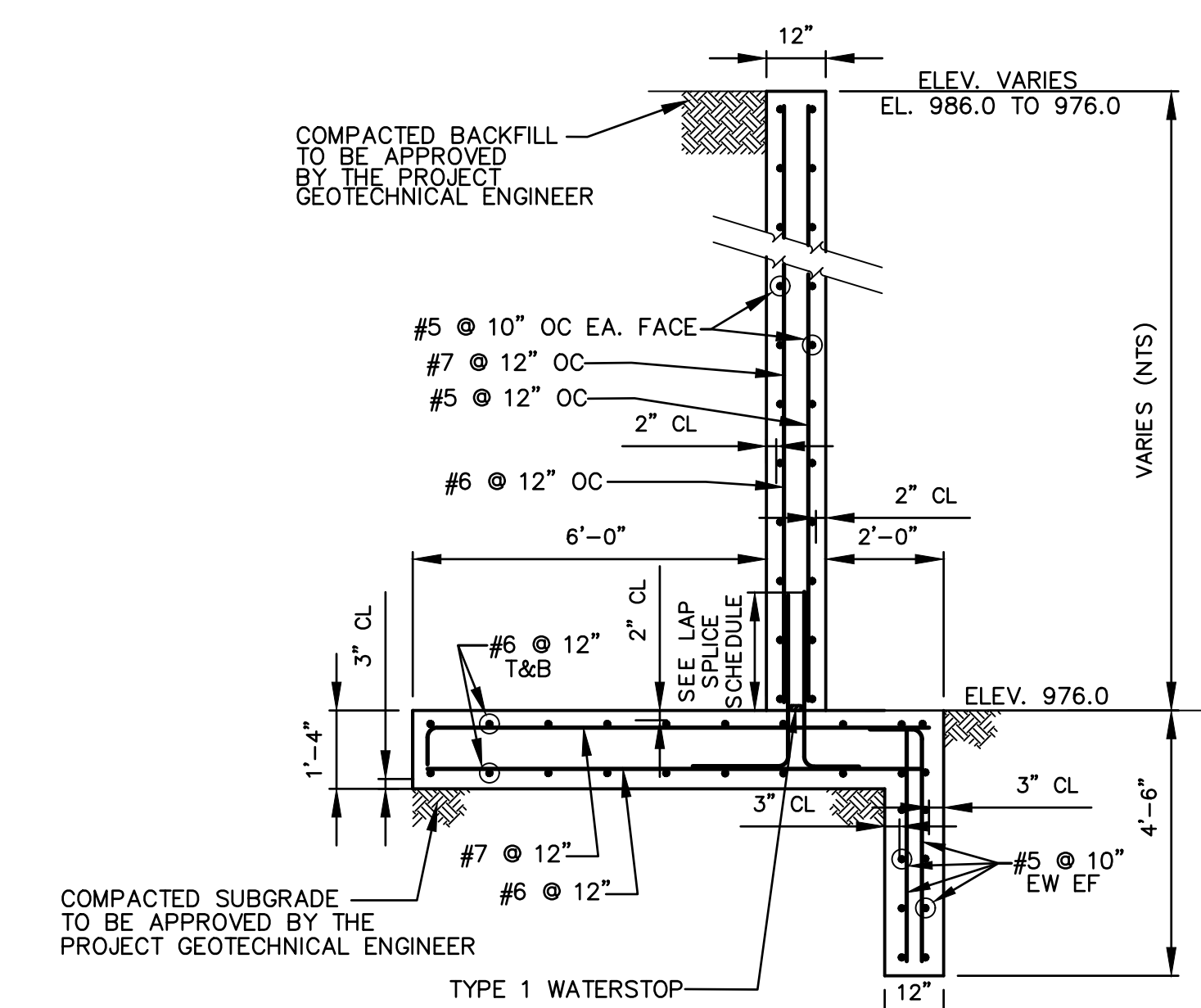
**11 DETAIL AT FLOOR BLOCK**  
SCALE: 3/8" = 1'-0"



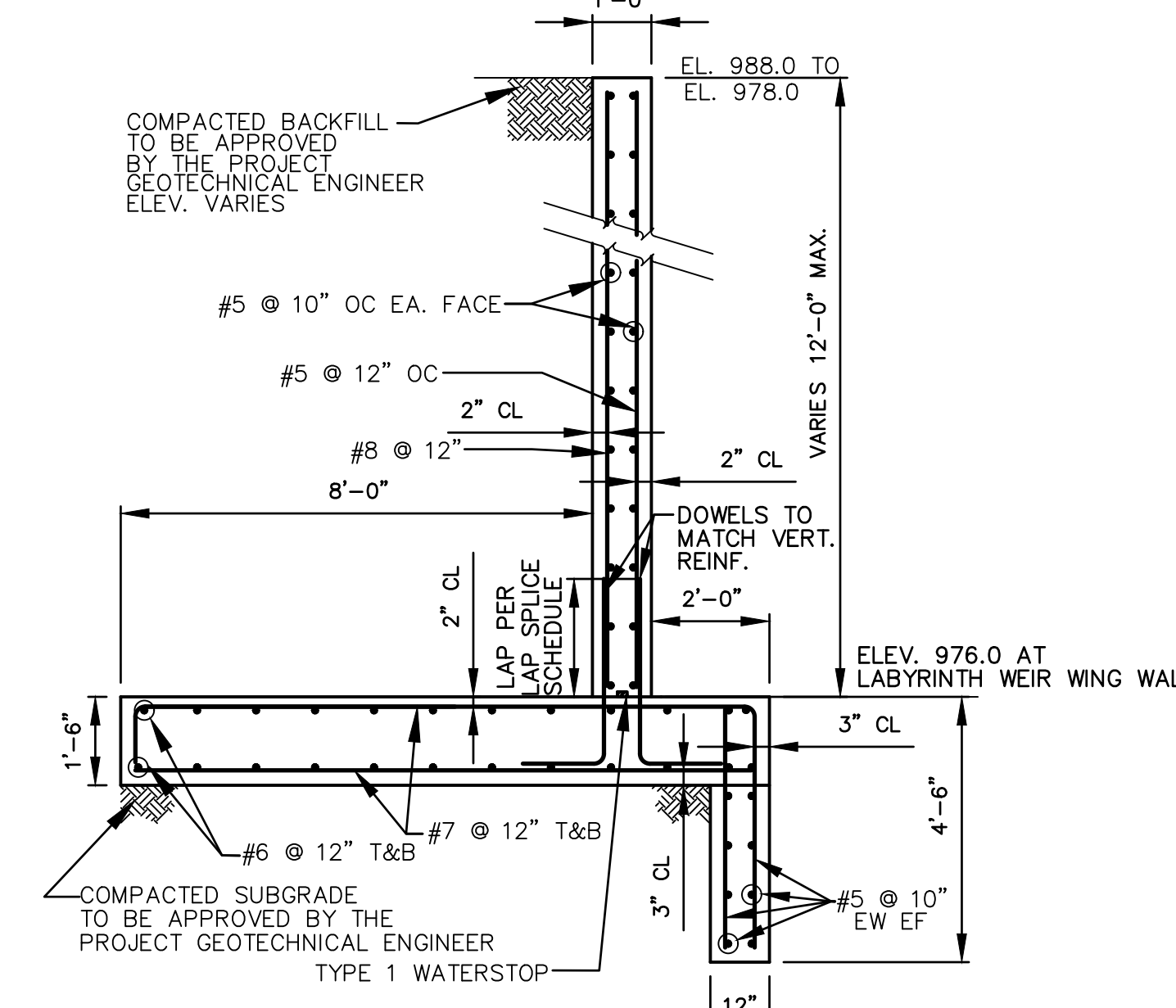
**12 DETAIL AT SLAB END**  
SCALE: 3/8" = 1'-0"



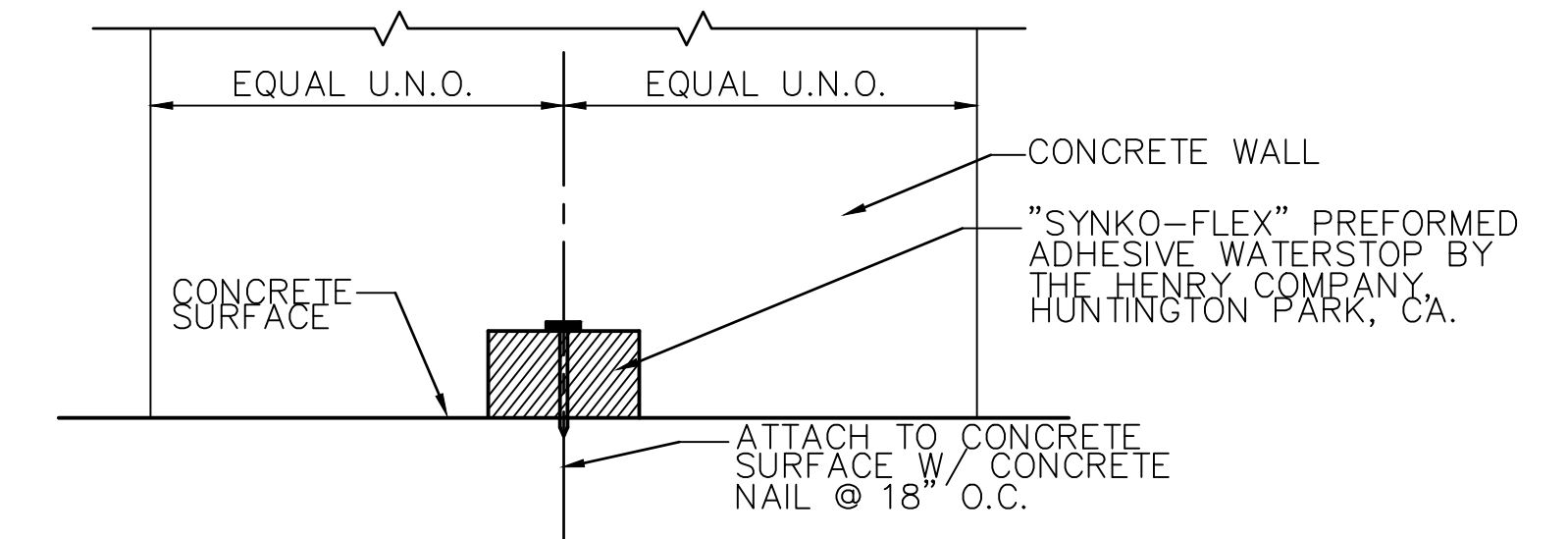
**MOVEMENT JOINT INTERSECTION DETAIL**  
N.T.S.



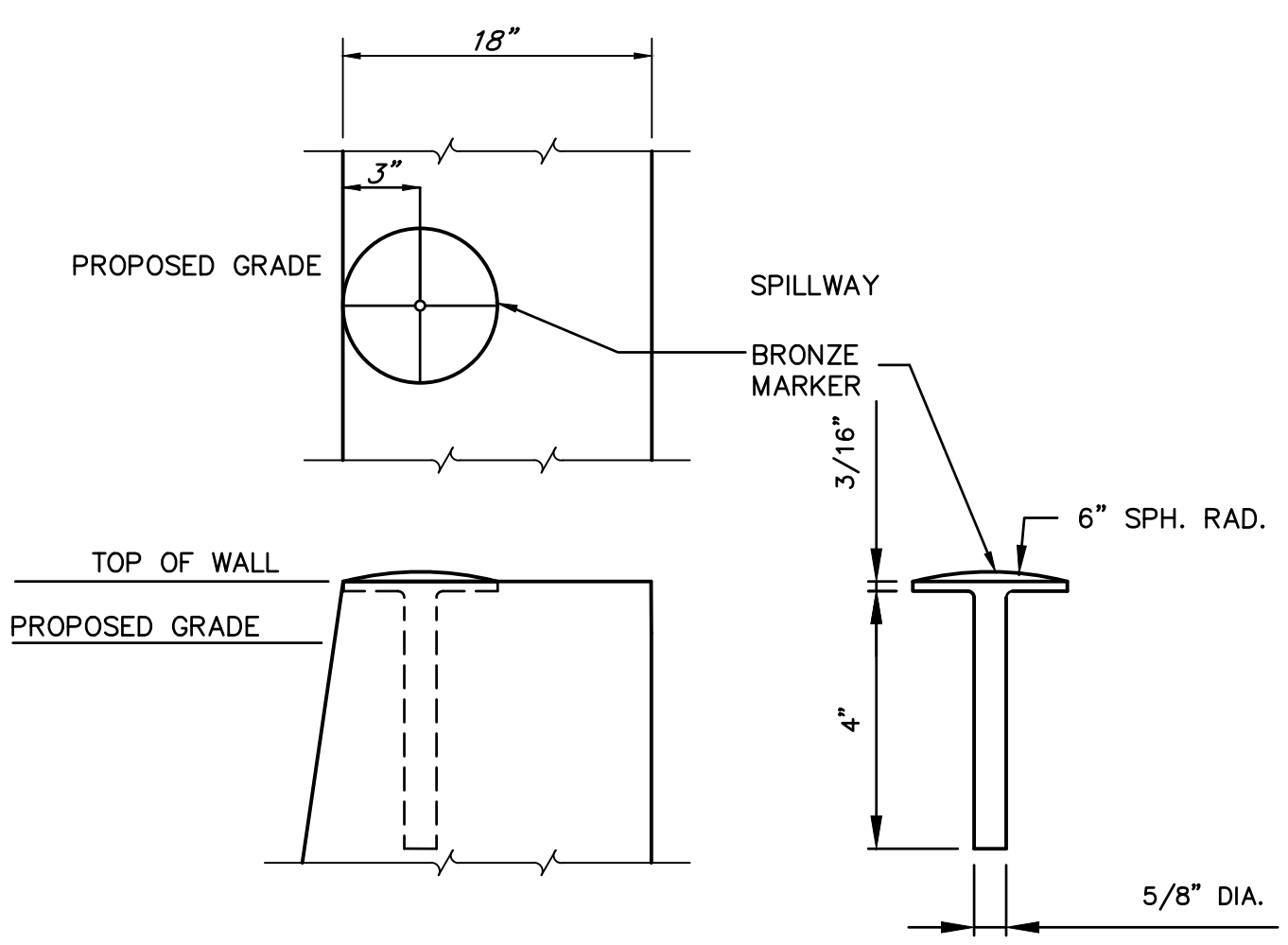
**D SECTION THRU UPPER WING WALL**  
SCALE: 3/8" = 1'-0"



**J SECTION THRU UPPER WING WALL**  
SCALE: 3/8" = 1'-0"



**TYPE 1 WATERSTOP DETAIL**  
N.T.S. (FOR USE AT CONSTRUCTION JOINTS)



**5 SETTLEMENT PLATE DETAIL**  
SCALE: NTS

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**24 HR. CONTACT PERSON**  
RIP ROBERTSON  
(470) 481-0205

NO.	REVISIONS	MADE	OKD.	DATE	DRAWING NO.	REFERENCE DRAWINGS

**GEORGIA REGISTERED PROFESSIONAL ENGINEER**  
No. 30441  
JASON F. RAPPLEAN  
06/19/2024

SCALE: AS SHOWN	DATE: 01/23
DESIGN BY: JR/JH	01/23
DRAWN BY: JH/EC	01/23
CHECKED BY: JR	01/23
CD# PEF 000707	EXP. 06/30/2024
JASON RAPPLEAN, PE, EDR	LEVEL 2 CERTIFICATION # 3031
EXPIRATION DATE:	12/07/26

**JOHNS HOMESTEAD PARK**  
**STRUCTURAL**  
**LOWER SPILLWAY**

**CITY OF TUCKER**

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